Application
19838-2024 Roadway Modernization
20434 - Hastings Highway 61 Modernization Project
Regional Solicitation - Roadways Including Multimodal Elements
Status:
Submitted
Submitted Date:
12/14/2023 4:04 PM

## Primary Contact

Feel free to edit your profile any time your information changes. Create your own personal alerts using My Alerts.

| Name:* | He/him/his |  | Charles | Eitemiller |
| :---: | :---: | :---: | :---: | :---: |
|  | Pronouns | First Name | Middle Name | Last Name |
| Title: | Finance Manager |  |  |  |
| Department: | Finance |  |  |  |
| Email: | CEitemiller@hastingsmn.gov |  |  |  |
| Address: | 101 4th St East |  |  |  |
| * | Hastings | Minnesota |  | 55033 |
|  | city | State/Province |  | Postal Code/Zip |
| Phone:* | 651-480-2347 |  |  |  |
|  | Phone |  |  | Ext. |

## Fax:

What Grant Programs are you most interested in?
Regional Solicitation - Roadways Including Multimodal Elements

## Organization Information

| Name: | HASTINGS, CITY OF |  |  |
| :---: | :---: | :---: | :---: |
| Jurisdictional Agency (if different): |  |  |  |
| Organization Type: | City |  |  |
| Organization Website: |  |  |  |
| Address: | 101 4THSTE |  |  |
| * | HASTINGS | Minnesota | 55033 |
|  | City | State/Province | Postal Code/Zip |
| County: | Dakota |  |  |
| Phone:* | 651-437-4127 |  |  |
| Fax: |  |  |  |
| PeopleSoft Vendor Number | 0000020950A1 |  |  |

## Project Information

Project Name
Primary County where the Project is Located
Cities or Townships where the Project is Located:
Jurisdictional Agency (If Different than the Applicant):

Hastings Highway 61 Modernization
Dakota
City of Hastings
Minnesota Department of Transportation

Brief Project Description (Include location, road name/functional class, type of improvement, etc.)

Highway 61 is a principal arterial in Hastings. The project proposes reconstruction from approximately 4th Street to 36th Street in Hastings. Improvements include multimodal improvements (new trail, new ped bridge over Vermillion River, close sidewalk gaps), new traffic signals, access modification and reduction for safety and simplified intersection movements, and two new roundabouts.

The north end of the project includes the conversion of intersections to median barrier (right-in, right-out) intersections providing a two-stage pedestrian crossing.

The TH 55 intersection is strained for capacity - modifications here include reduction of access, improved turning lanes, and enhanced pedestrian safety made possible in part by reconstruction of the deteriorating Todd Field wall.

The addition of a traffic signal at 18th Street will enable better freight access to Highway 61 while also providing better pedestrian crossing options through this commercial portion of the corridor.

At the County Road 47 intersection, a slight shift of alignment creates more space for the right turn lane from CR 47 to Hwy 61. Similarly, the elimination of on-street parking here (and elsewhere in the corridor) enables greater clarity for motorists to make turning movements with visibility to the newly daylighted intersection. A notable addition in this area is a new pedestrian bridge over the Vermillion River, which will keep bike/ped traffic separated from Highway 61 and encourage greater use of the entire trail system.

The busy Hwy 316 intersection will be rebuilt as a roundabout, creating safer operations for all users in the corridor, including residents, businesses, and interregional freight haulers.

The southern segment of the corridor will be augmented with new multipurpose trails and a gateway roundabout at 36th Street which will signal to motorists they are entering Hastings.
(Limit 2,800 characters; approximately 400 words)
TRANSPORTATIONIMPROVEMENT PROGRAM (TIP) DESCRIPTION - will be used in TIP US 61 VERMILLION STREET, FROM SOUTH CITY LIMITS TO 4TH ST IN if the project is selected for funding. See MnDOT's TIP description guidance. HASTINGS - RECONSTRUCT, SIDEWALK, PED/BIKE IMPROVEMENTS, SIGNALS, ADA
Include both the CSAHMSAS/TH references and their corresponding street names in the TIP Description (see Resources link on Regional Solicitation webpage for examples).
Project Length (Miles)
2.3
to the nearest one-tenth of a mile

## Project Funding

Are you applying for competitive funds from another source(s) to implement this Yes
project?
If yes, please identify the source(s) LRIP, TED
Federal Amount
\$7,000,000.00
Match Amount $\quad \$ 14,408,861.00$
Minimumof $20 \%$ of project total
Project Total
\$21,408,861.00
For transit projects, the total cost for the application is total cost minus fare revenues.

## Match Percentage

67.3\%

Minimumof 20\%
Compute the match percentage by dividing the match anount by the project total

# A minimum of $20 \%$ of the total project cost must cone fromnon-federal sources; additional match funds over the $20 \%$ minimum can cone fromother federal sources 

Preferred Program Year
Select one:
2028
Select 2026 or 2027 for TDM and Unique projects only. For all other applications, select 2028 or 2029.
Additional Program Years:
2027
Select all years that are feasible if funding in an earlier year becomes available.

## Project Information-Roadways

NOTE: If your project has already been assigned a State Aid Project \# (SAP or SP), please Indicate SAP\# here
SAP\#:

County, City, or Lead Agency
Functional Class of Road
Road System
TH, CSAH, MSAS, $\infty$. RD., TMP. RD., ATY STREET
Road/Route No.
i.e., 53 for CSAH 53

Name of Road
Example; 1st ST., MAINAVE
TERMIN:(Termini listed must be within 0.3 miles of any work)
From:
Road System
Road/Route No.
i.e., 53 for CSAH 53

Name of Road
Example; 1st ST., MAINAVE
To:
Road System
DO NOT INCLUDE LEGAL DESCRIPTION
Road/Route No.
i.e., 53 for CSAH 53

Name of Road
Example; 1st ST., MAINAVE
In the City/Cities of:
(List all cities within project linits)
OR:
At:
Road System
(TH, CSAH, MSAS, CO. RD., TMP. RD., City Street)
Road/Route No.
i.e., 53 for CSAH 53

Name of Road
Example; 1st ST., MAINAVE
In the City/Cities of:
(List all cities within project linits)
PROJECT LENGTH
Miles
2.3 miles
(nearest 0.1 miles)
Primary Types of Work (check all the apply)
New Construction
Reconstruction Yes
Resurfacing
Bituminous Pavement
Concrete Pavement
Roundabout
Yes
New Bridge
Yes
Bridge Replacement
Bridge Rehab
New Signal

City of Hastings
Principal Arterial
TH

61

Vermillion Street

City Street (MSA)
137

4th Street

City Street (MSA)

139

36th Street

Hastings

| Signal Replacement/Revision | Yes |
| :--- | :--- |
| Bike Trail | Yes |
| Other (do not include incidental items) | Wall reconstruction |
| BRIDGE/CULVERT PROJECTS (IF APPLICABLE) |  |
| Old Bridge/Culvert No.: |  |
| New Bridge/Culvert No.: |  |
| Structure is Over/Under <br> (Bridge or culvert name): |  |
| OTHER INFORMATION: | 55033 |
| Zip Code where Majority of Work is Being Performed | $04 / 03 / 2028$ |
| Approximate Begin Construction Date | $10 / 26 / 2029$ |
| Approximate End Construction Date | 1.4 |
| Miles of Trail (nearest 0.1 miles) | 0.1 |
| Miles of Sidewalk (nearest 0.1 miles) | 0 |
| Miles of trail on the Regional Bicycle Transportation Network (nearest 0.1 miles): | 0 |
| Is this a new trail? | Yes |

## Requirements - All Projects

All Projects

1. The project must be consistent with the goals and policies in these adopted regional plans: Thrive MSP 2040 (2014), the 2040 Transportation Policy Plan (2018), the 2040 Regional Parks Policy Plan (2018), and the 2040 Water Resources Policy Plan (2015).
Check the box to indicate that the project meets this requirement
Yes
2. The project must be consistent with the 2040 Transportation Policy Plan. Reference the 2040 Transportation Plan goals, objectives, and strategies that relate to the project.

Briefly list the goals, objectives, strategies, and associated pages: The project is consistent with the goals, objectives, and strategies from the 2040 Transportation Policy Plan. Highlights of the most applicable elements are summarized below:

Goal: A. Transportation System Stewardship
Objectives:
-Preserve and maintain a state of good repair

- Efficiently and cost-effectively move people and freight

Strategies:
A1 - Strategically preserving, maintaining and operating the transportation system
A2 - Regional transportation partners should...incorporate improvements for safety, lower-cost congestion management and mitigation, strategic capacity, bicycle, and pedestrian facilities.

Goal: B. Safety and Security
Objective: Reduce fatal and serious injury crashes and improve safety and security for all modes of passenger travel and freight transport

Strategies:
B1 - Regional transportation partners will incorporate safety and security considerations for all modes and users throughout the processes of planning, funding, construction, and operation.

B6 - Regional transportation partners will use best practices to provide and improve facilities for safe walking and bicycling, since pedestrians and bicyclists are the most vulnerable users of the transportation system.

These goals, objectives, and strategies are found within pages 2.2-2.8 of the 2040 Plan.
3. The project or the transportation problem/need that the project addresses must be in a local planning or programming document. Reference the name of the appropriate comprehensive plan, regional/statewide plan, capital improvement program, corridor study document [studies on trunk highway must be approved by the Minnesota Department of Transportation and the Metropolitan Council], or other official plan or program of the applicant agency [includes Safe Routes to School Plans] that the project is included in and/or a transportation problem/need that the project addresses.
List the applicable documents and pages: Unique projects are exempt MnDOT and the City of Hastings are completing a Highway 61 Corridor Study with from this qualifying requirement because of their innovative nature. final documentation including a resolution from the City Council which supports the final recommendations as provided by the project team through extensive public engagement. This Corridor Study builds upon prior City work, including the 2018 Vermillion Street Corridor Study, the 2020 Hastings 2040 Comprehensive Plan, and the 2021 Hastings People Movement Plan. MnDOT currently has a pavement preservation project on this corridor identified in its 2024-2027 State Transportation Improvement Program (STIP).
Limit 2,800 characters, approximately 400 words
4. The project must exclude costs for studies, preliminary engineering, design, or construction engineering. Right-of-way costs are only eligible as part of transit stations/stops, transit terminals, park-and-ride facilities, or pool-and-ride lots. Noise barriers, drainage projects, fences, landscaping, etc., are not eligible for funding as a standalone project, but can be included as part of the larger submitted project, which is otherwise eligible. Unique project costs are limited to those that are federally eligible.
Check the box to indicate that the project meets this requirement.

## Yes

5. Applicant is a public agency (e.g., county, city, tribal government, transit provider, etc.) or non-profit organization (TDM and Unique Projects applicants only). Applicants that are not State Aid cities or counties in the seven-county metro area with populations over 5,000 must contact the MnDOT Metro State Aid Office prior to submitting their application to determine if a public agency sponsor is required.
Check the box to indicate that the project meets this requirement. Yes
6. Applicants must not submit an application for the same project elements in more than one funding application category.

Check the box to indicate that the project meets this requirement. Yes
7. The requested funding amount must be more than or equal to the minimum award and less than or equal to the maximum award. The cost of preparing a project for funding authorization can be substantial. For that reason, minimum federal amounts apply. Other federal funds may be combined with the requested funds for projects exceeding the maximum award, but the source(s) must be identified in the application. Funding amounts by application category are listed belowin Table 1. For unique projects, the minimum award is $\$ 500,000$ and the maximum award is the total amount available each funding cycle (approximately $\$ 4,000,000$ for the 2024 funding cycle).

Strategic Capacity (Roadway Expansion): \$1,000,000 to \$10,000,000
Roadway Reconstruction/M odernization: \$1,000,000 to \$7,000,000
Traffic Management Technologies (Roadway System Management): \$500,000 to \$3,500,000
Spot M obility and Safety: \$1,000,000 to \$3,500,000
Bridges Rehabilitation/Repla cement: \$1,000,000 to \$7,000,000
Check the box to indicate that the project meets this requirement. Yes
8. The project must comply with the Americans with Disabilities Act (ADA).

Check the box to indicate that the project meets this requirement.
Yes
9. In order for a selected project to be included in the Transportation Improvement Program(TIP) and approved by USDOT, the public agency sponsor must either have a current Americans with Disabilities Act (ADA) self-evaluation or transition plan that covers the public right of way/transportation, as required under Title II of the ADA. The plan must be completed by the local agency before the Regional Solicitation application deadline. For future Regional Solicitation funding cycles, this requirement may include that the plan has undergone a recent update, e.g., within five years prior to application.
The applicant is a public agency that employs 50 or more people and has a completed ADA transition plan that covers the public right of way/transportation.

Yes
(TDM and Unique Project Applicants Only) The applicant is not a public agency subject to the self-evaluation requirements in Title II of the ADA.
Date plan completed:
Link to plan:
04/01/2019
https://www.hastingsmn.gov/city-government/city-departments/public-works/ada-transition-plan

The applicant is a public agency that employs fewer than 50 people and has a completed ADA self-evaluation that covers the public right of way/transportation.

Date self-evaluation completed:
Link to plan:
Upload plan or self-evaluation if there is no link
Upload as PDF
10. The project must be accessible and open to the general public.

Check the box to indicate that the project meets this requirement. Yes
11. The owner/operator of the facility must operate and maintain the project year-round for the useful life of the improvement. This includes assurance of year-round use of bicycle, pedestrian, and transit facilities, per FHWA direction established 8/27/2008 and updated 4/15/2019. Unique projects are exempt from this qualifying requirement.
Check the box to indicate that the project meets this requirement.
Yes
12. The project must represent a permanent improvement with independent utility. The term ?independent utility? means the project provides benefits described in the application by itself and does not depend on any construction elements of the project being funded from other sources outside the regional solicitation, excluding the required non-federal match. Projects that include traffic management or transit operating funds as part of a construction project are exempt from this policy.

Check the box to indicate that the project meets this requirement.
Yes
13. The project must not be a temporary construction project. A temporary construction project is defined as work that must be replaced within five years and is ineligible for funding. The project must also not be staged construction where the project will be replaced as part of future stages. Staged construction is eligible for funding as long as future stages build on, rather than replace, previous work.
Check the box to indicate that the project meets this requirement.

## Yes

14. The project applicant must send written notification regarding the proposed project to all affected state and local units of government prior to submitting the application

Check the box to indicate that the project meets this requirement.

## Roadways Including Multimodal Elements

1. All roadway projects must be identified as a principal arterial (non-freeway facilities only) or A-minor arterial as shown on the latest TAB approved roadway functional classification map. Bridge Rehabilitation/Replacement projects must be located on a minor collector and above functionally classified roadway in the urban areas or a major collector and above in the rural areas.
Check the box to indicate that the project meets this requirement. Yes
Roadway Strategic Capacity and Reconstruction/Modernization and Spot Mobility projects only:
2. The project must be designed to meet 10 -ton load limit standards.

Check the box to indicate that the project meets this requirement. Yes
Bridge Rehabilitation/Replacement and Strategic Capacity projects only:
3. Projects requiring a grade-separated crossing of a principal arterial freeway must be limited to the federal share of those project costs identified as local (non-MnDOT) cost responsibility using MnDOT?s ?Cost Participation for Cooperative Construction Projects and Maintenance Responsibilities? manual. In the case of a federally funded trunk highway project, the policy guidelines should be read as if the funded trunk highway route is under local jurisdiction.
Check the box to indicate that the project meets this requirement.
4. The bridge must carry vehicular traffic. Bridges can carry traffic from multiple modes. However, bridges that are exclusively for bicycle or pedestrian traffic must apply under one of the Bicycle and Pedestrian Facilities application categories. Rail-only bridges are ineligible for funding.
Check the box to indicate that the project meets this requirement.
Bridge Rehabilitation/Replacement projects only:
5. The length of the in-place structure is 20 feet or longer.

Check the box to indicate that the project meets this requirement.
6. The bridge must have a Local Planning Index (LPI) of less than 60 OR a National Bridge Inventory (NBI) Rating of 3 or less for either Deck Geometry, Approach Roadway, or Waterway Adequacy as reported on the most recent Minnesota Structure Inventory Report.
Check the box to indicate that the project meets this requirement.
Roadway Expansion, Reconstruction/Modernization, and Bridge Rehabilitation/Replacement projects only:
7. All roadway projects that involve the construction of a newexpanded interchange or newinterchange ramps must have approval by the Metropolitan Council/MnDOT Interchange Planning Review Committee prior to application submittal. Please contact David Evin at MnDOT (David. Evin@state.mn.us or 651-234-7795) to determine whether your project needs to go through this process as described in Appendix F of the 2040 Transportation Policy Plan.
Check the box to indicate that the project meets this requirement.

## Requirements - Roadways Including Multimodal Elements

| Specific Roadway Elements |  |
| :--- | ---: |
| CONSTRUCTION PROJECT E EMENTS/COST ESTIMATES | Cost |
| Mobilization (approx 5\% of total cost) | $\$ 769,300.00$ |
| Removals (approx 5\% of total cost) | $\$ 492,110.00$ |
| Roadway (grading, borrow, etc.) | $\$ 500,000.00$ |
| Roadway(aggregates and paving) | $\$ 3,599,864.00$ |
| Subgrade Correction (muck) | $\$ 0.00$ |
| Storm Sewer | $\$ 1,372,433.00$ |
| Ponds | $\$ 200,000.00$ |
| Concrete ltems (curb \& gutter, sidewalks, median barriers) | $\$ 3,341,469.00$ |
| Traffic Control | $\$ 969,700.00$ |
| Strining | $\$ 121,250.00$ |
| Signing | $\$ 121,250.00$ |
| Lighting | $\$ 505,125.00$ |
| Turf- Erosion \& Landscaping | $\$ 161,700.00$ |
| Bridge | $\$ 0.00$ |
| Retaining Walls | $\$ 195,500.00$ |
| Noise Wall (not calculated in cost effectiveness measure) | $\$ 0.00$ |
| Traffic Signals | $\$ 3,000,000.00$ |
| Wetland Mtigation | $\$ 0.00$ |
| Other Natural and Cultural Resource Protection | $\$ 0.00$ |
| RR Crossing | $\$ 0.00$ |
| Roadway Contingencies | $\$ 2,363,200.00$ |
| Other Roadway Elements | $\$ 1,269,550.00$ |
| Totals | $\$ 18,982,451.00$ |

## Specific Bicycle and Pedestrian Elements

CONSTRUCTION PROJECT E FMENTS/COST ESTIMATES

| Path/Trail Construction | $\$ 905,800.00$ |
| :--- | ---: |
| Sidewalk Construction | $\$ 810,000.00$ |
| On-Street Bicycle Facility Construction | $\$ 0.00$ |
| Right-of-Way | $\$ 0.00$ |
| Pedestrian Curb Ramps (ADA) | $\$ 210,610.00$ |
| Crossing Aids (e.g., Audible Pedestrian Signals, HAWK) | $\$ 0.00$ |
| Pedestrian-scale Lighting | $\$ 0.00$ |
| Streetscaping | $\$ 0.00$ |
| Wayfinding | $\$ 0.00$ |
| Bicycle and Pedestrian Contingencies | $\$ 0.00$ |
| Other Bicycle and Pedestrian Elements | $\$ 500,000.00$ |
| Totals | $\$ 2,426,410.00$ |
|  |  |
| Specific Transit and TDM Elements | Cost |
| CONSTRUCTION PROJECT EEMENTS/COST ESTIMATES | $\$ 0.00$ |
| Fixed Guideway Elements | $\$ 0.00$ |
| Stations, Stops, and Terminals | $\$ 0.00$ |
| Support Facilities | $\$ 0.00$ |
| Transit Systems (e.g. communications, signals, controls, fare collection, etc.) | $\$ 0.00$ |
| Vehicles | $\$ 0.00$ |
| Contingencies | $\$ 0.00$ |
| Right-of-Way | $\$ 0.00$ |
| Other Transit and TDMElements | $\$ 0.00$ |

## Transit Operating Costs

| Number of Platform hours | 0 |
| :--- | :--- |
| Cost Per Platform hour (full loaded Cost) | $\$ 0.00$ |
| Subtotal | $\$ 0.00$ |
| Other Costs - Administration, Overhead,etc. | $\$ 0.00$ |

## PROTECT Funds Eligibility

One of the newfederal funding sources is Promoting Resilient Operations for Transformative, Efficient, and Cost-Saving Transportation (PROTECT). Please describe which specific elements of your project and associated costs out of the Total TAB-Eligible Costs are eligible to receive PROTECT funds. Examples of potential eligible items may include: storm semer, ponding, erosion control/landscaping, retaining walls, newbridges over floodplains, and road realignments out of floodplains.
INFORMATION: Promoting Resilient Operations for Transformative, Efficient, and Cost-Saving Transportation (PROTECT) Formula Program Implementation Guidance (dot.gov).
Response:

## Totals

| Total Cost | $\$ 21,408,861.00$ |
| :--- | :--- |
| Construction Cost Total | $\$ 21,408,861.00$ |
| Transit Operating Cost Total | $\$ 0.00$ |

## Measure B: Project Location Relative to Jobs, Manufacturing, and Education

Existing Employment within 1 Mile: 5991
Existing Manufacturing/Distribution-Related Employment within 1 Mile: 1360
Existing Post-Secondary Students within 1 Mile: 14
Upload Map
1702263662195_Regional Economy.pdf
Please upload attachment in PDF form

## Measure C: Current Heavy Commercial Traffic

RESPONSE: Select one for your project, based on the updated 2021 Regional Truck Corridor Study:
Along Tier 1: Yes
Miles: $\quad 1.7$
(to the nearest 0.1 miles)
Along Tier 2 :
Miles:

| Along Tier 3: |
| :--- |
| Miles: <br> (to the nearest 0.1 miles) <br> The project provides a direct and immediate connection (i.e., intersects) with <br> either a Tier 1, Tier 2, or Tier 3 corridor: <br> None of the tiers: <br>  <br> Measure A: Current Daily Person Throughput <br> Location <br> Current AADT Volume <br> Existing Transit Routes on the Project <br> For New Roadways only, list transit routes that will likely be diverted to the new proposed roadway (if applicable). <br> Upload Transit Connections Map <br> Please upload attachment in PDF form |

## Response: Current Daily Person Throughput

| Average Annual Daily Transit Ridership | 0 |
| :--- | :--- |
| Current Daily Person Throughput | 40300.0 |

## Measure B: 2040 Forecast ADT

Use Metropolitan Council model to determine forecast (2040) ADT volume No
If checked, METC Staff will provide Forecast (2040) ADT volume
OR
Identify the approved county or city travel demand model to Dakota County Travel Demand Model
Forecast (2040) ADT volume 40000

## Measure A: Engagement

i. Describe any Black, Indigenous, and People of Color populations, low-income populations, disabled populations, youth, or older adults within a $1 / 2$ mile of the proposed project. Describe how these populations relate to regional context. Location of affordable housing will be addressed in Measure C.
ii. Describe how Black, Indigenous, and People of Color populations, low-income populations, persons with disabilities, youth, older adults, and residents in affordable housing were engaged, whether through community planning efforts, project needs identification, or during the project development process.
iii. Describe the progression of engagement activities in this project. A full response should answer these questions:

[^0]The City of Hastings has a higher white population (90.6\%) than the Twin Cities metro region as a whole ( $73.1 \%$ ). The BIPOC community in Hastings is $\sim 9.4 \%$. The population with a disability in Hastings (14.8\%) is higher than the metro area (11.8\%) and low income (below 200\% poverty level) is slightly below (19.3\%) the metro area (21.2\%).

An extensive, multi-step engagement process was used as part of study development: Phase one engagement presented existing conditions, including past studies, and gathered the community?s issues and experiences traveling along or across Hwy 61. Phase two engagement presented what we heard in phase one and gathered feedback on evaluation criteria and design concepts developed using data from phase one. Phase three presented what we heard in phase two and a proposed vision for the corridor for community feedback.

Engagement activities included three public meetings, a pop-up at community event, door-knocking businesses, and online engagement activities. The community was made aware of engagement opportunities by social media, fliering, multiple postcard mailers, and notice in the Hastings newsletter mailed to every property. Through the three rounds of engagement, more than 200 people attended in-person public meetings and more than 350 surveys or online map comments were received including 454 comments on the design concepts during phase two.

After completion of the study but prior to the City of Hastings resolution of support, the project team met individually with businesses and key stakeholders along the corridor to review the vision and consider whether refinements were needed. Some of the notable changes came from discussions with the small businesses to understand their customer movements and access and requirements? these discussions resulted in modifications to the allowable turning movements and the location of business access that meet business and community needs without impacting the design intent for the corridor as a whole.

Public engagement has continued through and beyond the corridor study, with updates to the project web site https://www.dot.state.mn.us/metro/projects/hwy61hastings/ routinely made. relate to:
? pedestrian and bicycle safety improvements;
? public health benefits;
? direct access improvements for residents or improved access to destinations such as jobs, school, health care, or other;
? travel time improvements;
? gap closures;
? newtransportation services or modal options;
? leveraging of other beneficial projects and investments;
? and/or community connection and cohesion improvements.
This is not an exhaustive list. A full response will support the benefits claimed, identify benefits specific to Disadvantaged communities residing or engaged in activities near the project area, identify benefits addressing a transportation issue affecting Disadvantaged communities specifically identified through engagement, and substantiate benefits with data.

Acknowedge and describe any negative project impacts to Black, Indigenous, and People of Color populations, Iow-income populations, children, people with disabilities, youth, and older adults. Describe measures to mitigate these impacts. Unidentified or unmitigated negative impacts may result in a reduction in points.
Belowis a list of potential negative impacts. This is not an exhaustive list.
? Decreased pedestrian access through sidewalk removal / narrowing, placement of barriers along the walking path, increase in auto-oriented curb cuts, etc.
? Increased speed and/or ?cut-through? traffic.
? Removed or diminished safe bicycle access.
? Inclusion of some other barrier to access to jobs and other destinations.

The City of Hastings is neighbors to the Prairie Island Indian Community (PIIC). We have engaged with their leadership and understand the economic and cultural importance regional access of TH 61 provides to Treasure Island Resort and Casino and events such as Pow Wows (see the attached letter of support from Prairie Island Indian Community). This project request would allow the investment in mobility improvements to keep travel times consistent with todays operations rather than allow travel times to delay to the point of gridlock if nothing is done. Furthermore, the safety improvements along the corridor and specific to major intersections will result in less crashes, reduced congestion, and engineered solutions to give more decision-making time to drivers along the corridor. TH 61 carries the majority of visitors to PIICs facilities, and support for these timely safety and mobility improvements is vital to their future success in the region.

The City of Hastings also engaged with THRIVE (a local organization that was created out of the Black Lives Matter movement) to relate these improvements to the BIPOC community. One significant benefit of this project was the commitment to a more walkable corridor. TH 61 is deficient in ADA sidewalk facilities and has several gaps in bike/pedestrian connectivity to places of interest. Further TH 61 acts as a divide of the community with the challenges of crossing a corridor that carries $30,000+$ vehicles per day with no pedestrian crossing enhancements. It was stated that the BIPOC community relies on multiple modes of transportation, especially with many families owning one vehicle. Pedestrian and bicycle connectivity along the corridor (and crossing it) opens up the ability for the entirety of the families to work, shop, attend church, and get the services they need (THRIVE is located on the project corridor). There are also key attractions that will benefit the BIPOC population, such as connections to the Downtown Business District, Lake Rebecca fishing (said to be a rite of passage for local indigenous groups), Pavilion located in Levee Park, and the Vermillion Falls Park. A key connection in this list is the pedestrian bridge across the Vermillion River and associated trail/sidewalk replacements and extensions to connect the TH 61 corridor. The Vermillion Falls Park is a key destination that the BIPOC community has requested multi-modal connection to. This would allow for a gathering space with opportunity for events and experiences for the BIPOC community.

We were informed that our BIPOC population owns businesses, such as the Coratel Inn that depends on mobility and safe access to attract business and stay in this market.

Describe the project?s benefits to current and future affordable housing residents within $1 / 2$ mile of the project. Benefits must relate to affordable housing residents. Examples may include:
? specific direct access improvements for residents
? improved access to destinations such as jobs, school, health care or other;
? newtransportation services or modal options;
? and/or community connection and cohesion improvements.
This is not an exhaustive list. Since residents of affordable housing are more likely not to own a private vehicle, higher points will be provided to roadway projects that include other multimodal access improvements. A full response will support the benefits claimed, identify benefits specific to residents of affordable housing, identify benefits addressing a transportation issue affecting residents of affordable housing specifically identified through engagement, and substantiate benefits with data.

Response:
TH 61 Vermillion Street connects residents to retail and service opportunities. The City of Hastings adopted a Fair Housing Plan in 2019 to support providing housing opportunities to those in need. Within a half mile of the corridor there are over 160 subsidized housing units. Guardian Angels, a 30 -unit apartment building operated by Common Bond Communities utilizes a historic school building. The Quill, a 90 -unit independent living senior housing building was recently opened. A Housing Tax Increment Finance District were established for both Guardian Angels and the Quill to ensure affordable rents. Mississippi Terrace, a 40-unit senior housing facility is operated by the Dakota County Community Development Agency (CDA) with all unit?s income restricted. Approximately 450 affordable housing units are available or under construction within one half mile of the corridor:

- Artspace of Hastings, a 37-unit apartment providing livelwork units for artists has implemented caps on rental prices.
- Hastings Terrace containing 65 lots for manufactured homes and travel trailers abuts the corridor.
- Suite Living of Hastings, a 32-unit senior living facility has been granted building approvals with construction anticipated in 2024.
- Current 33 a 213-unit market rate apartment building is currently under construction at the south end of the corridor.
- Lake Isabel Flats, a 90-unit market rate apartment is currently under construction in downtown.

Dakota County CDA has purchased additional land near the corridor for further workforce housing construction in the near future. Most of the 2,500 existing single-family housing units located within one half mile of the corridor were constructed prior to 1950 and are considered naturally occurring affordable housing (NOAH). TH 61 serves as a primary commercial and service corridor within the City containing a grocery store, two pharmacies, medical facilities, elementary school, and churches.

Organizations along the corridor serve those with socioeconomic challenges. Hastings Family Service operates a food shelf and provides social service assistance. Rise Up Recovery, a provides peer services and housing for those recovering from substance abuse. Stepping Out provides counseling and assistance for those with Prader-Willi syndrome.

TH 61 bisects the City and serves as both a corridor of opportunity as well as a physical barrier preventing access from one side to the other. The inclusion of improved sidewalks will improve pedestrian access. Controlled intersections and widened median areas will help facilitate crossing of the corridor. Hastings does not have a fixed route mass transit system adding to the importance of nonmotorized modal connections.

## Measure D: BONUS POINTS

Project is located in an Area of Concentrated Poverty:
Project?s census tracts are above the regional average for population in poverty or population of color (Regional Environmental Justice Area):
Project located in a census tract that is below the regional average for population Yes in poverty or populations of color (Regional Environmental Justice Area):
Upload the ?Socio-Economic Conditions? map used for this measure.

## Measure A: Year of Roadway Construction

| Year of Original |
| :---: |
| Roadway |
| Construction or |


| Segment |
| :---: |
| Most Recent |

Rength

## Total Project Length

Total Project Length (as entered in "Project Information" form)

## 2.3

Average Construction Year
Weighted Year 1960

Total Segment Length (Miles)
Total Segment Length
2.3

## Measure B: Geometric, Structural, or Infrastructure Improvements

Improved roadway to better accommodate freight movements: Response:
(Limit 700 characters; approximately 100 words) Improved clear zones or sight lines:
Response:
(Limit 700 characters; approximately 100 words)
Improved roadway geometrics:
Response:

Yes
The proposed project will reduce conflict points for freight traffic and improve the overall traffic flow. Specific elements include eliminating several bays of on-street parking, closing 23 accesses, restricting 20 accesses to either $3 / 4$-access or right$\mathrm{in} /$ right-out, and adding a new backage road for local land use connections away from the highway. Another key aspect is the realignment of key local road intersection with the highway and additional of a new traffic signal. This local road has heavy freight use by Ardent Mills and other local industry. The improvement will reduce delay for freight turning movements through the intersection.

Yes
As proposed, the project improvements to sight distance include 2 intersection realignments, eliminating on-street parking, and reducing access. One local road intersection realignment will align with the opposing side and reduce the road width to eliminate existing driver confusion on where to turn. The second intersection realignment will provide a 90-degree intersection with a County Road instead of the current skewed connection. Eliminating parking removes those vehicles from driver sight lines. Closing 23 accesses and restricting 20 others will increase the distance between conflict points, minimizing the number of turning movements within a driver?s sight lines.

## Yes

The proposed project includes multiple elements of improved geometry for this busy highway. On-street parking is eliminated to reduce conflicts, provide a boulevard separation for pedestrians, and provide a consistent curb lane along the corridor. Turn lanes will be added to two primary intersections, one with another highway and one with a county road. A roundabout will be constructed at an intersection with another highway to improve traffic flow and safety. Overall reduction in access will reduce conflicts for vehicles and pedestrians. The center median will be reconstructed to separate driving directions and provide space for left turn lanes or a refuge area for crossing pedestrians.

Access management enhancements:
Response:
(Limit 700 characters; approximately 100 words)
Vertical/horizontal alignment improvements:
Response:
(Limit 700 characters; approximately 100 words)
Improved stormwater mitigation:
Response:
(Limit 700 characters; approximately 100 words)
Signals/lighting upgrades:
Response:
(Limit 700 characters; approximately 100 words)
Other Improvements
Response:

Yes
The proposed access management of this project impacts a total of 43 intersecting private and public connections over the 2.3-mile corridor. Full access is reduced by approximately 19 accesses per mile. One public intersection is closed along with a reduction to right-in/right-out of 10 intersecting streets. Another 2 public streets will be reduced to $3 / 4$-access. 22 private driveways will be closed along the highway. An additional 6 private driveways will become right-in/right-out only and 2 will be reduced to $3 / 4$-access.

Yes
This project will improve the horizontal alignment of two intersecting public roads. A local road connection with heavy freight use will be aligned with the opposing side to create a four-legged intersection at 90 -degrees. A county road will also be realigned to remove the skew and provide a 90-degree intersection. These realignments will improve turning movements to and from the roads and improve sight lines for drivers.

Yes
Stormwater mitigation is expected to improve through the project by reducing the overall amount of impervious pavement and providing new areas for vegetation and landscaping. Pavement is expected to decrease through the elimination of onstreet parking, the reduction in access driveways, and the construction of 2 roundabouts in place of multilane traditional intersections. The project also includes several areas of increased center median width, where plantings can be placed instead of concrete. Similarly, 6 spots have been identified for community space making opportunities, which are envisioned as natural areas with City information like historical markers.

Yes
The existing 5 signal systems on this corridor will be reconstructed. An additional signal system will also be added at 1 intersection. These signals will be upgraded to include flashing yellow arrow operation. The project is also considering blankout No Right Turn on Red signs to better control vehicle movements against pedestrian crossings. All signals will have ADA compliant push buttons and crossings with overhead lighting. Street lighting will be added at the 2 proposed roundabout intersections.

Yes
This project includes multiple other improvements to provide for a complete corridor that is a part of the community as opposed to a barrier. A retaining wall will be reconstructed to allow space for ADA improvements and a turn lane. Protection will be installed around the adjacent high school stadium to protect against off-tracking vehicles. A pedestrian bridge will improve the connection across a river and include a connection to a regional trail. The existing downtown boulevard brick pattern and color will be included on part of the highway to better recognize its connection to the City and improve the sense of place.
(Limit 700 characters; approximately 100 words)

## Measure A: Congestion Reduction/Air Quality

| Total Peak Hour | Total Peak Hour | Total Peak Hour | Volume | Volume | Total Peak | Total | Total | EXPLANATION | Synchro or HCM Reports |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Delay Per Vehicle | Delay Per Vehicle | Delay Per Vehicle | without | with the | Hour Delay | Peak | Peak | of |  |
| Without The | With The Project | Reduced by | the | Project | without | Hour | hour | methodology |  |
| Project | (Seconds/Vehicle) | Project | Project | (Vehicles | the | Delay by | Delay | used to |  |
| (Seconds/Vehicle) |  | (Seconds/Vehicle) | (Vehicles | Per | Project: | the | Reduced | calculate |  |
|  |  |  | per | Hour): |  | Project: | by | railroad |  |
|  |  |  | hour) |  |  |  | project | crossing |  |
|  |  |  |  |  |  |  |  | applicable. |  |

$89.0 \quad 40.0 \quad 49.0 \quad 11406 \quad 11406 \quad 1015134.0456240 .0558894 .0$ N/A $\quad 1702332117333$ Synchro Outputs.pdf

## Vehicle Delay Reduced

## Measure B: Roadway projects that do not include new roadway segments or railroad grade-separation elements

| Total (CO, | Total (CO, | Total (CO, |
| :---: | :---: | :---: |
| NOX, and | NOX, and | NOX, and |
| VOC) Peak | VOC) Peak | VOC) Peak |
| Hour | Hour | Hour |
| Emissions | Emissions | Emissions |
| without the | with the | Reduced by |
| Project | Project | the Project |
| (Kilograms): | (Kilograms): | (Kilograms): |
| 22.55 | 18.99 | 3.56 |
| 23 | 19 | 4 |

## Total

| Total Emissions Reduced: | 3.56 |
| :--- | :--- |

Upload Synchro Report
1702514069066_02 Synchro Outputs.pdf

Please upload attachment in PDF form (Save Form then click 'Edit' in top right to upload file.)

Measure B: Roadway projects that are constructing new roadway segments, but do not include railroad gradeseparation elements (for Roadway Expansion applications only):

| Total (CO, | Total (CO, | Total (CO, |
| :---: | :---: | :---: |
| NOX, and | NOX, and | NOX, and |
| VOC) Peak | VOC) Peak | VOC) Peak |
| Hour | Hour | Hour |
| Emissions | Emissions | Emissions |
| without the | with the | Reduced by <br> Project |
| Project | the Project |  |
| (Kilograms): | (Kilograms): | (Kilograms): |
| 0 | 0 | 0 |
| 0 | 0 | 0 |

## Total Parallel Roadway

Emissions Reduced on Parallel Roadways 0

Upload Synchro Report
Please upload attachment in PDF form (Save Form then click 'Edit' in top right to upload file.)

## New Roadway Portion:

Cruise speed in miles per hour with the project: 0
Vehicle miles traveled with the project: ..... 0
Total delay in hours with the project: ..... 0
Total stops in vehicles per hour with the project: ..... 0
Fuel consumption in gallons: ..... 0
Total (CO, NOX, and VOC) Peak Hour Emissions Reduced or Produced on New Roadway (Kilograms): ..... 0
EXPLANATION of methodology and assumptions used:(Limit 1,400characters; approximately 200 words)
Total (CO, NOX, and VOC) Peak Hour Emissions Reduced by the Project (Kilograms): ..... 0.0
Measure B: Roadway projects that include railroad grade-separation elements
Cruise speed in miles per hour without the project: ..... 0
Vehicle miles traveled without the project: ..... 0
Total delay in hours without the project: ..... 0
Total stops in vehicles per hour without the project: ..... 0
Cruise speed in miles per hour with the project: ..... 0
Vehicle miles traveled with the project: ..... 0
Total delay in hours with the project: ..... 0
Total stops in vehicles per hour with the project: ..... 0

| Fuel consumption in gallons (F1) | 0 |
| :--- | :--- |
| Fuel consumption in gallons (F2) | 0 |
| Fuel consumption in gallons (F3) 0 <br> Total (CO, NOX, and VOC) Peak Hour Emissions Reduced by the Project <br> (Kilograms): 0 <br> EXPLANATION of methodology and assumptions used:(Limit 1,400 <br> characters; approximately 200 words)  ( 10 |  |

## Measure A: Roadway Projects that do not Include Railroad Grade-Separation Elements

Crash Modification Factor Used:
(Limit 700 Characters; approximately 100 words)
Rationale for Crash Modification Selected:

CMFs at various intersections include conversion to right-in right out intersection (9821), raised median for pedestrians (8800), presence of three-leg intersection vs. four-leg intersection (5233), replace direct left turn with right turn/U-turn (351), install traffic signal (7983), provide right turn lane on one major-road approach (286), convert from minor-road stop to roundabout (227), and prohibit on-street parking (154)

These modifications come directly from the Highway 61 Hastings Corridor Study which used extensive public engagement to create a vision for the corridor through Hastings.

The selected CMFs closely match the improvements being proposed for safety and operational benefits, and most closely match the project area, prior conditions, and applicability criteria in the CMF information. No direct CMF was found for converting 14th Street to $3 / 4$ access, so CMF 351 (replace direct left turn with right turn/U-turn) was used instead. Although a U-turn is not being encouraged or designed for in this improvement, this CMF most closely represents the safety benefits of eliminating a direct left turn from a minor street approach.
(Limit 1400 Characters; approximately 200 words)

| Project Benefit (\$) from B/C Ratio | $\$ 26,396,962.00$ |
| :--- | :--- |
| Total Fatal (K) Crashes: | 1 |
| Total Serious Injury (A) Crashes: | 2 |
| Total Non-Motorized Fatal and Serious Injury Crashes: | 1 |
| Total Crashes: | 190 |
| Total Fatal (K) Crashes Reduced by Project: | 0 |
| Total Serious Injury (A) Crashes Reduced by Project: | 0 |
| Total Non-Motorized Fatal and Serious Injury Crashes Reduced by Project: | 0 |
| Total Crashes Reduced by Project: | 40 |
| Worksheet Attachment | 1702332472850 CMFs for Regional Solicitation.pdf |
| Pease upad attachment PDF form |  |

Project Benefit (\$) from B/C Ratio

Total Serious Injury (A) Crashes:
Total Non-Motorized Fatal and Serious Injury Crashes:
Total Crashes:

1702332472850_CMFs for Regional Solicitation.pdf

Please upload attachment in PDF form

## Roadway projects that include railroad grade-separation elements:

Current AADT volume: 0

Average daily trains: 0
Crash Risk Exposure eliminated: 0

## Measure B: Pedestrian Safety

Determine if these measures do not apply to your project. Does the project match either of the following descriptions?
If either of the items are checked yes, then score for entire pedestrian safety measure is zero. Applicant does not need to respond to the sub-measures and can proceed to the next section.
Project is primarily a freeway (or transitioning to a freeway) and does not provide No safe and comfortable pedestrian facilities and crossings.
Existing location lacks any pedestrian facilities (e.g., sidewalks, marked crossings, wide shoulders in rural contexts) and project does not add pedestrian elements (e.g., reconstruction of a roadway without sidewalks, that doesn?t also add pedestrian crossings and sidewalk or sidepath on one or both sides).

SUB-M EASURE 1: Project-Based Pedestrian Safety Enhancements and Risk Elements
To receive maximum points in this category, pedestrian safety countermeasures selected for implementation in projects should be, to the greatest extent feasible, consistent with the countermeasure recommendations in the Regional Pedestrian Safety Action Plan and state and national best practices. Links to resources are provided on the Regional Solicitation Resources web page.

Please answer the following two questions with as much detail as possible based on the known attributes of the proposed design. If any aspect referenced in this section is not yet determined, describe the range of options being considered, to the greatest extent available. If there are project elements that may increase pedestrian risk, describe howthese risks are being mitigated.
 roundabouts

Treatments and countermeasures should be well-matched to the roadway?s context (e.g., appropriate for the speed, volume, crossing distance, and other location attributes). Refer to the Regional Solicitation Resources web page for guidance links.

Response:
Multiple unsignalized intersections will be converted to signals (one) or roundabouts (two). These changes increase safety by reducing speeds, signaling to motorists that they are entering an urban environment, simplifying pedestrian movements, and in the case of the 18th Street realignment and signalization, the crossing distance for pedestrians is reduced.

One of the busiest intersections in the corridor is at 10th Street. The west leg of 10th Street will be converted to one-way traffic only, thereby simplifying the potential conflicts for pedestrians at this location (adjacent to the high school football stadium)

Gaps in the sidewalk/trail system adjacent to Hwy 61 will be closed with new sidewalks and made safer through the elimination of access points, thereby reducing pedestrian/vehicle conflicts.

Additionally, two unsignalized intersections in the residential land use areas will be converted to right-in/right out movements with a median barrier on Hwy 61, providing a refuge for two-stage crossing of Hwy 61, changing what is currently a five-lane section to two, two-lane crossings. These measures, along with striping lanes to 11-foot width, are anticipated to slow motorist speed and provide a more comfortable pedestrian environment. Two locations in the corridor include a widened or new right turn lane. In both instances, the change creates a safer environment by incorporating other elements such as elimination of parking (better sightlines for all users), wider sidewalks, better defined crossings, and reduced driver confusion.
(Limit 2,800 characters; approximately 400 words)
Is the distance in between signalized intersections increasing (e.g., removing a signal)?
Select one:

## No

If yes, describe what measures are being used to fill the gap between protected crossing opportunities for pedestrians (e.g., adding High-Intensity Activated Crosswalk beacons to help motorists yield and help pedestrians find a suitable gap for crossing, turning signal into a roundabout to slowmotorist speed, etc.).
Response:
(Linit 1,400 characters; approximately 200 words)
Will your design increase the crossing distance or crossing time across any leg of an intersection? (e.g., by adding turn or through lanes, widening lanes, using a multi-phase crossing, prohibiting crossing on any leg of an intersection, pedestrian bridge requiring length detour, etc.). This does not include any increases to crossing distances solely due to the addition of bike lanes (i.e., no other through or turn lanes being added or widened).
Select one:
If yes,
? How many intersections will likely be affected?
Response: 2
? Describe what measures are being used to reduce exposure and delay for pedestrians (e.g., median crossing islands, curb bulb-outs, etc.)
Response:
(Limit 1,400 characters; approximately 200 words)
? If grade separated pedestrian crossings are being added and increasing crossing time, describe any features that are included that will reduce the detour required of pedestrians and make the separated crossing a more appealing option (e.g., shallowtunnel that doesn?t require much elevation change instead of pedestrian bridge with numerous suitchbacks).
Response:
(Limit 1,400 characters; approximately 200 words) enhanced crossing opportunity).

## Response:

(Linit 1,400 characters; approximately 200 words)
2. Describe how motorist speed will be managed in the project design, both for through traffic and turning movements. Describe any project-related factors that may affect speed directly or indirectly, even if speed is not the intended outcome (e.g., vider lanes and turning radii to facilitate freight movements, adding turn lanes to alleviate peak hour congestion, etc.). Note any strategies or treatments being considered that are intended to help motorists drive slower (e.g., visual narrowing, narrowlanes, truck aprons to mitigate wide turning radii, etc.) or protect pedestrians if increasing motorist speed (e.g., buffers or other separation from moving vehicles, crossing treatments appropriate for higher speed roadways, etc.).

Roundabouts at critical locations in the southern portion of the corridor serve as gateway features to the corridor to encourage slower speeds, lanes will be striped at 11 feet wide - the narrowest we felt comfortable with for a high use freight corridor. This, combined with addition of median barriers for intersection closures will provide multiple cues to motorists that make the corridor feel tighter and encourage slower speeds.

One of the aspects of this corridor is the high demand left turning movement from Hwy 61 to Hwy 316. The addition of a roundabout at this intersection will help limit the feeling that motorists need to race to the left turn lane knowing that both lanes can access the left turn move through the roundabout. This will further encourage safer driving in the corridor.
(Limit 2,800 characters; approximately 400 words)
If known, what are the existing and proposed design, operation, and posted speeds? Is this an increase or decrease from existing conditions?
Response:
No change in design, operation, and/or posted speeds are anticipated
(Limit 1,400 characters; approximately 200 words)
SUB-M EASURE 2: Existing Location-Based Pedestrian Safety Risk Factors
 factors are present. Applicants receive more points if more risk factors are present.
Existing road configuration is a One-way, $3+$ through lanes
or
Existing road configuration is a Two-way, 4+ through lanes
Existing road has a design speed, posted speed limit, or speed study/data
showing 85th percentile travel speeds in excess of 30 MPH or more
Existing road has AADT of greater than 15,000 vehicles per day Yes

List the AADT 31000
SUB-M EASURE 3: Existing Location-Based Pedestrian Safety Exposure Factors
These factors are based on based on trends and patterns observed in pedestrian crash analysis done for the Regional Pedestrian Safety Action Plan. Check off how many of the following existing location exposure factors are present. Applicants receive more points if more risk factors are present.

Existing road has transit running on or across it with 1+ transit stops in the project area (If flag-stop route with no fixed stops, then 1+ locations in the project area where roadside stops are allowed. Do not count portions of transit routes with no stops, such as non-stop freeway sections of express or limited-stop routes.)
Existing road has high-frequency transit running on or across it and 1+ high-
frequency stops in the project area (high-frequency defined as service at least
every 15 minutes from 6am to 7pm weekdays and 9am to 6pm Saturdays.)
Existing road is within 500? of $\mathbf{1 +}$ shopping, dining, or entertainment destinations Yes (e.g., grocery store, restaurant)

If checked, please describe:
More than a dozen quick service and sit-down dining establishments line Hwy 61 with other commercial destinations in between creating a high demand environment for bike and pedestrian mobility. The area between TH 55 and 18th Street is the most dense for pedestrian/bike crashes in the corridor and features amenities such as ice cream shops, convenience stores, pharmacies, and specialty shops.

Hastings historic downtown is immediately adjacent to the Hwy 61 corridor as well, and with the residential neighborhood to its southwest, Hwy 61 is a natural corridor for pedestrian movement to and from downtown.

Existing road is within 500 feet of other known pedestrian generators (e.g., school, civic/community center, senior housing, multifamily housing, regulatorilydesignated affordable housing)
(Limit 1,400 characters; approximately 200 words)
Existing road is within 500 ? of other known pedestrian generators (e.g., school, civic/community center, senior housing, multifamily housing, regulatorily- Yes designated affordable housing)
If checked, please describe:
The most notable pedestrian generator in the corridor is Todd Field, which is the location for Hastings School District football and soccer events. Located at the intersection of TH 55 and Hwy 61, this is a high traffic area with narrow sidewalks. Parking for events here is dispersed through the community; along with local restaurants and housing complexes on the opposite side of Hwy 61, pedestrian crossings of Hwy 61 are common.

At the intersection of Hwy 61 and 316 is the Hastings Civic Arena (HCA). As an event center for hockey games and associated recreational activities, this destination is directly affected by operations at the intersection. The previous Hwy 316 project (finished in 2021) provided pedestrian and traffic calming features for one major leg of the intersection. This proposed project will finish off needed changes including a roundabout at Hwy 61 and 316 and close existing gaps in the trail network leading to HCA.

Several churches are located along the corridor as well, serving as valuable community resources for the neighborhoods immediately adjacent as well as attendees from beyond, who often depend on on-street parking (and crossing Hwy 61) to access the church facilities.

## Measure A: Multimodal Elements and Existing Connections

Response:

This project will improve safety and mobility for all users by reducing conflicts, providing new facilities, and incorporating elements to reduce vehicle speeds.

Transit service is currently limited to The Loop and Transit Link, dial-a-ride transportation services. The Loop has one set stop along Highway 61. Future transit plans include the Red Rock Corridor, which connects Hastings to St. Paul. Existing and future transit use on Highway 61 will benefit from improved traffic flow and a safer corridor. Eliminating on-street parking and adding a boulevard will provide space for future transit stops along Highway 61 if needed.

The City?s prior work for pedestrian and bicycle travel includes the People Movement Plan (2021), the Hastings 2040 Comprehensive Plan, and the Vermillion St. Corridor Plan (2018). The Highway 61 Improvements build on that work by providing multiuse trails on both sides from the Vermillion River bridge south to 36th Street. These trails will connect this area with the Vermillion River Greenway Regional Trail and the Mississippi River Trail Bikeway (USBR 45). This work will also indirectly connect to the Point Douglas Regional Trail. The Mississippi River Trail Bikeway and Point Douglas Regional Trail are Regional Bicycle Transportation Network Tier 2 alignments.

Consistent with other plans, bicycle travel between the Vermillion River bridge north to 4th Street will occur on other adjacent corridors with less vehicle traffic. While not providing multiuse trails in this area, Highway 61 will improve the sidewalks on both sides of the road. The proposed plan will provide a consistent sidewalk width for the entire length along with a boulevard to separate pedestrian movements from the vehicle travel lanes. Equally important, the plan provides multiple safe crossings of Highway 61 via existing and proposed traffic signals, two new roundabout controlled intersections, and several identified locations for mid-block crossings. The mid-block crossings will have a wide refuge median to allow people to cross one direction of travel at a time. Vehicle conflicts will be reduced by eliminating multiple driveways and reducing full access movements for others.

A detailed field review of the corridor identified multiple concerns regarding ADA compliance. The entire highway will be updated for full compliance, consistent with MnDOT, Dakota County, and the City of Hastings ADA transition plans.

Elements are proposed to slow vehicle speeds along the corridor, such as striping narrow vehicle lanes, narrowing the corridor through eliminating on-street parking and a wider median in select areas, reducing the corner radius where possible, and adding an outside truck apron for highway-to-highway or highway-tocounty road right turn movements.

## Transit Projects Not Requiring Construction

If the applicant is completing a transit application that is operations only, check the box and do not complete the remainder of the form. These projects will receive full points for the Risk Assessment.

Park-and-Ride and other transit construction projects require completion of the Risk Assessment below.
Check Here if Your Transit Project Does Not Require Construction

## Measure A: Risk Assessment - Construction Projects

## 1. Public Involvement ( 20 Percent of Points)

Projects that have been through a public process with residents and other interested public entities are more likely than others to be successful. The project applicant must indicate that events and/or targeted outreach (e.g., surveys and other web-based input) were held to help identify the transportation problem, howthe potential solution was selected instead of other options, and the public involvement completed to date on the project. The focus of this section is on the opportunity for public input as opposed to the quality of input. NOTE: A written response is required and failure to respond will result in zero points.

Multiple types of targeted outreach efforts (such as meetings or online/mail outreach) specific to this project with the general public and partner agencies have been used to help identify the project need.

100\%
At least one meeting specific to this project with the general public has been used to help identify the project need.

50\%
At least online/mail outreach effort specific to this project with the general public has been used to help identify the project need.

50\%
No meeting or outreach specific to this project was conducted, but the project was identified through meetings and/or outreach related to a larger planning effort.

25\%
No outreach has led to the selection of this project.
0\%
Describe the type(s) of outreach selected for this project (i.e., online or in-person meetings, surveys, demonstration projects), the method(s) used to announce outreach opportunities, and how many people participated. Include any public website links to outreach opportunities.

Response:
An extensive, multi-step engagement process was used as part of study development: Phase one engagement presented existing conditions, including past studies, and gathered the community?s issues and experiences traveling along or across Hwy 61. Phase two engagement presented what we heard in phase one and gathered feedback on evaluation criteria and design concepts developed using data from phase one. Phase three presented what we heard in phase two and a proposed vision for the corridor for community feedback.

Engagement activities included three public meetings, a pop-up at community event, door-knocking businesses, and online engagement activities. The community was made aware of engagement opportunities by social media, fliering, multiple postcard mailers, and notice in the Hastings newsletter mailed to every property. Through the three rounds of engagement, more than 200 people attended in-person public meetings and more than 350 surveys or online map comments were received including 454 comments on the design concepts during phase two.

Community members were also able to obtain project updates through multiple presentations given to the Hastings City Council, the City Council commemorated the conclusion of the study with a resolution of support in June 2023.

All businesses along the Hwy 61 corridor were contacted and given the opportunity to provide input directly to the project team. In addition to meetings with individual businesses, the MnDOT/City of Hastings project team also met with the Chamber of Commerce and Downtown Business Association

The project has an active web site that is currently maintained by MnDOT: https://www.dot.state.mn.us/metro/projects/hwy61hastings/

## Limit 2,800 characters; approximately 400 words)

## 2. Layout ( $\mathbf{2 5}$ Percent of Points)

Layout includes proposed geometrics and existing and proposed right-of-way boundaries. A basic layout should include a base map (north arrow, scale; legend;* city and/or county limits; existing ROW, labeled; existing signals;* and bridge numbers*) and design data (proposed alignments; bike and/or roadway lane widths; shoulder width;* proposed signals;* and proposed ROW). An aerial photograph with a line showing the project?s termini does not suffice and will be awarded zero points. *If applicable
Layout approved by the applicant and all impacted jurisdictions (i.e.,
cities/counties/MnDOT. If a MnDOT trunk highway is impacted, approval by MnDOT
must have occurred to receive full points. A PDF of the layout must be attached along with letters from each jurisdiction to receive points.
100\%
A layout does not apply (signal replacement/signal timing, stand-alone
streetscaping, minor intersection improvements). Applicants that are not certain whether a layout is required should contact Colleen Brown at MnDOT Metro State Aid ? colleen.brown@state.mn.us.

100\%

For projects where MnDOT trunk highways are impacted and a MnDOT Staff Approved layout is required. Layout approved by the applicant and all impacted local jurisdictions (i.e., cities/counties), and layout review and approval by MnDOT is pending. A PDF of the layout must be attached along with letters from each jurisdiction to receive points. 75\%

Layout completed but not approved by all jurisdictions. A PDF of the layout must be attached to receive points. 50\%

Layout has been started but is not complete. A PDF of the layout must be attached to receive points.
25\%
Layout has not been started
0\%
Attach Layout
1702265450022 Appendix A Project Layout.pdf
Please upload attachment in PDF form
Additional Attachments
Please upload attachment in PDF form
3. Review of Section 106 Historic Resources (15 Percent of Points)

No known historic properties eligible for or listed in the National Register of Historic Places are located in the project area, and project is not located on an dentified historic bridge

100\%
There are historical/archeological properties present but determination of ?no historic properties affected? is anticipated.

100\%
Historic/archeological property impacted; determination of ?no adverse effect? anticipated

80\%
Historic/archeological property impacted; determination of ?adverse effect? anticipated

40\%
Unsure if there are any historic/archaeological properties in the project area.
0\%
Project is located on an identified historic bridge
4. Right-of-Way ( 25 Percent of Points)

Right-of-way, permanent or temporary easements, and MnDOT agreement/limited-use permit either not required or all have been acquired 100\%

Right-of-way, permanent or temporary easements, and/or MnDOT agreement/limited-use permit required - plat, legal descriptions, or official map complete
50\%
Right-of-way, permanent or temporary easements, and/or MnDOT agreement/limited-use permit required - parcels identified
25\%
Right-of-way, permanent or temporary easements, and/or MnDOT agreement/limited-use permit required - parcels not all identified 0\%
5. Railroad Involvement (15 Percent of Points)

No railroad involvement on project or railroad Right-of-Way agreement is executed (include signature page, if applicable)
100\%
Signature Page
Please upload attachment in PDF form
Railroad Right-of-Way Agreement required; negotiations have begun
50\%
Railroad Right-of-Way Agreement required; negotiations have not begun.
0\%

## Measure A: Cost Effectiveness

Total Project Cost (entered in Project Cost Form):
\$21,408,861.00
Enter Amount of the Noise Walls:
Total Project Cost subtract the amount of the noise walls:
Enter amount of any outside, competitive funding:

## Other Attachments

File Name
01 MetCouncilMaps-ALL.pdf
02a Synchro Outputs part 1.pdf
02b Synchro Outputs part 2.pdf
03 HSIP Benefit Cost Worksheet-Regional Solicitation.pdf
04a CMFs for Regional Solicitation part 1.pdf
04b CMFs for Regional Solicitation part 2.pdf
05 Hwy61 Project Summary.pdf
06 Hwy 61 Layout Sheets All.pdf
07 MnDOT Hwy 61 Letter of Support.pdf
08 Prairie Island letter of support.pdf

| Description | File Size |
| :--- | :--- |
| Make a Map forms | 2.1 MB |
| Congestion and Emissions Data | 333 KB |
| Congestion and Emissions Data | 91 KB |
| Benefit Cost worksheet | 267 KB |
| Crash Modification Factors | 133 KB |
| Crash Modification Factors | 38 KB |
| Project Information and Pictures | 1.1 MB |
| Project Improvement Layouts | 4.1 MB |
| MnDOT Letter of Support | 208 KB |
| Prairie Island Letter of Support | 604 KB |




## Socio-Economic Conditions

Total of publicly subsidized rental
housing units in census
tracts within $1 / 2$ mile: 255
Project located in census tracts that are BELOW the regional average for population in poverty or population of color.

$\square$
$\square$
Area of Concentrated Poverty
Regional Environmental Justice Area

For complete disclaimer of accuracy, please visit http://giswebsite.metc.state.mn.us/gissite/notice.aspx

| Hastings Highway 61 Modernization Project |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Congestion Reduction Calculation Table |  |  |  |  |  |
|  | TH 316 | CSAH 47 | TH 55 | 10th St | TOTAL |
| Peak Hour Delay/Vehicle without the Project (seconds/vehicle) | 47 | 12 | 19 | 11 | $\mathbf{8 9}$ |
| Peak Hour Delay/Vehicle with the Project (seconds/vehicle) | 6 | 9 | 13 | 12 | $\mathbf{4 0}$ |
| Peak Hour Delay/Vehicle Reduced by the Project (seconds/vehicle) | 41 | 3 | 6 | -1 | $\mathbf{4 9}$ |
| Volume (vehicles/hour) | 2302 | 2914 | 3141 | 3049 | $\mathbf{1 1 4 0 6}$ |
| Total Peak Hour Delay Reduced by the Project (seconds) | 94382 | 8742 | 18846 | -3049 | $\mathbf{1 1 8 9 2 1}$ |


| Emissions Reduction Calculation Table |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Total Peak Hour Emissions without the Project (kg) | 8.05 | 5.82 | 4.37 | 4.31 | $\mathbf{2 2 . 5 5}$ |
| Total Peak Hour Emissions with the Project (kg) | 4.96 | 5.62 | 3.81 | 4.6 | $\mathbf{1 8 . 9 9}$ |
| Total Peak Hour Emissions Reduced by the Project (kg) | 3.09 | 0.2 | 0.56 | -0.29 | 3.56 |



|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

[^1]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\emptyset 8$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases |  | 4 | 2 |  |  |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 |  | 1.0 |
| Minimum Split (s) | 20.0 | 20.0 | 10.0 | 21.0 | 27.5 |  | 5.0 |
| Total Split (s) | 24.0 | 24.0 | 18.0 | 71.0 | 53.0 |  | 5.0 |
| Total Split (\%) | 24.0\% | 24.0\% | 18.0\% | 71.0\% | 53.0\% |  | 5\% |
| Maximum Green (s) | 18.0 | 18.0 | 13.0 | 65.5 | 47.5 |  | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 |  | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 |  | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 |  |  |
| Lead/Lag | Lag | Lag | Lag |  | Lead |  | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 |  | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 |  | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max |  | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 16.8 | 16.8 | 71.2 | 70.7 | 52.7 |  |  |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.71 | 0.71 | 0.53 |  |  |
| v/c Ratio | 0.72 | 0.58 | 0.55 | 0.40 | 0.71 |  |  |
| Control Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| LOS | D | A | C | A | A |  |  |
| Approach Delay | 28.0 |  |  | 10.5 | 7.2 |  |  |
| Approach LOS | C |  |  | B | A |  |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 42 (42\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 70
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.72
Intersection Signal Delay: 12.0
Intersection LOS: B
Intersection Capacity Utilization 71.7\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47


|  | 4 |  |  | 7 |  |  | 4 | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ | \% | ${ }^{7}$ | $\hat{}$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | 个 $\uparrow$ | F |
| Traffic Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Future Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 400 |  | 0 | 0 |  | 0 | 150 |  | 0 | 100 |  | 25 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 50 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt |  |  | 0.850 |  | 0.942 |  |  | 0.995 |  |  |  | 0.850 |
| Flt Protected | 0.950 | 0.961 |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 1752 | 3491 | 0 | 1805 | 3471 | 1599 |
| FIt Permitted | 0.950 | 0.961 |  | 0.950 |  |  | 0.218 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 402 | 3491 | 0 | 1805 | 3471 | 1599 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 357 |  | 17 |  |  | 4 |  |  |  | 164 |
| Link Speed (mph) |  | 35 |  |  | 15 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 874 |  |  | 846 |  |  | 1453 |  |  | 382 |  |
| Travel Time (s) |  | 17.0 |  |  | 38.5 |  |  | 28.3 |  |  | 7.4 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 4\% | 0\% | 1\% | 0\% | 0\% | 0\% | 3\% | 3\% | 0\% | 0\% | 4\% | 1\% |
| Adj. Flow (vph) | 371 | 38 | 357 | 23 | 27 | 17 | 232 | 873 | 29 | 6 | 913 | 359 |
| Shared Lane Traffic (\%) | 45\% |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 204 | 205 | 357 | 23 | 44 | 0 | 232 | 902 | 0 | , | 913 | 359 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | LNA | Right | L NA | Left | Right | Left | Left | LNA |
| Median Width(t) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 2 | 2 | 1 | 1 | 1 |  | 1 | 1 |  | 1 | 1 | 0 |
| Detector Template |  |  | Right |  |  |  |  |  |  |  |  |  |
| Leading Detector (tt) | 103 | 103 | 20 | 45 | 45 |  | 50 | 117 |  | 65 | 126 | 0 |
| Trailing Detector (ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 40 | 40 |  | 50 | 6 |  | 50 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) | 97 | 97 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Size(ft) | 6 | 6 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  |  |  |  |  |  |  |  |  |  |
| Turn Type | Split | NA | Perm | Split | NA |  | pm+pt | NA |  | Prot | NA | Perm |
| Protected Phases | 4 | 4 |  | 3 | 3 |  | 5 | 2 |  | 1 | 6 |  |


|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ |  | ( | 1 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  | 4 |  |  |  | 2 |  |  |  |  | 6 |
| Detector Phase | 4 | 4 | 4 | 3 | 3 |  | 52 | 2 |  | 16 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 7.0 | 5.0 | 5.0 |  | 7.0 | 15.0 |  | 7.0 | 15.0 | 15.0 |
| Minimum Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 12.5 | 34.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 19.0 | 47.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (\%) | 26.0\% | 26.0\% | 26.0\% | 14.0\% | 14.0\% |  | 19.0\% | 47.5\% |  | 12.5\% | 41.0\% | 41.0\% |
| Maximum Green (s) | 20.0 | 20.0 | 20.0 | 8.0 | 8.0 |  | 13.5 | 41.5 |  | 7.0 | 35.0 | 35.0 |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 3.5 | 3.5 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |  | 2.5 | 2.0 |  | 2.5 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |  | 5.5 | 6.0 |  | 5.5 | 6.0 | 6.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  | Lag | Lag |  | Lead | Lead | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |  | 0.2 | 2.5 |  | 0.2 | 2.5 | 2.5 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Recall Mode | None | None | None | None | None |  | None | C-Max |  | None | C-Max | C-Max |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |  |  | 9.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) | 19.0 | 19.0 | 19.0 | 22.0 | 22.0 |  |  | 19.0 |  |  | 21.0 | 21.0 |
| Pedestrian Calls (\#/hr) | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 17.2 | 17.2 | 17.2 | 6.7 | 6.7 |  | 58.3 | 57.8 |  | 7.0 | 41.3 | 41.3 |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.17 | 0.07 | 0.07 |  | 0.58 | 0.58 |  | 0.07 | 0.41 | 0.41 |
| v/c Ratio | 0.72 | 0.71 | 0.63 | 0.19 | 0.32 |  | 0.56 | 0.45 |  | 0.05 | 0.64 | 0.47 |
| Control Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 18.8 | 6.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.3 | 0.3 |
| Total Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 19.1 | 7.1 |
| LOS | D | D | A | D | D |  | B | A |  | D | B | A |
| Approach Delay |  | 32.5 |  |  | 40.6 |  |  | 11.1 |  |  | 15.9 |  |
| Approach LOS |  | C |  |  | D |  |  | B |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 86 (86\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 95 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.72 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 18.7 |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 68.6\% |  |  |  | ICU Level of Service C |  |  |  |  |  |  |  |  |

Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55/Walgreens Ent


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  |  | $\uparrow$ | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ |  |  |
| Traffic Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Future Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 25 |  | 0 | 0 |  | 250 | 95 |  | 170 | 280 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 1 | 1 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.992 |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected | 0.950 |  |  |  | 0.972 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1805 | 1833 | 0 | 0 | 1785 | 1599 | 1805 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted | 0.550 |  |  |  | 0.800 |  | 0.210 |  |  | 0.166 |  |  |
| Satd. Flow (perm) | 1045 | 1833 | 0 | 0 | 1469 | 1599 | 399 | 3471 | 1615 | 315 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 2 |  |  |  | 159 |  |  | 142 |  | 2 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 747 |  |  | 1138 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 17.0 |  |  | 25.9 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 32 | 38 | 2 | 97 | 72 | 209 | 7 | 1159 | 94 | 168 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 32 | 40 | 0 | 0 | 169 | 209 | 7 | 1159 | 94 | 168 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 1 | 0 | 1 | 1 |  |
| Detector Template | Left |  |  | Left |  |  |  |  |  |  |  |  |
| Leading Detector (ft) | 20 | 106 |  | 20 | 126 | 25 | 55 | 126 | 0 | 55 | 126 |  |
| Trailing Detector (ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Position(ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Size(ft) | 20 | 20 |  | 20 | 20 | 20 | 50 | 6 | 20 | 50 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  | 100 |  |  | 120 |  |  |  |  |  |  |  |
| Detector 2 Size(ft) |  | 6 |  |  | 6 |  |  |  |  |  |  |  |
| Detector 2 Type |  | Cl+Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  | 0.0 |  |  | 0.0 |  |  |  |  |  |  |  |
| Turn Type | Perm | NA |  | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  | 5 | 2 |  | 1 | 6 |  |


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 | 8 | 52 | 2 | 2 | 16 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 31.0 | 31.0 | 10.0 | 23.0 |  |
| Total Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 54.0 | 54.0 | 16.0 | 60.0 |  |
| Total Split (\%) | 30.0\% | 30.0\% |  | 30.0\% | 30.0\% | 30.0\% | 10.0\% | 54.0\% | 54.0\% | 16.0\% | 60.0\% |  |
| Maximum Green (s) | 24.0 | 24.0 |  | 24.0 | 24.0 | 24.0 | 5.0 | 48.0 | 48.0 | 11.0 | 54.0 |  |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.0 | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) | 2.5 | 2.5 |  | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 |  |  | 6.0 | 6.0 | 5.0 | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  | Lead | Lag | Lag | Lead | Lag |  |
| Lead-Lag Optimize? |  |  |  |  |  |  | Yes | Yes | Yes | Yes | Yes |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 2.0 | 3.5 | 3.5 | 2.0 | 3.5 |  |
| Minimum Gap (s) | 0.2 | 0.2 |  | 0.2 | 0.2 | 0.2 | 0.2 | 2.5 | 2.5 | 0.2 | 2.5 |  |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Time To Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Recall Mode | None | None |  | None | None | None | None | C-Max | C-Max | None | C-Max |  |
| Walk Time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 |  | 7.0 | 7.0 |  | 7.0 |  |
| Flash Dont Walk (s) | 23.0 | 23.0 |  | 20.0 | 20.0 | 20.0 |  | 18.0 | 18.0 |  | 10.0 |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 |  | 0 |  |
| Act Effct Green (s) | 16.9 | 16.9 |  |  | 16.9 | 16.9 | 64.5 | 58.5 | 58.5 | 72.1 | 69.1 |  |
| Actuated g/C Ratio | 0.17 | 0.17 |  |  | 0.17 | 0.17 | 0.64 | 0.58 | 0.58 | 0.72 | 0.69 |  |
| v/c Ratio | 0.18 | 0.13 |  |  | 0.68 | 0.52 | 0.02 | 0.57 | 0.09 | 0.49 | 0.50 |  |
| Control Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.7 | 1.4 | 11.6 | 4.9 |  |
| Queue Delay | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.9 | 1.4 | 11.6 | 4.9 |  |
| LOS | D | C |  |  | D | B | A | B | A | B | A |  |
| Approach Delay |  | 33.7 |  |  | 31.4 |  |  | 10.1 |  |  | 5.7 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 80 ( $80 \%$ ), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 11.3
Intersection LOS: B
Intersection Capacity Utilization 69.3\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 109: TH 61 \& 10th St


Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing No Build

| Intersection |  |  |  |
| :--- | ---: | ---: | ---: |
| Intersection Delay, s/veh | 6.4 |  |  |
| Intersection LOS | A |  | SB |
| Approach | WB | 1 | 2 |
| Entry Lanes | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 369 | 1213 |
| Adj Approach Flow, veh/h | 791 | 373 | 1254 |
| Demand Flow Rate, veh/h | 816 | 770 | 8 |
| Vehicles Circulating, veh/h | 344 | 492 | 344 |
| Vehicles Exiting, veh/h | 799 | 0 | 0 |
| Ped Vol Crossing Leg, \#/h | 0 | 1.000 | 7.4 |
| Ped Cap Adj | 1.000 | 16.8 | A |


| Lane | Left | Bypass | Left | Left | Right |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Designated Moves | L | R | TR | L | TR |
| Assumed Moves | L | R | TR | L | TR |
| RT Channelized |  | Free |  |  |  |
| Lane Util | 1.000 |  | 1.000 | 0.614 | 0.386 |
| Follow-Up Headway, s | 2.609 |  | 2.609 | 2.535 | 2.535 |
| Critical Headway, s | 4.976 | 808 | 4.976 | 4.544 | 4.544 |
| Entry Flow, veh/h | 8 | 1957 | 373 | 770 | 484 |
| Cap Entry Lane, veh/h | 972 | 0.971 | 629 | 1410 | 1410 |
| Entry HV Adj Factor | 0.875 | 784 | 0.988 | 0.971 | 0.962 |
| Flow Entry, veh/h | 7 | 1900 | 369 | 748 | 465 |
| Cap Entry, veh/h | 850 | 0.413 | 622 | 1370 | 1356 |
| V/C Ratio | 0.008 | 0.0 | 0.593 | 0.546 | 0.343 |
| Control Delay, s/veh | 4.3 | A | 16.8 | 8.5 | 5.8 |
| LOS | A | 2 | C | A | A |
| 95th \%tile Queue, veh | 0 |  | 4 | 3 | 2 |



[^2]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | Ø8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 7.0 | 1.0 |
| Minimum Split (s) | 13.0 | 13.0 | 10.0 | 21.0 | 27.5 | 13.0 | 5.0 |
| Total Split (s) | 25.0 | 25.0 | 18.0 | 70.0 | 52.0 | 25.0 | 5.0 |
| Total Split (\%) | $25.0 \%$ | $25.0 \%$ | $18.0 \%$ | $70.0 \%$ | $52.0 \%$ | $25.0 \%$ | $5 \%$ |
| Maximum Green (s) | 19.0 | 19.0 | 13.0 | 64.5 | 46.5 | 19.0 | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 | 4.0 | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 | 2.0 | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 | 6.0 |  |
| Lead/Lag | Lead | Lead | Lag |  | Lead | Lead | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 | 3.0 | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 | 0.2 | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max | None | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 14.1 | 14.1 | 73.9 | 73.4 | 55.4 | 73.8 |  |
| Actuated g/C Ratio | 0.14 | 0.14 | 0.74 | 0.73 | 0.55 | 0.74 |  |
| v/c Ratio | 0.44 | 0.62 | 0.44 | 0.38 | 0.55 | 0.18 |  |
| Control Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| LOS | D | B | B | A | A | A |  |
| Approach Delay | 23.6 |  |  | 7.7 | 5.3 |  |  |
| Approach LOS | C |  |  | A | A |  |  |
|  |  |  |  |  |  |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 8 (8\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 9.3 Intersection LOS: A

Intersection Capacity Utilization 59.4\% ICU Level of Service B
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\varnothing 5$ | $\varnothing 9$ |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 15.0 | 15.0 | 15.0 | 7.0 | 7.0 |
| Minimum Split (s) | 26.0 | 26.0 |  | 34.5 | 41.0 | 41.0 | 12.5 | 12.5 |
| Total Split (s) | 26.0 | 26.0 |  | 74.0 | 43.3 | 43.3 | 18.2 | 12.5 |
| Total Split (\%) | $26.0 \%$ | $26.0 \%$ |  | $74.0 \%$ | $43.3 \%$ | $43.3 \%$ | $18 \%$ | $13 \%$ |
| Maximum Green (s) | 20.0 | 20.0 |  | 68.0 | 37.3 | 37.3 | 12.7 | 7.0 |
| Yellow Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |  |  |
| Lead/Lag |  |  |  |  | Lag | Lag |  | Lead |
| Lead-Lag Optimize? |  |  |  |  | Yes | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| Minimum Gap (s) | 0.2 | 0.2 |  | 2.5 | 2.5 | 2.5 | 0.2 | 3.0 |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Recall Mode | None | None |  | C-Max | C-Max | C-Max | None | None |
| Walk Time (s) | 7.0 | 7.0 |  | 9.0 | 7.0 | 7.0 |  |  |
| Flash Dont Walk (s) | 19.0 | 19.0 |  | 19.0 | 21.0 | 21.0 |  |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  |  |
| Act Effct Green (s) | 18.9 | 18.9 | 69.6 | 69.1 | 50.9 | 50.9 |  |  |
| Actuated g/C Ratio | 0.19 | 0.19 | 0.70 | 0.69 | 0.51 | 0.51 |  |  |
| v/c Ratio | 0.75 | 0.61 | 0.51 | 0.36 | 0.53 | 0.40 |  |  |
| Control Delay | 46.5 | 8.6 | 12.2 | 4.3 | 10.2 | 1.4 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.3 |  |  |
| Total Delay | 46.5 | 8.6 | 12.2 | 4.4 | 10.5 | 1.7 |  |  |
| LOS | D | A | B | A | B | A |  |  |
| Approach Delay | 30.2 |  |  | 6.1 | 7.9 |  |  |  |
| Approach LOS | C |  |  |  | A | A |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 48 (48\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 95
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.75
Intersection Signal Delay: 13.0
Intersection Capacity Utilization 65.2\%
Intersection LOS: B
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  |  |  |  | $\uparrow$ | 「 |  | 44 | 「 | 1 | 44 |  |
| Traffic Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Future Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 250 | 0 |  | 150 | 225 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 1 | 0 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  |  |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected |  |  |  |  | 0.968 |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted |  |  |  |  | 0.968 |  |  |  |  | 0.152 |  |  |
| Satd. Flow (perm) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 289 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  | 226 |  |  | 133 |  | 3 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 713 |  |  | 848 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 16.2 |  |  | 19.3 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 0 | 0 | 0 | 147 | 72 | 226 | 0 | 1192 | 169 | 174 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 219 | 226 | 0 | 1192 | 169 | 174 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors |  |  |  | 1 | 2 | 1 |  | 2 | 1 | 1 | 2 |  |
| Detector Template |  |  |  | Left | Thru | Right |  | Thru | Right | Left | Thru |  |
| Leading Detector (ft) |  |  |  | 20 | 100 | 20 |  | 100 | 20 | 20 | 100 |  |
| Trailing Detector (ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Position(ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Size(ft) |  |  |  | 20 | 6 | 20 |  | 6 | 20 | 20 | 6 |  |
| Detector 1 Type |  |  |  | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  |  |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  |  |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  |  |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  |  |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type |  |  |  | Split | NA | Perm |  | NA | Perm | pm+pt | NA |  |
| Protected Phases |  |  |  | 8 | 8 |  |  | 2 |  | 1 | 6 |  |


|  | 4 |  |  |  |  |  |  | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  |  |  |  | 8 |  |  | 2 | 6 |  |  |
| Detector Phase |  |  |  | 8 | 8 | 8 |  | 2 | 2 | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  |  |  | 6.5 | 6.5 | 6.5 |  | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) |  |  |  | 12.5 | 12.5 | 12.5 |  | 22.0 | 22.0 | 10.0 | 22.0 |  |
| Total Split (s) |  |  |  | 29.0 | 29.0 | 29.0 |  | 55.0 | 55.0 | 16.0 | 71.0 |  |
| Total Split (\%) |  |  |  | 29.0\% | 29.0\% | 29.0\% |  | 55.0\% | 55.0\% | 16.0\% | 71.0\% |  |
| Maximum Green (s) |  |  |  | 23.0 | 23.0 | 23.0 |  | 49.0 | 49.0 | 11.0 | 65.0 |  |
| Yellow Time (s) |  |  |  | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) |  |  |  | 2.5 | 2.5 | 2.5 |  | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) |  |  |  |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  |  | Lag | Lag | Lead |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  | Yes | Yes | Yes |  |  |
| Vehicle Extension (s) |  |  |  | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 |  |
| Recall Mode |  |  |  | None | None | None |  | C-Max | C-Max | None | C-Max |  |
| Act Effict Green (s) |  |  |  |  | 17.6 | 17.6 |  | 56.8 | 56.8 | 71.4 | 70.4 |  |
| Actuated g/C Ratio |  |  |  |  | 0.18 | 0.18 |  | 0.57 | 0.57 | 0.71 | 0.70 |  |
| v/c Ratio |  |  |  |  | 0.71 | 0.48 |  | 0.61 | 0.17 | 0.52 | 0.49 |  |
| Control Delay |  |  |  |  | 50.7 | 8.2 |  | 14.7 | 4.4 | 10.0 | 4.7 |  |
| Queue Delay |  |  |  |  | 0.0 | 0.0 |  | 0.5 | 0.0 | 0.0 | 0.0 |  |
| Total Delay |  |  |  |  | 50.7 | 8.2 |  | 15.3 | 4.4 | 10.0 | 4.7 |  |
| LOS |  |  |  |  | D | A |  | B | A | B | A |  |
| Approach Delay |  |  |  |  | 29.1 |  |  | 13.9 |  |  | 5.4 |  |
| Approach LOS |  |  |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 58 (58\%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 12.4
Intersection LOS: B
Intersection Capacity Utilization 66.5\%
ICU Level of Service C
Analysis Period (min) 15


5: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 47 |
| CO Emissions $(\mathrm{kg})$ | 5.64 |
| NOx Emissions $(\mathrm{kg})$ | 1.10 |
| VOC Emissions (kg) | 1.31 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 4.08 |
| NOx Emissions $(\mathrm{kg})$ | 0.79 |
| VOC Emissions $(\mathrm{kg})$ | 0.95 |

108: TH 61 \& TH 55/Walgreens Ent

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3116 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 19 |
| CO Emissions $(\mathrm{kg}$ | 3.06 |
| NOx Emissions kg$)$ | 0.60 |
| VOC Emissions (kg) | 0.71 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2952 |
| Total Delay / veh (s/v) | 11 |
| CO Emissions $(\mathrm{kg})$ | 3.02 |
| NOx Emissions kg$)$ | 0.59 |
| VOC Emissions $(\mathrm{kg})$ | 0.70 |

9: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 0 |
| CO Emissions $(\mathrm{kg})$ | 3.47 |
| NOx Emissions $(\mathrm{kg})$ | 0.68 |
| VOC Emissions (kg) | 0.81 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 9 |
| CO Emissions kg ) | 3.94 |
| NOx Emissions $(\mathrm{kg})$ | 0.77 |
| VOC Emissions $(\mathrm{kg})$ | 0.91 |

108: TH 61 \& TH 55

| Direction | All |
| :--- | :---: |
| Future Volume (vph) | 3141 |
| Total Delay / veh (s/v) | 13 |
| CO Emissions $(\mathrm{kg}$ | 2.67 |
| NOx Emissions kg$)$ | 0.52 |
| VOC Emissions $(\mathrm{kg})$ | 0.62 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3049 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 3.22 |
| NOx Emissions $(\mathrm{kg})$ | 0.63 |
| VOC Emissions $(\mathrm{kg})$ | 0.75 |


| Hastings Highway 61 Modernization Project |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Congestion Reduction Calculation Table |  |  |  |  |  |
|  | TH 316 | CSAH 47 | TH 55 | 10th St | TOTAL |
| Peak Hour Delay/Vehicle without the Project (seconds/vehicle) | 47 | 12 | 19 | 11 | $\mathbf{8 9}$ |
| Peak Hour Delay/Vehicle with the Project (seconds/vehicle) | 6 | 9 | 13 | 12 | $\mathbf{4 0}$ |
| Peak Hour Delay/Vehicle Reduced by the Project (seconds/vehicle) | 41 | 3 | 6 | -1 | $\mathbf{4 9}$ |
| Volume (vehicles/hour) | 2302 | 2914 | 3141 | 3049 | $\mathbf{1 1 4 0 6}$ |
| Total Peak Hour Delay Reduced by the Project (seconds) | 94382 | 8742 | 18846 | -3049 | $\mathbf{1 1 8 9 2 1}$ |


| Emissions Reduction Calculation Table |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Total Peak Hour Emissions without the Project (kg) | 8.05 | 5.82 | 4.37 | 4.31 | $\mathbf{2 2 . 5 5}$ |
| Total Peak Hour Emissions with the Project (kg) | 4.96 | 5.62 | 3.81 | 4.6 | $\mathbf{1 8 . 9 9}$ |
| Total Peak Hour Emissions Reduced by the Project (kg) | 3.09 | 0.2 | 0.56 | -0.29 | 3.56 |



|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

[^3]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\emptyset 8$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases |  | 4 | 2 |  |  |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 |  | 1.0 |
| Minimum Split (s) | 20.0 | 20.0 | 10.0 | 21.0 | 27.5 |  | 5.0 |
| Total Split (s) | 24.0 | 24.0 | 18.0 | 71.0 | 53.0 |  | 5.0 |
| Total Split (\%) | 24.0\% | 24.0\% | 18.0\% | 71.0\% | 53.0\% |  | 5\% |
| Maximum Green (s) | 18.0 | 18.0 | 13.0 | 65.5 | 47.5 |  | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 |  | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 |  | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 |  |  |
| Lead/Lag | Lag | Lag | Lag |  | Lead |  | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 |  | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 |  | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max |  | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 16.8 | 16.8 | 71.2 | 70.7 | 52.7 |  |  |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.71 | 0.71 | 0.53 |  |  |
| v/c Ratio | 0.72 | 0.58 | 0.55 | 0.40 | 0.71 |  |  |
| Control Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| LOS | D | A | C | A | A |  |  |
| Approach Delay | 28.0 |  |  | 10.5 | 7.2 |  |  |
| Approach LOS | C |  |  | B | A |  |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 42 (42\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 70
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.72
Intersection Signal Delay: 12.0
Intersection LOS: B
Intersection Capacity Utilization 71.7\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47


|  | 4 |  |  | 7 |  |  | 4 | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ | \% | ${ }^{7}$ | $\hat{}$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | 个 $\uparrow$ | F |
| Traffic Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Future Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 400 |  | 0 | 0 |  | 0 | 150 |  | 0 | 100 |  | 25 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 50 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt |  |  | 0.850 |  | 0.942 |  |  | 0.995 |  |  |  | 0.850 |
| Flt Protected | 0.950 | 0.961 |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 1752 | 3491 | 0 | 1805 | 3471 | 1599 |
| FIt Permitted | 0.950 | 0.961 |  | 0.950 |  |  | 0.218 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 402 | 3491 | 0 | 1805 | 3471 | 1599 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 357 |  | 17 |  |  | 4 |  |  |  | 164 |
| Link Speed (mph) |  | 35 |  |  | 15 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 874 |  |  | 846 |  |  | 1453 |  |  | 382 |  |
| Travel Time (s) |  | 17.0 |  |  | 38.5 |  |  | 28.3 |  |  | 7.4 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 4\% | 0\% | 1\% | 0\% | 0\% | 0\% | 3\% | 3\% | 0\% | 0\% | 4\% | 1\% |
| Adj. Flow (vph) | 371 | 38 | 357 | 23 | 27 | 17 | 232 | 873 | 29 | 6 | 913 | 359 |
| Shared Lane Traffic (\%) | 45\% |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 204 | 205 | 357 | 23 | 44 | 0 | 232 | 902 | 0 | , | 913 | 359 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | LNA | Right | L NA | Left | Right | Left | Left | LNA |
| Median Width(t) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 2 | 2 | 1 | 1 | 1 |  | 1 | 1 |  | 1 | 1 | 0 |
| Detector Template |  |  | Right |  |  |  |  |  |  |  |  |  |
| Leading Detector (tt) | 103 | 103 | 20 | 45 | 45 |  | 50 | 117 |  | 65 | 126 | 0 |
| Trailing Detector (ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 40 | 40 |  | 50 | 6 |  | 50 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) | 97 | 97 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Size(ft) | 6 | 6 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  |  |  |  |  |  |  |  |  |  |
| Turn Type | Split | NA | Perm | Split | NA |  | pm+pt | NA |  | Prot | NA | Perm |
| Protected Phases | 4 | 4 |  | 3 | 3 |  | 5 | 2 |  | 1 | 6 |  |


|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ |  | ( | 1 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  | 4 |  |  |  | 2 |  |  |  |  | 6 |
| Detector Phase | 4 | 4 | 4 | 3 | 3 |  | 52 | 2 |  | 16 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 7.0 | 5.0 | 5.0 |  | 7.0 | 15.0 |  | 7.0 | 15.0 | 15.0 |
| Minimum Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 12.5 | 34.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 19.0 | 47.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (\%) | 26.0\% | 26.0\% | 26.0\% | 14.0\% | 14.0\% |  | 19.0\% | 47.5\% |  | 12.5\% | 41.0\% | 41.0\% |
| Maximum Green (s) | 20.0 | 20.0 | 20.0 | 8.0 | 8.0 |  | 13.5 | 41.5 |  | 7.0 | 35.0 | 35.0 |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 3.5 | 3.5 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |  | 2.5 | 2.0 |  | 2.5 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |  | 5.5 | 6.0 |  | 5.5 | 6.0 | 6.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  | Lag | Lag |  | Lead | Lead | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |  | 0.2 | 2.5 |  | 0.2 | 2.5 | 2.5 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Recall Mode | None | None | None | None | None |  | None | C-Max |  | None | C-Max | C-Max |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |  |  | 9.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) | 19.0 | 19.0 | 19.0 | 22.0 | 22.0 |  |  | 19.0 |  |  | 21.0 | 21.0 |
| Pedestrian Calls (\#/hr) | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 17.2 | 17.2 | 17.2 | 6.7 | 6.7 |  | 58.3 | 57.8 |  | 7.0 | 41.3 | 41.3 |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.17 | 0.07 | 0.07 |  | 0.58 | 0.58 |  | 0.07 | 0.41 | 0.41 |
| v/c Ratio | 0.72 | 0.71 | 0.63 | 0.19 | 0.32 |  | 0.56 | 0.45 |  | 0.05 | 0.64 | 0.47 |
| Control Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 18.8 | 6.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.3 | 0.3 |
| Total Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 19.1 | 7.1 |
| LOS | D | D | A | D | D |  | B | A |  | D | B | A |
| Approach Delay |  | 32.5 |  |  | 40.6 |  |  | 11.1 |  |  | 15.9 |  |
| Approach LOS |  | C |  |  | D |  |  | B |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 86 (86\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 95 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.72 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 18.7 |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 68.6\% |  |  |  | ICU Level of Service C |  |  |  |  |  |  |  |  |

Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55/Walgreens Ent


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  |  | $\uparrow$ | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ |  |  |
| Traffic Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Future Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 25 |  | 0 | 0 |  | 250 | 95 |  | 170 | 280 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 1 | 1 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.992 |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected | 0.950 |  |  |  | 0.972 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1805 | 1833 | 0 | 0 | 1785 | 1599 | 1805 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted | 0.550 |  |  |  | 0.800 |  | 0.210 |  |  | 0.166 |  |  |
| Satd. Flow (perm) | 1045 | 1833 | 0 | 0 | 1469 | 1599 | 399 | 3471 | 1615 | 315 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 2 |  |  |  | 159 |  |  | 142 |  | 2 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 747 |  |  | 1138 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 17.0 |  |  | 25.9 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 32 | 38 | 2 | 97 | 72 | 209 | 7 | 1159 | 94 | 168 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 32 | 40 | 0 | 0 | 169 | 209 | 7 | 1159 | 94 | 168 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 1 | 0 | 1 | 1 |  |
| Detector Template | Left |  |  | Left |  |  |  |  |  |  |  |  |
| Leading Detector (ft) | 20 | 106 |  | 20 | 126 | 25 | 55 | 126 | 0 | 55 | 126 |  |
| Trailing Detector (ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Position(ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Size(ft) | 20 | 20 |  | 20 | 20 | 20 | 50 | 6 | 20 | 50 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  | 100 |  |  | 120 |  |  |  |  |  |  |  |
| Detector 2 Size(ft) |  | 6 |  |  | 6 |  |  |  |  |  |  |  |
| Detector 2 Type |  | Cl+Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  | 0.0 |  |  | 0.0 |  |  |  |  |  |  |  |
| Turn Type | Perm | NA |  | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  | 5 | 2 |  | 1 | 6 |  |


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 | 8 | 52 | 2 | 2 | 16 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 31.0 | 31.0 | 10.0 | 23.0 |  |
| Total Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 54.0 | 54.0 | 16.0 | 60.0 |  |
| Total Split (\%) | 30.0\% | 30.0\% |  | 30.0\% | 30.0\% | 30.0\% | 10.0\% | 54.0\% | 54.0\% | 16.0\% | 60.0\% |  |
| Maximum Green (s) | 24.0 | 24.0 |  | 24.0 | 24.0 | 24.0 | 5.0 | 48.0 | 48.0 | 11.0 | 54.0 |  |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.0 | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) | 2.5 | 2.5 |  | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 |  |  | 6.0 | 6.0 | 5.0 | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  | Lead | Lag | Lag | Lead | Lag |  |
| Lead-Lag Optimize? |  |  |  |  |  |  | Yes | Yes | Yes | Yes | Yes |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 2.0 | 3.5 | 3.5 | 2.0 | 3.5 |  |
| Minimum Gap (s) | 0.2 | 0.2 |  | 0.2 | 0.2 | 0.2 | 0.2 | 2.5 | 2.5 | 0.2 | 2.5 |  |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Time To Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Recall Mode | None | None |  | None | None | None | None | C-Max | C-Max | None | C-Max |  |
| Walk Time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 |  | 7.0 | 7.0 |  | 7.0 |  |
| Flash Dont Walk (s) | 23.0 | 23.0 |  | 20.0 | 20.0 | 20.0 |  | 18.0 | 18.0 |  | 10.0 |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 |  | 0 |  |
| Act Effct Green (s) | 16.9 | 16.9 |  |  | 16.9 | 16.9 | 64.5 | 58.5 | 58.5 | 72.1 | 69.1 |  |
| Actuated g/C Ratio | 0.17 | 0.17 |  |  | 0.17 | 0.17 | 0.64 | 0.58 | 0.58 | 0.72 | 0.69 |  |
| v/c Ratio | 0.18 | 0.13 |  |  | 0.68 | 0.52 | 0.02 | 0.57 | 0.09 | 0.49 | 0.50 |  |
| Control Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.7 | 1.4 | 11.6 | 4.9 |  |
| Queue Delay | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.9 | 1.4 | 11.6 | 4.9 |  |
| LOS | D | C |  |  | D | B | A | B | A | B | A |  |
| Approach Delay |  | 33.7 |  |  | 31.4 |  |  | 10.1 |  |  | 5.7 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 80 ( $80 \%$ ), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 11.3
Intersection LOS: B
Intersection Capacity Utilization 69.3\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 109: TH 61 \& 10th St


Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing No Build

| Intersection |  |  |  |
| :--- | ---: | ---: | ---: |
| Intersection Delay, s/veh | 6.4 |  |  |
| Intersection LOS | A |  | SB |
| Approach | WB | 1 | 2 |
| Entry Lanes | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 369 | 1213 |
| Adj Approach Flow, veh/h | 791 | 373 | 1254 |
| Demand Flow Rate, veh/h | 816 | 770 | 8 |
| Vehicles Circulating, veh/h | 344 | 492 | 344 |
| Vehicles Exiting, veh/h | 799 | 0 | 0 |
| Ped Vol Crossing Leg, \#/h | 0 | 1.000 | 7.4 |
| Ped Cap Adj | 1.000 | 16.8 | A |


| Lane | Left | Bypass | Left | Left | Right |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Designated Moves | L | R | TR | L | TR |
| Assumed Moves | L | R | TR | L | TR |
| RT Channelized |  | Free |  |  |  |
| Lane Util | 1.000 |  | 1.000 | 0.614 | 0.386 |
| Follow-Up Headway, s | 2.609 |  | 2.609 | 2.535 | 2.535 |
| Critical Headway, s | 4.976 | 808 | 4.976 | 4.544 | 4.544 |
| Entry Flow, veh/h | 8 | 1957 | 373 | 770 | 484 |
| Cap Entry Lane, veh/h | 972 | 0.971 | 629 | 1410 | 1410 |
| Entry HV Adj Factor | 0.875 | 784 | 0.988 | 0.971 | 0.962 |
| Flow Entry, veh/h | 7 | 1900 | 369 | 748 | 465 |
| Cap Entry, veh/h | 850 | 0.413 | 622 | 1370 | 1356 |
| V/C Ratio | 0.008 | 0.0 | 0.593 | 0.546 | 0.343 |
| Control Delay, s/veh | 4.3 | A | 16.8 | 8.5 | 5.8 |
| LOS | A | 2 | C | A | A |
| 95th \%tile Queue, veh | 0 |  | 4 | 3 | 2 |



[^4]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | Ø8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 7.0 | 1.0 |
| Minimum Split (s) | 13.0 | 13.0 | 10.0 | 21.0 | 27.5 | 13.0 | 5.0 |
| Total Split (s) | 25.0 | 25.0 | 18.0 | 70.0 | 52.0 | 25.0 | 5.0 |
| Total Split (\%) | $25.0 \%$ | $25.0 \%$ | $18.0 \%$ | $70.0 \%$ | $52.0 \%$ | $25.0 \%$ | $5 \%$ |
| Maximum Green (s) | 19.0 | 19.0 | 13.0 | 64.5 | 46.5 | 19.0 | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 | 4.0 | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 | 2.0 | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 | 6.0 |  |
| Lead/Lag | Lead | Lead | Lag |  | Lead | Lead | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 | 3.0 | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 | 0.2 | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max | None | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 14.1 | 14.1 | 73.9 | 73.4 | 55.4 | 73.8 |  |
| Actuated g/C Ratio | 0.14 | 0.14 | 0.74 | 0.73 | 0.55 | 0.74 |  |
| v/c Ratio | 0.44 | 0.62 | 0.44 | 0.38 | 0.55 | 0.18 |  |
| Control Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| LOS | D | B | B | A | A | A |  |
| Approach Delay | 23.6 |  |  | 7.7 | 5.3 |  |  |
| Approach LOS | C |  |  | A | A |  |  |
|  |  |  |  |  |  |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 8 (8\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 9.3 Intersection LOS: A

Intersection Capacity Utilization 59.4\% ICU Level of Service B
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\varnothing 5$ | $\varnothing 9$ |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 15.0 | 15.0 | 15.0 | 7.0 | 7.0 |
| Minimum Split (s) | 26.0 | 26.0 |  | 34.5 | 41.0 | 41.0 | 12.5 | 12.5 |
| Total Split (s) | 26.0 | 26.0 |  | 74.0 | 43.3 | 43.3 | 18.2 | 12.5 |
| Total Split (\%) | $26.0 \%$ | $26.0 \%$ |  | $74.0 \%$ | $43.3 \%$ | $43.3 \%$ | $18 \%$ | $13 \%$ |
| Maximum Green (s) | 20.0 | 20.0 |  | 68.0 | 37.3 | 37.3 | 12.7 | 7.0 |
| Yellow Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |  |  |
| Lead/Lag |  |  |  |  | Lag | Lag |  | Lead |
| Lead-Lag Optimize? |  |  |  |  | Yes | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| Minimum Gap (s) | 0.2 | 0.2 |  | 2.5 | 2.5 | 2.5 | 0.2 | 3.0 |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Recall Mode | None | None |  | C-Max | C-Max | C-Max | None | None |
| Walk Time (s) | 7.0 | 7.0 |  | 9.0 | 7.0 | 7.0 |  |  |
| Flash Dont Walk (s) | 19.0 | 19.0 |  | 19.0 | 21.0 | 21.0 |  |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  |  |
| Act Effct Green (s) | 18.9 | 18.9 | 69.6 | 69.1 | 50.9 | 50.9 |  |  |
| Actuated g/C Ratio | 0.19 | 0.19 | 0.70 | 0.69 | 0.51 | 0.51 |  |  |
| v/c Ratio | 0.75 | 0.61 | 0.51 | 0.36 | 0.53 | 0.40 |  |  |
| Control Delay | 46.5 | 8.6 | 12.2 | 4.3 | 10.2 | 1.4 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.3 |  |  |
| Total Delay | 46.5 | 8.6 | 12.2 | 4.4 | 10.5 | 1.7 |  |  |
| LOS | D | A | B | A | B | A |  |  |
| Approach Delay | 30.2 |  |  | 6.1 | 7.9 |  |  |  |
| Approach LOS | C |  |  |  | A | A |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 48 (48\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 95
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.75
Intersection Signal Delay: 13.0
Intersection Capacity Utilization 65.2\%
Intersection LOS: B
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  |  |  |  | $\uparrow$ | 「 |  | 44 | 「 | 1 | 44 |  |
| Traffic Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Future Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 250 | 0 |  | 150 | 225 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 1 | 0 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  |  |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected |  |  |  |  | 0.968 |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted |  |  |  |  | 0.968 |  |  |  |  | 0.152 |  |  |
| Satd. Flow (perm) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 289 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  | 226 |  |  | 133 |  | 3 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 713 |  |  | 848 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 16.2 |  |  | 19.3 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 0 | 0 | 0 | 147 | 72 | 226 | 0 | 1192 | 169 | 174 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 219 | 226 | 0 | 1192 | 169 | 174 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors |  |  |  | 1 | 2 | 1 |  | 2 | 1 | 1 | 2 |  |
| Detector Template |  |  |  | Left | Thru | Right |  | Thru | Right | Left | Thru |  |
| Leading Detector (ft) |  |  |  | 20 | 100 | 20 |  | 100 | 20 | 20 | 100 |  |
| Trailing Detector (ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Position(ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Size(ft) |  |  |  | 20 | 6 | 20 |  | 6 | 20 | 20 | 6 |  |
| Detector 1 Type |  |  |  | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  |  |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  |  |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  |  |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  |  |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type |  |  |  | Split | NA | Perm |  | NA | Perm | pm+pt | NA |  |
| Protected Phases |  |  |  | 8 | 8 |  |  | 2 |  | 1 | 6 |  |


|  | 4 |  |  |  |  |  |  | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  |  |  |  | 8 |  |  | 2 | 6 |  |  |
| Detector Phase |  |  |  | 8 | 8 | 8 |  | 2 | 2 | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  |  |  | 6.5 | 6.5 | 6.5 |  | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) |  |  |  | 12.5 | 12.5 | 12.5 |  | 22.0 | 22.0 | 10.0 | 22.0 |  |
| Total Split (s) |  |  |  | 29.0 | 29.0 | 29.0 |  | 55.0 | 55.0 | 16.0 | 71.0 |  |
| Total Split (\%) |  |  |  | 29.0\% | 29.0\% | 29.0\% |  | 55.0\% | 55.0\% | 16.0\% | 71.0\% |  |
| Maximum Green (s) |  |  |  | 23.0 | 23.0 | 23.0 |  | 49.0 | 49.0 | 11.0 | 65.0 |  |
| Yellow Time (s) |  |  |  | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) |  |  |  | 2.5 | 2.5 | 2.5 |  | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) |  |  |  |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  |  | Lag | Lag | Lead |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  | Yes | Yes | Yes |  |  |
| Vehicle Extension (s) |  |  |  | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 |  |
| Recall Mode |  |  |  | None | None | None |  | C-Max | C-Max | None | C-Max |  |
| Act Effict Green (s) |  |  |  |  | 17.6 | 17.6 |  | 56.8 | 56.8 | 71.4 | 70.4 |  |
| Actuated g/C Ratio |  |  |  |  | 0.18 | 0.18 |  | 0.57 | 0.57 | 0.71 | 0.70 |  |
| v/c Ratio |  |  |  |  | 0.71 | 0.48 |  | 0.61 | 0.17 | 0.52 | 0.49 |  |
| Control Delay |  |  |  |  | 50.7 | 8.2 |  | 14.7 | 4.4 | 10.0 | 4.7 |  |
| Queue Delay |  |  |  |  | 0.0 | 0.0 |  | 0.5 | 0.0 | 0.0 | 0.0 |  |
| Total Delay |  |  |  |  | 50.7 | 8.2 |  | 15.3 | 4.4 | 10.0 | 4.7 |  |
| LOS |  |  |  |  | D | A |  | B | A | B | A |  |
| Approach Delay |  |  |  |  | 29.1 |  |  | 13.9 |  |  | 5.4 |  |
| Approach LOS |  |  |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 58 (58\%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 12.4
Intersection LOS: B
Intersection Capacity Utilization 66.5\%
ICU Level of Service C
Analysis Period (min) 15


5: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 47 |
| CO Emissions $(\mathrm{kg})$ | 5.64 |
| NOx Emissions $(\mathrm{kg})$ | 1.10 |
| VOC Emissions (kg) | 1.31 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 4.08 |
| NOx Emissions $(\mathrm{kg})$ | 0.79 |
| VOC Emissions $(\mathrm{kg})$ | 0.95 |

108: TH 61 \& TH 55/Walgreens Ent

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3116 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 19 |
| CO Emissions $(\mathrm{kg}$ | 3.06 |
| NOx Emissions kg$)$ | 0.60 |
| VOC Emissions (kg) | 0.71 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2952 |
| Total Delay / veh (s/v) | 11 |
| CO Emissions $(\mathrm{kg})$ | 3.02 |
| NOx Emissions kg$)$ | 0.59 |
| VOC Emissions $(\mathrm{kg})$ | 0.70 |

9: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 0 |
| CO Emissions $(\mathrm{kg})$ | 3.47 |
| NOx Emissions $(\mathrm{kg})$ | 0.68 |
| VOC Emissions (kg) | 0.81 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 9 |
| CO Emissions kg ) | 3.94 |
| NOx Emissions $(\mathrm{kg})$ | 0.77 |
| VOC Emissions $(\mathrm{kg})$ | 0.91 |

108: TH 61 \& TH 55

| Direction | All |
| :--- | :---: |
| Future Volume (vph) | 3141 |
| Total Delay / veh (s/v) | 13 |
| CO Emissions $(\mathrm{kg}$ | 2.67 |
| NOx Emissions kg$)$ | 0.52 |
| VOC Emissions $(\mathrm{kg})$ | 0.62 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3049 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 3.22 |
| NOx Emissions $(\mathrm{kg})$ | 0.63 |
| VOC Emissions $(\mathrm{kg})$ | 0.75 |



| Intersection | Crashes Eliminated |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\kappa$ | A | B | c | $\bigcirc$ |
| 5 th Street | 0.00 | 0.00 | 1.18 | 0.00 | 2.37 |
| 7 th Street | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 10 th Street | 0.00 | 0.00 | 0.25 | 1.75 | 4.25 |
| Trunk Hwy 55 | 0.00 | 0.00 | 0.25 | 0.75 | 3.00 |
| 12 th Street | 0.00 | 0.00 | 0.00 | 0.45 | 4.95 |
| 144h Street | 0.00 | 0.00 | 0.00 | 0.40 | 1.20 |
| 18th Street | 0.00 | 0.00 | 0.36 | 0.36 | 2.89 |
| County Hwy 47 | 0.00 | 0.00 | 0.04 | 0.04 | 0.64 |
| 21 1ststreet | 0.00 | 0.00 | 0.45 | 0.00 | 3.60 |
| 24 th Street | 0.00 | 0.00 | 0.00 | 0.00 | 0.90 |
| Trunk Hwy 316 | 0.00 | 0.00 | 0.00 | 1.76 | 2.64 |
| Cannon | 0.00 | 0.00 | 1.00 | 0.00 | 2.00 |
| 36th Street | 0.00 | 0.00 | 0.44 | 0.44 | 0.88 |
| Corridor | 0.00 | 0.27 | 0.27 | 0.00 | 0.27 |
| Corridor Total (All intersections and segments) | 0.00 | 0.27 | 4.24 | 5.95 | 29.5 |
| Product of Crashes Observed and CRF (Table 1). |  |  |  |  |  |
| Effective CMF (1Crashes Eliminated/Crashes Observed) |  |  |  |  |  |

CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 9821
CMF Name: Install right-in-right-out (RIRO) operations at stop-controlled inters

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Access management

Study ID: Safety Effects of Turning Movement Restrictions at Stop-Controlled Intersections, Le et al. 2018

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.55 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.09 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 45 |  |
| Adjusted Standard Error: |  |  |
| Unadjusted Standard Error: | 9 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: | 4 |
| Maximum Number of Lanes: | 6 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: | 4 and 6 Lanes |
| Road Division Type: | Divided by Median |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | All |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: | Minimum of 13433 to Maximum of 75000 Annual Average Daily Traffic (AADT) |
| Minor Road Traffic Volume: | Minimum of 51 to Maximum of 2600 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

38724 Annual Average Daily Traffic (AADT)

519 Annual Average Daily Traffic (AADT)

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: | CA |
| Country: | USA |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (crashes): | 483 crashes |
| Sample Size (sites): | 138 sites |

## Other Details

| Included in HSM: | No |
| ---: | :--- | :--- |
| Date Added to Clearinghouse: | Oct 27, 2018 |
| Comments: | This CMF compares urban, three-legged, stop-controlled intersections with <br> RIRO operation to full movement. This CMF looks at Total crashes. Total <br> crashes are defined as all crashes within 100 ft of intersection (all types and <br> severities combined) |

[^5]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 8800
CMF Name: Install raised median with or without marked crosswalk (uncontroll

## Description:

## Prior Condition: No median

## Category: Pedestrians

Study ID: Development of Crash Modification Factors for Uncontrolled Pedestrian Crossing Treatments, Zegeer et al. 2017

|  |  |
| ---: | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.742 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.071 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 25.8 |  |
| Adjusted Standard Error: |  |  |
| Unadjusted Standard Error: | 7.1 |  |


| Applicability |  |
| :---: | :---: |
| Crash Type: | All |
| Crash Severity: | All |
| Roadway Types: | Minor Arterial |
| Minimum Number of Lanes: | 2 |
| Maximum Number of Lanes: | 8 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: | Divided by Median |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban and suburban |
| Traffic Volume: | Minimum of 1245 to Maximum of 46000 Annual Average Daily Traffic (AADT) |
| Average Traffic Volume: |  |
| Time of Day: | All |
|  | If countermeasure is intersection-based. |
| Intersection Type: |  |
| Intersection Geometry: |  |
| Traffic Control: |  |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: | 2004 to 2013 |
| ---: | :--- |
| Municipality: |  |
| State: | AZ,FL,IL,MA,NY,NC,OR,VA,WI |
| Country: | USA |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (crashes): | 10666 crashes |
| Sample Size (site-years): | 5021 site-years |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Nov 17, 2017 |
| Comments: | Study sites were a combination of intersection and mid-block locations. |

[^6]
## CMF / CRF Details

CMF ID: 5233
CMF Name: Presence of three leg intersection vs. four leg intersection

## Description:

## Prior Condition: Four-leg intersection

## Category: Intersection geometry

Study ID: Corridor-level signalized intersection safety analysis in Shanghai, China using Bayesian hierarchal models, Kun Xie, Xuesong Wang, Helai Huang, Xiahong Chen 2013

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.75 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.13 |


|  | Crash Reduction Factor |
| ---: | :---: |
| Value: | 25 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 12.8 |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | Not specified |
|  | If countermeasure is intersection-based. |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Signalized |
| Major Road Traffic Volume: | Minimum of 7700 to Maximum of 140300 Average Daily Traffic (ADT) |
| Minor Road Traffic Volume: |  |

## Average Major Road Volume:

Average Minor Road Volume:

46200 Average Daily Traffic (ADT)
$\square$

## Development Details

| Date Range of Data Used: | 2009 to 2009 |
| ---: | :--- |
| Municipality: |  |
| State: | notusa |
| Country: | China |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (site-years): | 195 site-years |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Dec 02, 2013 |
| Comments: | Major Road ADT is total entering vehicles for the intersection |
|  |  |

[^7]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 351
CMF Name: Replace direct left-turn with right-turn/U-turn

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Access management

Study ID: Right Turns Followed by U-Turns Versus Direct Left Turns: A Comparison of Safety Issues, Xu 2001

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.8 |
| Adjusted Standard Error: | 0.13 |
| Unadjusted Standard Error: | 0.03 |


|  |  | Crash Reduction Factor |
| ---: | :---: | :---: |
| Value: | 20 |  |
| Adjusted Standard Error: | 13 |  |
| Unadjusted Standard Error: | 3 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Principal Arterial Other |
| Minimum Number of Lanes: | 4 |
| Maximum Number of Lanes: | 8 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Not Specified |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | Not Specified |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: | Minimum of 0 to Maximum of 34000 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | ---: |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Non-regression cross-section |


|  | Other Details |  |  |  |  |
| ---: | :--- | :---: | :---: | :---: | :---: |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |  |  |  |  |
| Date Added to Clearinghouse: | Dec 01, 2009 |  |  |  |  |
| Comments: |  |  |  |  |  |
|  |  |  |  |  |  |

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## CMF / CRF Details

CMF ID: 7983
CMF Name: Install a traffic signal

## Description:

## Prior Condition: Intersections with a stop sign on minor roads

## Category: Intersection traffic control

Study ID: Safety Evaluation of Signal Installation With and Without Left Turn Lanes on Two Lane Roads in Rural and Suburban Areas, Srinivasan et al. $\underline{2014}$

## Star Quality Rating

|  | Crash Modification Factor (CMF) |
| ---: | :--- | :--- |
| Value: | 0.639 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.033 |


|  | Crash Reduction Factor |
| ---: | :--- | :--- | :--- |
| Value: | 36.1 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 3.3 |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: | 2 |
| Maximum Number of Lanes: | 2 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | All |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Not specified |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: | Minimum of 2480 to Maximum of 18025 Annual Average Daily Traffic (AADT) |
| Minor Road Traffic Volume: | Minimum of 746 to Maximum of 6829 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

9778 Annual Average Daily Traffic (AADT)

5767 Annual Average Daily Traffic (AADT)

## Development Details

| Date Range of Data Used: | 1992 to 2012 |
| ---: | :--- | :--- |
| State: | NC |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |
| Sample Size (crashes): | 899 crashes before, 575 crashes after |
| Sample Size (sites): | 50 sites before, 50 sites after |
| Sample Size (site-years): | site-years before, 240 site-years after |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Nov 10, 2016 |
| Comments: | The CMF was developed for both rural and suburban areas. |
|  |  |

[^8]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 286
CMF Name: Provide a right-turn lane on one major-road approach

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Intersection geometry

Study ID: Safety Effectiveness of Intersection Left- and Right-Turn Lanes, Harwood et al. 2002

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.96 |
| Adjusted Standard Error: | 0.02 |
| Unadjusted Standard Error: | 0.02 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 4 |  |
| Adjusted Standard Error: | 2 |  |
| Unadjusted Standard Error: | 2 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not Specified |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Signalized |
| Major Road Traffic Volume: | Minimum of 7200 to Maximum of 55100 Average Daily Traffic (ADT) |
| Minor Road Traffic Volume: | Minimum of 550 to Maximum of 8400 Average Daily Traffic (ADT) |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | ---: |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |


|  | Other Details |
| :---: | :--- |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: | Countermeasure name changed to match HSM The number of crashes in the <br> after period were not reported in this study, however, they have been recorded <br> as 300 to give 10 points as a beneift of doubt for one or more of the following: <br> (1) number of miles/sites in the reference/treatment group, (2) number of <br> crashes in the references/treatment group, (3) reporting AADTs for the |
| aggregate dataset but not for the disaggragate dataset used for CMF |  |
| development. |  |

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a substitute for sound engineering judgment.

CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 227
CMF Name: Convert intersection with minor-road stop control to modern round

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Intersection geometry

Study ID: NCHRP Report 572: Applying Roundabouts in the United States, Rodegerdts et al. 2007

|  | Star Quality Rating |
| :--- | :--- |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.56 |
| Adjusted Standard Error: | 0.05 |
| Unadjusted Standard Error: | 0.04 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 44 |  |
| Adjusted Standard Error: | 5 |  |
| Unadjusted Standard Error: | 4 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not Specified |
| Minimum Number of Lanes: | 1 |
| Maximum Number of Lanes: | 2 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
|  | If countermeasure is intersection-based. |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 4-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |


|  | Other Details |
| ---: | :--- |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: | Countermeasure name changed from |

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CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 154
CMF Name: Prohibit on-street parking

## Description:

## Prior Condition: No Prior Condition(s)

## Category: On-street parking

Study ID: Handbook of Road Safety Measures, Elvik, R. and Vaa, T. 2004

|  |  | Star Quality Rating |
| ---: | :--- | :--- |
| Star Quality Rating: | 4 Stars |  |
|  | Crash Modification Factor (CMF) |  |
| Value: | 0.73 |  |
| Adjusted Standard Error: | 0.02 |  |
| Unadjusted Standard Error: | 0.01 |  |
| Value: | 27 |  |
| Adjusted Standard Error: | 2 |  |
| Unadjusted Standard Error: | 1 |  |
|  |  |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | O (property damage only) |
| Roadway Types: | Minor Arterial |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
|  | If countermeasure is intersection-based. |
| Intersection Type: |  |
| Intersection Geometry: |  |
| Traffic Control: |  |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Meta-analysis |


|  | Other Details |
| ---: | :--- |
| Included in HSM: | No |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: |  |
|  |  |

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## Socio-Economic Conditions

Total of publicly subsidized rental
housing units in census
tracts within $1 / 2$ mile: 255
Project located in census tracts that are BELOW the regional average for population in poverty or population of color.



Area of Concentrated Poverty
Regional Environmental Justice Area

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Hastings Socioeconomic Community Landmarks Highway 61 Corridor
Regional Solicitation Application
December 2023


| Hastings Highway 61 Modernization Project |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Congestion Reduction Calculation Table |  |  |  |  |  |
|  | TH 316 | CSAH 47 | TH 55 | 10th St | TOTAL |
| Peak Hour Delay/Vehicle without the Project (seconds/vehicle) | 47 | 12 | 19 | 11 | $\mathbf{8 9}$ |
| Peak Hour Delay/Vehicle with the Project (seconds/vehicle) | 6 | 9 | 13 | 12 | $\mathbf{4 0}$ |
| Peak Hour Delay/Vehicle Reduced by the Project (seconds/vehicle) | 41 | 3 | 6 | -1 | $\mathbf{4 9}$ |
| Volume (vehicles/hour) | 2302 | 2914 | 3141 | 3049 | $\mathbf{1 1 4 0 6}$ |
| Total Peak Hour Delay Reduced by the Project (seconds) | 94382 | 8742 | 18846 | -3049 | $\mathbf{1 1 8 9 2 1}$ |


| Emissions Reduction Calculation Table |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Total Peak Hour Emissions without the Project (kg) | 8.05 | 5.82 | 4.37 | 4.31 | $\mathbf{2 2 . 5 5}$ |
| Total Peak Hour Emissions with the Project (kg) | 4.96 | 5.62 | 3.81 | 4.6 | $\mathbf{1 8 . 9 9}$ |
| Total Peak Hour Emissions Reduced by the Project (kg) | 3.09 | 0.2 | 0.56 | -0.29 | 3.56 |



|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

[^10]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\emptyset 8$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases |  | 4 | 2 |  |  |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 |  | 1.0 |
| Minimum Split (s) | 20.0 | 20.0 | 10.0 | 21.0 | 27.5 |  | 5.0 |
| Total Split (s) | 24.0 | 24.0 | 18.0 | 71.0 | 53.0 |  | 5.0 |
| Total Split (\%) | 24.0\% | 24.0\% | 18.0\% | 71.0\% | 53.0\% |  | 5\% |
| Maximum Green (s) | 18.0 | 18.0 | 13.0 | 65.5 | 47.5 |  | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 |  | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 |  | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 |  |  |
| Lead/Lag | Lag | Lag | Lag |  | Lead |  | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 |  | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 |  | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 |  | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max |  | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 16.8 | 16.8 | 71.2 | 70.7 | 52.7 |  |  |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.71 | 0.71 | 0.53 |  |  |
| v/c Ratio | 0.72 | 0.58 | 0.55 | 0.40 | 0.71 |  |  |
| Control Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total Delay | 53.6 | 9.4 | 27.7 | 7.0 | 7.2 |  |  |
| LOS | D | A | C | A | A |  |  |
| Approach Delay | 28.0 |  |  | 10.5 | 7.2 |  |  |
| Approach LOS | C |  |  | B | A |  |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 42 (42\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 70
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.72
Intersection Signal Delay: 12.0
Intersection LOS: B
Intersection Capacity Utilization 71.7\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47


|  | 4 |  |  | 7 |  |  | 4 | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ | \% | ${ }^{7}$ | $\hat{}$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | 个 $\uparrow$ | F |
| Traffic Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Future Volume (vph) | 356 | 36 | 343 | 22 | 26 | 16 | 223 | 838 | 28 | 6 | 876 | 345 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 400 |  | 0 | 0 |  | 0 | 150 |  | 0 | 100 |  | 25 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 50 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt |  |  | 0.850 |  | 0.942 |  |  | 0.995 |  |  |  | 0.850 |
| Flt Protected | 0.950 | 0.961 |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 1752 | 3491 | 0 | 1805 | 3471 | 1599 |
| FIt Permitted | 0.950 | 0.961 |  | 0.950 |  |  | 0.218 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1649 | 1680 | 1599 | 1805 | 1790 | 0 | 402 | 3491 | 0 | 1805 | 3471 | 1599 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 357 |  | 17 |  |  | 4 |  |  |  | 164 |
| Link Speed (mph) |  | 35 |  |  | 15 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 874 |  |  | 846 |  |  | 1453 |  |  | 382 |  |
| Travel Time (s) |  | 17.0 |  |  | 38.5 |  |  | 28.3 |  |  | 7.4 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 4\% | 0\% | 1\% | 0\% | 0\% | 0\% | 3\% | 3\% | 0\% | 0\% | 4\% | 1\% |
| Adj. Flow (vph) | 371 | 38 | 357 | 23 | 27 | 17 | 232 | 873 | 29 | 6 | 913 | 359 |
| Shared Lane Traffic (\%) | 45\% |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 204 | 205 | 357 | 23 | 44 | 0 | 232 | 902 | 0 | , | 913 | 359 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | LNA | Right | L NA | Left | Right | Left | Left | LNA |
| Median Width(t) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 2 | 2 | 1 | 1 | 1 |  | 1 | 1 |  | 1 | 1 | 0 |
| Detector Template |  |  | Right |  |  |  |  |  |  |  |  |  |
| Leading Detector (tt) | 103 | 103 | 20 | 45 | 45 |  | 50 | 117 |  | 65 | 126 | 0 |
| Trailing Detector (ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 5 | 5 |  | 0 | 111 |  | 15 | 120 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 40 | 40 |  | 50 | 6 |  | 50 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) | 97 | 97 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Size(ft) | 6 | 6 |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  |  |  |  |  |  |  |  |  |  |
| Turn Type | Split | NA | Perm | Split | NA |  | pm+pt | NA |  | Prot | NA | Perm |
| Protected Phases | 4 | 4 |  | 3 | 3 |  | 5 | 2 |  | 1 | 6 |  |


|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ |  | ( | 1 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  | 4 |  |  |  | 2 |  |  |  |  | 6 |
| Detector Phase | 4 | 4 | 4 | 3 | 3 |  | 52 | 2 |  | 16 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 7.0 | 5.0 | 5.0 |  | 7.0 | 15.0 |  | 7.0 | 15.0 | 15.0 |
| Minimum Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 12.5 | 34.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (s) | 26.0 | 26.0 | 26.0 | 14.0 | 14.0 |  | 19.0 | 47.5 |  | 12.5 | 41.0 | 41.0 |
| Total Split (\%) | 26.0\% | 26.0\% | 26.0\% | 14.0\% | 14.0\% |  | 19.0\% | 47.5\% |  | 12.5\% | 41.0\% | 41.0\% |
| Maximum Green (s) | 20.0 | 20.0 | 20.0 | 8.0 | 8.0 |  | 13.5 | 41.5 |  | 7.0 | 35.0 | 35.0 |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 3.5 | 3.5 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |  | 2.5 | 2.0 |  | 2.5 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |  | 5.5 | 6.0 |  | 5.5 | 6.0 | 6.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  | Lag | Lag |  | Lead | Lead | Lead |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |  | 0.2 | 2.5 |  | 0.2 | 2.5 | 2.5 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 15.0 |  | 0.0 | 15.0 | 15.0 |
| Recall Mode | None | None | None | None | None |  | None | C-Max |  | None | C-Max | C-Max |
| Walk Time (s) | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |  |  | 9.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) | 19.0 | 19.0 | 19.0 | 22.0 | 22.0 |  |  | 19.0 |  |  | 21.0 | 21.0 |
| Pedestrian Calls (\#/hr) | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 17.2 | 17.2 | 17.2 | 6.7 | 6.7 |  | 58.3 | 57.8 |  | 7.0 | 41.3 | 41.3 |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.17 | 0.07 | 0.07 |  | 0.58 | 0.58 |  | 0.07 | 0.41 | 0.41 |
| v/c Ratio | 0.72 | 0.71 | 0.63 | 0.19 | 0.32 |  | 0.56 | 0.45 |  | 0.05 | 0.64 | 0.47 |
| Control Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 18.8 | 6.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.3 | 0.3 |
| Total Delay | 53.4 | 52.5 | 9.2 | 47.2 | 37.1 |  | 19.1 | 9.0 |  | 54.7 | 19.1 | 7.1 |
| LOS | D | D | A | D | D |  | B | A |  | D | B | A |
| Approach Delay |  | 32.5 |  |  | 40.6 |  |  | 11.1 |  |  | 15.9 |  |
| Approach LOS |  | C |  |  | D |  |  | B |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 86 (86\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 95 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.72 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 18.7 |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 68.6\% |  |  |  | ICU Level of Service C |  |  |  |  |  |  |  |  |

Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55/Walgreens Ent


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  |  | $\uparrow$ | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{7}$ |  |  |
| Traffic Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Future Volume (vph) | 31 | 36 | 2 | 93 | 69 | 201 | 7 | 1113 | 90 | 161 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 25 |  | 0 | 0 |  | 250 | 95 |  | 170 | 280 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 1 | 1 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.992 |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected | 0.950 |  |  |  | 0.972 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1805 | 1833 | 0 | 0 | 1785 | 1599 | 1805 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted | 0.550 |  |  |  | 0.800 |  | 0.210 |  |  | 0.166 |  |  |
| Satd. Flow (perm) | 1045 | 1833 | 0 | 0 | 1469 | 1599 | 399 | 3471 | 1615 | 315 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 2 |  |  |  | 159 |  |  | 142 |  | 2 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 747 |  |  | 1138 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 17.0 |  |  | 25.9 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 32 | 38 | 2 | 97 | 72 | 209 | 7 | 1159 | 94 | 168 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 32 | 40 | 0 | 0 | 169 | 209 | 7 | 1159 | 94 | 168 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 1 | 0 | 1 | 1 |  |
| Detector Template | Left |  |  | Left |  |  |  |  |  |  |  |  |
| Leading Detector (ft) | 20 | 106 |  | 20 | 126 | 25 | 55 | 126 | 0 | 55 | 126 |  |
| Trailing Detector (ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Position(ft) | 0 | 5 |  | 0 | 5 | 5 | 5 | 120 | 0 | 5 | 120 |  |
| Detector 1 Size(ft) | 20 | 20 |  | 20 | 20 | 20 | 50 | 6 | 20 | 50 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  | 100 |  |  | 120 |  |  |  |  |  |  |  |
| Detector 2 Size(ft) |  | 6 |  |  | 6 |  |  |  |  |  |  |  |
| Detector 2 Type |  | Cl+Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  |  |  |  |  |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  | 0.0 |  |  | 0.0 |  |  |  |  |  |  |  |
| Turn Type | Perm | NA |  | Perm | NA | Perm | pm+pt | NA | Perm | pm+pt | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  | 5 | 2 |  | 1 | 6 |  |


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 |  | 8 | 8 | 8 | 52 | 2 | 2 | 16 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 31.0 | 31.0 | 10.0 | 23.0 |  |
| Total Split (s) | 30.0 | 30.0 |  | 30.0 | 30.0 | 30.0 | 10.0 | 54.0 | 54.0 | 16.0 | 60.0 |  |
| Total Split (\%) | 30.0\% | 30.0\% |  | 30.0\% | 30.0\% | 30.0\% | 10.0\% | 54.0\% | 54.0\% | 16.0\% | 60.0\% |  |
| Maximum Green (s) | 24.0 | 24.0 |  | 24.0 | 24.0 | 24.0 | 5.0 | 48.0 | 48.0 | 11.0 | 54.0 |  |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.0 | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) | 2.5 | 2.5 |  | 2.5 | 2.5 | 2.5 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 |  |  | 6.0 | 6.0 | 5.0 | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  | Lead | Lag | Lag | Lead | Lag |  |
| Lead-Lag Optimize? |  |  |  |  |  |  | Yes | Yes | Yes | Yes | Yes |  |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 2.0 | 3.5 | 3.5 | 2.0 | 3.5 |  |
| Minimum Gap (s) | 0.2 | 0.2 |  | 0.2 | 0.2 | 0.2 | 0.2 | 2.5 | 2.5 | 0.2 | 2.5 |  |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Time To Reduce (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 15.0 | 15.0 | 0.0 | 15.0 |  |
| Recall Mode | None | None |  | None | None | None | None | C-Max | C-Max | None | C-Max |  |
| Walk Time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 | 7.0 |  | 7.0 | 7.0 |  | 7.0 |  |
| Flash Dont Walk (s) | 23.0 | 23.0 |  | 20.0 | 20.0 | 20.0 |  | 18.0 | 18.0 |  | 10.0 |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 |  | 0 |  |
| Act Effct Green (s) | 16.9 | 16.9 |  |  | 16.9 | 16.9 | 64.5 | 58.5 | 58.5 | 72.1 | 69.1 |  |
| Actuated g/C Ratio | 0.17 | 0.17 |  |  | 0.17 | 0.17 | 0.64 | 0.58 | 0.58 | 0.72 | 0.69 |  |
| v/c Ratio | 0.18 | 0.13 |  |  | 0.68 | 0.52 | 0.02 | 0.57 | 0.09 | 0.49 | 0.50 |  |
| Control Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.7 | 1.4 | 11.6 | 4.9 |  |
| Queue Delay | 0.0 | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 35.5 | 32.3 |  |  | 52.1 | 14.7 | 5.4 | 10.9 | 1.4 | 11.6 | 4.9 |  |
| LOS | D | C |  |  | D | B | A | B | A | B | A |  |
| Approach Delay |  | 33.7 |  |  | 31.4 |  |  | 10.1 |  |  | 5.7 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 80 ( $80 \%$ ), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 11.3
Intersection LOS: B
Intersection Capacity Utilization 69.3\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 109: TH 61 \& 10th St


Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing No Build

| Intersection |  |  |  |
| :--- | ---: | ---: | ---: |
| Intersection Delay, s/veh | 6.4 |  |  |
| Intersection LOS | A |  | SB |
| Approach | WB | 1 | 2 |
| Entry Lanes | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 369 | 1213 |
| Adj Approach Flow, veh/h | 791 | 373 | 1254 |
| Demand Flow Rate, veh/h | 816 | 770 | 8 |
| Vehicles Circulating, veh/h | 344 | 492 | 344 |
| Vehicles Exiting, veh/h | 799 | 0 | 0 |
| Ped Vol Crossing Leg, \#/h | 0 | 1.000 | 7.4 |
| Ped Cap Adj | 1.000 | 16.8 | A |


| Lane | Left | Bypass | Left | Left | Right |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Designated Moves | L | R | TR | L | TR |
| Assumed Moves | L | R | TR | L | TR |
| RT Channelized |  | Free |  |  |  |
| Lane Util | 1.000 |  | 1.000 | 0.614 | 0.386 |
| Follow-Up Headway, s | 2.609 |  | 2.609 | 2.535 | 2.535 |
| Critical Headway, s | 4.976 | 808 | 4.976 | 4.544 | 4.544 |
| Entry Flow, veh/h | 8 | 1957 | 373 | 770 | 484 |
| Cap Entry Lane, veh/h | 972 | 0.971 | 629 | 1410 | 1410 |
| Entry HV Adj Factor | 0.875 | 784 | 0.988 | 0.971 | 0.962 |
| Flow Entry, veh/h | 7 | 1900 | 369 | 748 | 465 |
| Cap Entry, veh/h | 850 | 0.413 | 622 | 1370 | 1356 |
| V/C Ratio | 0.008 | 0.0 | 0.593 | 0.546 | 0.343 |
| Control Delay, s/veh | 4.3 | A | 16.8 | 8.5 | 5.8 |
| LOS | A | 2 | C | A | A |
| 95th \%tile Queue, veh | 0 |  | 4 | 3 | 2 |



[^11]Synchro 11 Report

| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | Ø8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 | 5.0 | 15.0 | 15.0 | 7.0 | 1.0 |
| Minimum Split (s) | 13.0 | 13.0 | 10.0 | 21.0 | 27.5 | 13.0 | 5.0 |
| Total Split (s) | 25.0 | 25.0 | 18.0 | 70.0 | 52.0 | 25.0 | 5.0 |
| Total Split (\%) | $25.0 \%$ | $25.0 \%$ | $18.0 \%$ | $70.0 \%$ | $52.0 \%$ | $25.0 \%$ | $5 \%$ |
| Maximum Green (s) | 19.0 | 19.0 | 13.0 | 64.5 | 46.5 | 19.0 | 2.0 |
| Yellow Time (s) | 4.0 | 4.0 | 3.0 | 4.0 | 4.0 | 4.0 | 2.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 1.5 | 1.5 | 2.0 | 1.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) | 6.0 | 6.0 | 5.0 | 5.5 | 5.5 | 6.0 |  |
| Lead/Lag | Lead | Lead | Lag |  | Lead | Lead | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 | 2.0 | 5.0 | 5.0 | 3.0 | 0.2 |
| Minimum Gap (s) | 0.2 | 0.2 | 0.2 | 2.0 | 2.0 | 0.2 | 0.2 |
| Time Before Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 | 0.0 | 20.0 | 20.0 | 0.0 | 0.0 |
| Recall Mode | None | None | None | C-Max | C-Max | None | None |
| Walk Time (s) |  |  |  |  | 7.0 |  | 7.0 |
| Flash Dont Walk (s) |  |  |  |  | 15.0 |  | 17.0 |
| Pedestrian Calls (\#/hr) |  |  |  |  | 0 |  | 4 |
| Act Effct Green (s) | 14.1 | 14.1 | 73.9 | 73.4 | 55.4 | 73.8 |  |
| Actuated g/C Ratio | 0.14 | 0.14 | 0.74 | 0.73 | 0.55 | 0.74 |  |
| v/c Ratio | 0.44 | 0.62 | 0.44 | 0.38 | 0.55 | 0.18 |  |
| Control Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay | 41.3 | 10.7 | 15.6 | 6.1 | 6.3 | 0.3 |  |
| LOS | D | B | B | A | A | A |  |
| Approach Delay | 23.6 |  |  | 7.7 | 5.3 |  |  |
| Approach LOS | C |  |  | A | A |  |  |
|  |  |  |  |  |  |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 8 (8\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 9.3 Intersection LOS: A

Intersection Capacity Utilization 59.4\% ICU Level of Service B
Analysis Period (min) 15
Splits and Phases: 106: TH 61 \& CSAH 47



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR | $\varnothing 5$ | $\varnothing 9$ |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Permitted Phases |  | 4 | 2 |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 52 | 2 | 6 | 6 |  |  |
| Switch Phase |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 7.0 | 7.0 |  | 15.0 | 15.0 | 15.0 | 7.0 | 7.0 |
| Minimum Split (s) | 26.0 | 26.0 |  | 34.5 | 41.0 | 41.0 | 12.5 | 12.5 |
| Total Split (s) | 26.0 | 26.0 |  | 74.0 | 43.3 | 43.3 | 18.2 | 12.5 |
| Total Split (\%) | $26.0 \%$ | $26.0 \%$ |  | $74.0 \%$ | $43.3 \%$ | $43.3 \%$ | $18 \%$ | $13 \%$ |
| Maximum Green (s) | 20.0 | 20.0 |  | 68.0 | 37.3 | 37.3 | 12.7 | 7.0 |
| Yellow Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.5 | 2.5 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |  |  |
| Lead/Lag |  |  |  |  | Lag | Lag |  | Lead |
| Lead-Lag Optimize? |  |  |  |  | Yes | Yes |  | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| Minimum Gap (s) | 0.2 | 0.2 |  | 2.5 | 2.5 | 2.5 | 0.2 | 3.0 |
| Time Before Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Time To Reduce (s) | 0.0 | 0.0 |  | 15.0 | 15.0 | 15.0 | 0.0 | 0.0 |
| Recall Mode | None | None |  | C-Max | C-Max | C-Max | None | None |
| Walk Time (s) | 7.0 | 7.0 |  | 9.0 | 7.0 | 7.0 |  |  |
| Flash Dont Walk (s) | 19.0 | 19.0 |  | 19.0 | 21.0 | 21.0 |  |  |
| Pedestrian Calls (\#/hr) | 0 | 0 |  | 0 | 0 | 0 |  |  |
| Act Effct Green (s) | 18.9 | 18.9 | 69.6 | 69.1 | 50.9 | 50.9 |  |  |
| Actuated g/C Ratio | 0.19 | 0.19 | 0.70 | 0.69 | 0.51 | 0.51 |  |  |
| v/c Ratio | 0.75 | 0.61 | 0.51 | 0.36 | 0.53 | 0.40 |  |  |
| Control Delay | 46.5 | 8.6 | 12.2 | 4.3 | 10.2 | 1.4 |  |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.3 |  |  |
| Total Delay | 46.5 | 8.6 | 12.2 | 4.4 | 10.5 | 1.7 |  |  |
| LOS | D | A | B | A | B | A |  |  |
| Approach Delay | 30.2 |  |  | 6.1 | 7.9 |  |  |  |
| Approach LOS | C |  |  |  | A | A |  |  |

Intersection Summary
Area Type: Other
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 48 (48\%), Referenced to phase 2:NBTL and 6:SBT, Start of 1st Green
Natural Cycle: 95
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.75
Intersection Signal Delay: 13.0
Intersection Capacity Utilization 65.2\%
Intersection LOS: B
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 108: TH 61 \& TH 55


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  |  |  |  | $\uparrow$ | 「 |  | 44 | 「 | 1 | 44 |  |
| Traffic Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Future Volume (vph) | 0 | 0 | 0 | 141 | 69 | 217 | 0 | 1144 | 162 | 167 | 1132 | 17 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 250 | 0 |  | 150 | 225 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 1 | 0 |  | 1 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 50 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 |
| Frt |  |  |  |  |  | 0.850 |  |  | 0.850 |  | 0.998 |  |
| Flt Protected |  |  |  |  | 0.968 |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 1805 | 3499 | 0 |
| Flt Permitted |  |  |  |  | 0.968 |  |  |  |  | 0.152 |  |  |
| Satd. Flow (perm) | 0 | 0 | 0 | 0 | 1768 | 1599 | 0 | 3471 | 1615 | 289 | 3499 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  | 226 |  |  | 133 |  | 3 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 713 |  |  | 848 |  |  | 382 |  |  | 2197 |  |
| Travel Time (s) |  | 16.2 |  |  | 19.3 |  |  | 7.4 |  |  | 42.8 |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Heavy Vehicles (\%) | 0\% | 3\% | 0\% | 6\% | 0\% | 1\% | 0\% | 4\% | 0\% | 0\% | 3\% | 0\% |
| Adj. Flow (vph) | 0 | 0 | 0 | 147 | 72 | 226 | 0 | 1192 | 169 | 174 | 1179 | 18 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 219 | 226 | 0 | 1192 | 169 | 174 | 1197 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 16 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors |  |  |  | 1 | 2 | 1 |  | 2 | 1 | 1 | 2 |  |
| Detector Template |  |  |  | Left | Thru | Right |  | Thru | Right | Left | Thru |  |
| Leading Detector (ft) |  |  |  | 20 | 100 | 20 |  | 100 | 20 | 20 | 100 |  |
| Trailing Detector (ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Position(ft) |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |
| Detector 1 Size(ft) |  |  |  | 20 | 6 | 20 |  | 6 | 20 | 20 | 6 |  |
| Detector 1 Type |  |  |  | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Queue (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 1 Delay (s) |  |  |  | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Detector 2 Position(ft) |  |  |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  |  |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  |  |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  |  |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type |  |  |  | Split | NA | Perm |  | NA | Perm | pm+pt | NA |  |
| Protected Phases |  |  |  | 8 | 8 |  |  | 2 |  | 1 | 6 |  |


|  | 4 |  |  |  |  |  |  | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Permitted Phases |  |  |  |  |  | 8 |  |  | 2 | 6 |  |  |
| Detector Phase |  |  |  | 8 | 8 | 8 |  | 2 | 2 | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  |  |  | 6.5 | 6.5 | 6.5 |  | 15.0 | 15.0 | 5.0 | 15.0 |  |
| Minimum Split (s) |  |  |  | 12.5 | 12.5 | 12.5 |  | 22.0 | 22.0 | 10.0 | 22.0 |  |
| Total Split (s) |  |  |  | 29.0 | 29.0 | 29.0 |  | 55.0 | 55.0 | 16.0 | 71.0 |  |
| Total Split (\%) |  |  |  | 29.0\% | 29.0\% | 29.0\% |  | 55.0\% | 55.0\% | 16.0\% | 71.0\% |  |
| Maximum Green (s) |  |  |  | 23.0 | 23.0 | 23.0 |  | 49.0 | 49.0 | 11.0 | 65.0 |  |
| Yellow Time (s) |  |  |  | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 | 3.0 | 4.0 |  |
| All-Red Time (s) |  |  |  | 2.5 | 2.5 | 2.5 |  | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  |  |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) |  |  |  |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 5.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  |  | Lag | Lag | Lead |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  | Yes | Yes | Yes |  |  |
| Vehicle Extension (s) |  |  |  | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 |  |
| Recall Mode |  |  |  | None | None | None |  | C-Max | C-Max | None | C-Max |  |
| Act Effict Green (s) |  |  |  |  | 17.6 | 17.6 |  | 56.8 | 56.8 | 71.4 | 70.4 |  |
| Actuated g/C Ratio |  |  |  |  | 0.18 | 0.18 |  | 0.57 | 0.57 | 0.71 | 0.70 |  |
| v/c Ratio |  |  |  |  | 0.71 | 0.48 |  | 0.61 | 0.17 | 0.52 | 0.49 |  |
| Control Delay |  |  |  |  | 50.7 | 8.2 |  | 14.7 | 4.4 | 10.0 | 4.7 |  |
| Queue Delay |  |  |  |  | 0.0 | 0.0 |  | 0.5 | 0.0 | 0.0 | 0.0 |  |
| Total Delay |  |  |  |  | 50.7 | 8.2 |  | 15.3 | 4.4 | 10.0 | 4.7 |  |
| LOS |  |  |  |  | D | A |  | B | A | B | A |  |
| Approach Delay |  |  |  |  | 29.1 |  |  | 13.9 |  |  | 5.4 |  |
| Approach LOS |  |  |  |  | C |  |  | B |  |  | A |  |

Intersection Summary
Area Type: Other

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 58 (58\%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 12.4
Intersection LOS: B
Intersection Capacity Utilization 66.5\%
ICU Level of Service C
Analysis Period (min) 15


5: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 47 |
| CO Emissions $(\mathrm{kg})$ | 5.64 |
| NOx Emissions $(\mathrm{kg})$ | 1.10 |
| VOC Emissions (kg) | 1.31 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 4.08 |
| NOx Emissions $(\mathrm{kg})$ | 0.79 |
| VOC Emissions $(\mathrm{kg})$ | 0.95 |

108: TH 61 \& TH 55/Walgreens Ent

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3116 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 19 |
| CO Emissions $(\mathrm{kg}$ | 3.06 |
| NOx Emissions kg$)$ | 0.60 |
| VOC Emissions (kg) | 0.71 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2952 |
| Total Delay / veh (s/v) | 11 |
| CO Emissions $(\mathrm{kg})$ | 3.02 |
| NOx Emissions kg$)$ | 0.59 |
| VOC Emissions $(\mathrm{kg})$ | 0.70 |

9: TH 61 \& TH 316

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2302 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 0 |
| CO Emissions $(\mathrm{kg})$ | 3.47 |
| NOx Emissions $(\mathrm{kg})$ | 0.68 |
| VOC Emissions (kg) | 0.81 |

106: TH 61 \& CSAH 47

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 2914 |
| Total Delay / Veh $(\mathrm{s} / \mathrm{v})$ | 9 |
| CO Emissions kg ) | 3.94 |
| NOx Emissions $(\mathrm{kg})$ | 0.77 |
| VOC Emissions $(\mathrm{kg})$ | 0.91 |

108: TH 61 \& TH 55

| Direction | All |
| :--- | :---: |
| Future Volume (vph) | 3141 |
| Total Delay / veh (s/v) | 13 |
| CO Emissions $(\mathrm{kg}$ | 2.67 |
| NOx Emissions kg$)$ | 0.52 |
| VOC Emissions $(\mathrm{kg})$ | 0.62 |

109: TH 61 \& 10th St

| Direction | All |
| :--- | ---: |
| Future Volume (vph) | 3049 |
| Total Delay $/$ Veh $(\mathrm{s} / \mathrm{v})$ | 12 |
| CO Emissions $(\mathrm{kg})$ | 3.22 |
| NOx Emissions $(\mathrm{kg})$ | 0.63 |
| VOC Emissions $(\mathrm{kg})$ | 0.75 |

Traffic Safety Benefit-Cost Calculation
Highway Safety Improvement Program (HSIP) Reactive Project

## A. Roadway Description

| Route | US 61 | District | Metro | County | Dakota |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Begin RP | 4th Street | End RP | 36th Street | Miles | 2.3 miles |
| Location | Highway 61 corridor entirely within City of Hastings municipal boundary |  |  |  |  |

B. Project Description

| Proposed Work <br> Project Cost* | Corridor improvements at several intersections |  |  |
| :---: | :---: | :---: | :---: |
|  | \$21,408,861 | Installation Year | 2027 |
| Project Service Life | 30 years | Traffic Growth Factor | 1.2\% |
| * exclude Right of Way from Project Cost |  |  |  |

C. Crash Modification Factor

| 1.00 | Fatal (K) Crashes | Reference | See attached worksheet |
| :--- | :--- | :--- | :--- |
|  |  |  |  |
| 0.87 | Serious Injury (A) Crashes |  |  |
| 0.65 | Moderate Injury (B) Crashes | Crash Type All |  |
| 0.80 | Possible Injury (C) Crashes |  |  |
| 0.80 | Property Damage Only Crashes |  | www.CMFclearinghouse.org |

D. Crash Modification Factor (optional second CMF)

|  | Fatal (K) Crashes | Reference |  |
| :--- | :--- | :--- | :--- |
|  | Serious Injury (A) Crashes |  |  |
|  | Moderate Injury (B) Crashes | Crash Type |  |
|  |  |  | www.CMFclearinghouse.org |


F. Analysis Assumptions

Crash Severity

| K crashes | $\$ 1,600,000$ |
| :--- | ---: |
| A crashes | $\$ 800,000$ |
| B crashes | $\$ 250,000$ |
| C crashes | $\$ 130,000$ |
| PDO crashes | $\$ 15,000$ |

Link: mndot.gov/planning/program/appendix_a.html
Real Discount Rate: $0.8 \%$ Default
Traffic Growth Rate: $\quad 1.2 \%$ Revised

Project Service Life: 30 years Revised

## G. Annual Benefit

| Crash Severity | Crash Reduction | Annual Reduction | Annual Benefit |
| :--- | :---: | :---: | :---: |
| K crashes | 0.00 | 0.00 | $\$ 0$ |
| A crashes | 0.27 | 0.09 | $\$ 72,000$ |
| B crashes | 4.24 | 1.41 | $\$ 353,700$ |
| C crashes | 5.95 | 1.98 | $\$ 257,920$ |
| PDO crashes | 29.58 | 9.86 | $\$ 147,900$ |


| Year | Crash Benefits | Present Value |  |
| :---: | :---: | :---: | :---: |
| 2027 | \$831,520 | \$831,520 | Total $=$ \$26,396,962 |
| 2028 | \$841,415 | \$834,737 |  |
| 2029 | \$851,428 | \$837,967 |  |
| 2030 | \$861,560 | \$841,209 |  |
| 2031 | \$871,812 | \$844,464 |  |
| 2032 | \$882,187 | \$847,731 |  |
| 2033 | \$892,685 | \$851,011 |  |
| 2034 | \$903,308 | \$854,303 |  |
| 2035 | \$914,057 | \$857,609 |  |
| 2036 | \$924,935 | \$860,927 |  |
| 2037 | \$935,941 | \$864,258 |  |
| 2038 | \$947,079 | \$867,602 |  |
| 2039 | \$958,349 | \$870,959 |  |
| 2040 | \$969,754 | \$874,328 |  |
| 2041 | \$981,294 | \$877,711 |  |
| 2042 | \$992,971 | \$881,107 |  |
| 2043 | \$1,004,788 | \$884,516 |  |
| 2044 | \$1,016,744 | \$887,938 |  |
| 2045 | \$1,028,844 | \$891,374 |  |
| 2046 | \$1,041,087 | \$894,823 |  |
| 2047 | \$1,053,476 | \$898,285 |  |
| 2048 | \$1,066,012 | \$901,760 |  |
| 2049 | \$1,078,698 | \$905,249 |  |
| 2050 | \$1,091,534 | \$908,752 |  |
| 2051 | \$1,104,524 | \$912,268 |  |
| 2052 | \$1,117,667 | \$915,797 |  |
| 2053 | \$1,130,968 | \$919,340 |  |
| 2054 | \$1,144,426 | \$922,897 | NOTE: |
| 2055 | \$1,158,045 | \$926,468 | This calculation relies on the real discount rate, which accounts |
| 2056 | \$1,171,826 | \$930,053 | for inflation. No further discounting is necessary. |
| 0 | \$0 | \$0 |  |



| Intersection | Crashes Eliminated |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\kappa$ | A | B | c | $\bigcirc$ |
| 5 th Street | 0.00 | 0.00 | 1.18 | 0.00 | 2.37 |
| 7 th Street | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 10 th Street | 0.00 | 0.00 | 0.25 | 1.75 | 4.25 |
| Trunk Hwy 55 | 0.00 | 0.00 | 0.25 | 0.75 | 3.00 |
| 12 th Street | 0.00 | 0.00 | 0.00 | 0.45 | 4.95 |
| 144h Street | 0.00 | 0.00 | 0.00 | 0.40 | 1.20 |
| 18th Street | 0.00 | 0.00 | 0.36 | 0.36 | 2.89 |
| County Hwy 47 | 0.00 | 0.00 | 0.04 | 0.04 | 0.64 |
| 21 1ststreet | 0.00 | 0.00 | 0.45 | 0.00 | 3.60 |
| 24 th Street | 0.00 | 0.00 | 0.00 | 0.00 | 0.90 |
| Trunk Hwy 316 | 0.00 | 0.00 | 0.00 | 1.76 | 2.64 |
| Cannon | 0.00 | 0.00 | 1.00 | 0.00 | 2.00 |
| 36th Street | 0.00 | 0.00 | 0.44 | 0.44 | 0.88 |
| Corridor | 0.00 | 0.27 | 0.27 | 0.00 | 0.27 |
| Corridor Total (All intersections and segments) | 0.00 | 0.27 | 4.24 | 5.95 | 29.5 |
| Product of Crashes Observed and CRF (Table 1). |  |  |  |  |  |
| Effective CMF (1Crashes Eliminated/Crashes Observed) |  |  |  |  |  |

CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 9821
CMF Name: Install right-in-right-out (RIRO) operations at stop-controlled inters

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Access management

Study ID: Safety Effects of Turning Movement Restrictions at Stop-Controlled Intersections, Le et al. 2018

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.55 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.09 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 45 |  |
| Adjusted Standard Error: |  |  |
| Unadjusted Standard Error: | 9 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: | 4 |
| Maximum Number of Lanes: | 6 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: | 4 and 6 Lanes |
| Road Division Type: | Divided by Median |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | All |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: | Minimum of 13433 to Maximum of 75000 Annual Average Daily Traffic (AADT) |
| Minor Road Traffic Volume: | Minimum of 51 to Maximum of 2600 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

38724 Annual Average Daily Traffic (AADT)

519 Annual Average Daily Traffic (AADT)

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: | CA |
| Country: | USA |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (crashes): | 483 crashes |
| Sample Size (sites): | 138 sites |

## Other Details

| Included in HSM: | No |
| ---: | :--- | :--- |
| Date Added to Clearinghouse: | Oct 27, 2018 |
| Comments: | This CMF compares urban, three-legged, stop-controlled intersections with <br> RIRO operation to full movement. This CMF looks at Total crashes. Total <br> crashes are defined as all crashes within 100 ft of intersection (all types and <br> severities combined) |

[^12]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 8800
CMF Name: Install raised median with or without marked crosswalk (uncontroll

## Description:

## Prior Condition: No median

## Category: Pedestrians

Study ID: Development of Crash Modification Factors for Uncontrolled Pedestrian Crossing Treatments, Zegeer et al. 2017

|  |  |
| ---: | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.742 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.071 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 25.8 |  |
| Adjusted Standard Error: |  |  |
| Unadjusted Standard Error: | 7.1 |  |


| Applicability |  |
| :---: | :---: |
| Crash Type: | All |
| Crash Severity: | All |
| Roadway Types: | Minor Arterial |
| Minimum Number of Lanes: | 2 |
| Maximum Number of Lanes: | 8 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: | Divided by Median |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban and suburban |
| Traffic Volume: | Minimum of 1245 to Maximum of 46000 Annual Average Daily Traffic (AADT) |
| Average Traffic Volume: |  |
| Time of Day: | All |
|  | If countermeasure is intersection-based. |
| Intersection Type: |  |
| Intersection Geometry: |  |
| Traffic Control: |  |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: | 2004 to 2013 |
| ---: | :--- |
| Municipality: |  |
| State: | AZ,FL,IL,MA,NY,NC,OR,VA,WI |
| Country: | USA |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (crashes): | 10666 crashes |
| Sample Size (site-years): | 5021 site-years |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Nov 17, 2017 |
| Comments: | Study sites were a combination of intersection and mid-block locations. |

[^13]
## CMF / CRF Details

CMF ID: 5233
CMF Name: Presence of three leg intersection vs. four leg intersection

## Description:

## Prior Condition: Four-leg intersection

## Category: Intersection geometry

Study ID: Corridor-level signalized intersection safety analysis in Shanghai, China using Bayesian hierarchal models, Kun Xie, Xuesong Wang, Helai Huang, Xiahong Chen 2013

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.75 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.13 |


|  | Crash Reduction Factor |
| ---: | :---: |
| Value: | 25 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 12.8 |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | Not specified |
|  | If countermeasure is intersection-based. |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Signalized |
| Major Road Traffic Volume: | Minimum of 7700 to Maximum of 140300 Average Daily Traffic (ADT) |
| Minor Road Traffic Volume: |  |

## Average Major Road Volume:

Average Minor Road Volume:

46200 Average Daily Traffic (ADT)
$\square$

## Development Details

| Date Range of Data Used: | 2009 to 2009 |
| ---: | :--- |
| Municipality: |  |
| State: | notusa |
| Country: | China |
| Type of Methodology Used: | Regression cross-section |
| Sample Size (site-years): | 195 site-years |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Dec 02, 2013 |
| Comments: | Major Road ADT is total entering vehicles for the intersection |
|  |  |

[^14]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 351
CMF Name: Replace direct left-turn with right-turn/U-turn

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Access management

Study ID: Right Turns Followed by U-Turns Versus Direct Left Turns: A Comparison of Safety Issues, Xu 2001

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.8 |
| Adjusted Standard Error: | 0.13 |
| Unadjusted Standard Error: | 0.03 |


|  |  | Crash Reduction Factor |
| ---: | :---: | :---: |
| Value: | 20 |  |
| Adjusted Standard Error: | 13 |  |
| Unadjusted Standard Error: | 3 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Principal Arterial Other |
| Minimum Number of Lanes: | 4 |
| Maximum Number of Lanes: | 8 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Not Specified |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | Not Specified |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: | Minimum of 0 to Maximum of 34000 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | ---: |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Non-regression cross-section |


|  | Other Details |  |  |  |  |
| ---: | :--- | :---: | :---: | :---: | :---: |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |  |  |  |  |
| Date Added to Clearinghouse: | Dec 01, 2009 |  |  |  |  |
| Comments: |  |  |  |  |  |
|  |  |  |  |  |  |

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## CMF / CRF Details

CMF ID: 7983
CMF Name: Install a traffic signal

## Description:

## Prior Condition: Intersections with a stop sign on minor roads

## Category: Intersection traffic control

Study ID: Safety Evaluation of Signal Installation With and Without Left Turn Lanes on Two Lane Roads in Rural and Suburban Areas, Srinivasan et al. $\underline{2014}$

## Star Quality Rating

|  | Crash Modification Factor (CMF) |
| ---: | :--- | :--- |
| Value: | 0.639 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.033 |


|  | Crash Reduction Factor |
| ---: | :--- | :--- | :--- |
| Value: | 36.1 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 3.3 |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not specified |
| Minimum Number of Lanes: | 2 |
| Maximum Number of Lanes: | 2 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: | All |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Not specified |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: | Minimum of 2480 to Maximum of 18025 Annual Average Daily Traffic (AADT) |
| Minor Road Traffic Volume: | Minimum of 746 to Maximum of 6829 Annual Average Daily Traffic (AADT) |

Average Major Road Volume:

Average Minor Road Volume:

9778 Annual Average Daily Traffic (AADT)

5767 Annual Average Daily Traffic (AADT)

## Development Details

| Date Range of Data Used: | 1992 to 2012 |
| ---: | :--- | :--- |
| State: | NC |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |
| Sample Size (crashes): | 899 crashes before, 575 crashes after |
| Sample Size (sites): | 50 sites before, 50 sites after |
| Sample Size (site-years): | site-years before, 240 site-years after |

## Other Details

| Included in HSM: | No |
| ---: | :--- |
| Date Added to Clearinghouse: | Nov 10, 2016 |
| Comments: | The CMF was developed for both rural and suburban areas. |
|  |  |

[^15]CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 286
CMF Name: Provide a right-turn lane on one major-road approach

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Intersection geometry

Study ID: Safety Effectiveness of Intersection Left- and Right-Turn Lanes, Harwood et al. 2002

|  |  |
| :--- | :--- |
|  | Star Quality Rating |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.96 |
| Adjusted Standard Error: | 0.02 |
| Unadjusted Standard Error: | 0.02 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 4 |  |
| Adjusted Standard Error: | 2 |  |
| Unadjusted Standard Error: | 2 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not Specified |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
| If countermeasure is intersection-based. |  |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 3-leg,4-leg |
| Traffic Control: | Signalized |
| Major Road Traffic Volume: | Minimum of 7200 to Maximum of 55100 Average Daily Traffic (ADT) |
| Minor Road Traffic Volume: | Minimum of 550 to Maximum of 8400 Average Daily Traffic (ADT) |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | ---: |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |


|  | Other Details |
| :---: | :--- |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: | Countermeasure name changed to match HSM The number of crashes in the <br> after period were not reported in this study, however, they have been recorded <br> as 300 to give 10 points as a beneift of doubt for one or more of the following: <br> (1) number of miles/sites in the reference/treatment group, (2) number of <br> crashes in the references/treatment group, (3) reporting AADTs for the |
| aggregate dataset but not for the disaggragate dataset used for CMF |  |
| development. |  |

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a substitute for sound engineering judgment.

CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 227
CMF Name: Convert intersection with minor-road stop control to modern round

## Description:

## Prior Condition: No Prior Condition(s)

## Category: Intersection geometry

Study ID: NCHRP Report 572: Applying Roundabouts in the United States, Rodegerdts et al. 2007

|  | Star Quality Rating |
| :--- | :--- |
| Star Quality Rating: | 4 Stars |
|  |  |
|  | Crash Modification Factor (CMF) |
| Value: | 0.56 |
| Adjusted Standard Error: | 0.05 |
| Unadjusted Standard Error: | 0.04 |


|  |  | Crash Reduction Factor |
| ---: | :--- | :--- |
| Value: | 44 |  |
| Adjusted Standard Error: | 5 |  |
| Unadjusted Standard Error: | 4 |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | All |
| Roadway Types: | Not Specified |
| Minimum Number of Lanes: | 1 |
| Maximum Number of Lanes: | 2 |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
|  | If countermeasure is intersection-based. |
| Intersection Type: | Roadway/roadway (not interchange related) |
| Intersection Geometry: | 4-leg |
| Traffic Control: | Stop-controlled |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |


|  | Other Details |
| ---: | :--- |
| Included in HSM: | Yes. HSM lists this CMF in <strong>bold</strong> font to indicate that it has the highes |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: | Countermeasure name changed from |

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CRASH MODIFICATION FACTORS CLEARINGHOUSE

## CMF / CRF Details

CMF ID: 154
CMF Name: Prohibit on-street parking

## Description:

## Prior Condition: No Prior Condition(s)

## Category: On-street parking

Study ID: Handbook of Road Safety Measures, Elvik, R. and Vaa, T. 2004

|  |  | Star Quality Rating |
| ---: | :--- | :--- |
| Star Quality Rating: | 4 Stars |  |
|  | Crash Modification Factor (CMF) |  |
| Value: | 0.73 |  |
| Adjusted Standard Error: | 0.02 |  |
| Unadjusted Standard Error: | 0.01 |  |
| Value: | 27 |  |
| Adjusted Standard Error: | 2 |  |
| Unadjusted Standard Error: | 1 |  |
|  |  |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | O (property damage only) |
| Roadway Types: | Minor Arterial |
| Minimum Number of Lanes: |  |
| Maximum Number of Lanes: |  |
| Number of Lanes Direction: |  |
| Number of Lanes Comment: |  |
| Road Division Type: |  |
| Minimum Speed Limit: |  |
| Maximum Speed Limit: |  |
| Speed Unit: |  |
| Speed Limit Comment: |  |
| Area Type: | Urban |
| Traffic Volume: |  |
| Average Traffic Volume: |  |
| Time of Day: |  |
|  | If countermeasure is intersection-based. |
| Intersection Type: |  |
| Intersection Geometry: |  |
| Traffic Control: |  |
| Major Road Traffic Volume: |  |
| Minor Road Traffic Volume: |  |

Average Major Road Volume:

Average Minor Road Volume:

## Development Details

| Date Range of Data Used: |  |
| ---: | :--- | :--- |
| Municipality: |  |
| State: |  |
| Country: |  |
| Type of Methodology Used: | Meta-analysis |


|  | Other Details |
| ---: | :--- |
| Included in HSM: | No |
| Date Added to Clearinghouse: | Dec 01, 2009 |
| Comments: |  |
|  |  |

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## Project Name: Hastings Highway 61 Modernization

## Applicant: City of Hastings

Project Background: This proposed project is the outcome of a Highway 61 Corridor Study completed by the Minnesota Department of Transportation (MnDOT) and the City of Hastings, with the intention of establishing a vision for long-term implementation of projects on the Highway 61 corridor in Hastings. The resulting vision covers approximately 2.3 miles of Highway 61 entirely within the city of Hastings.

Roadway Issues: The existing roadway (portions of which were last reconstructed in 1931) is nearing the end of its useful life and warrants replacement. A majority of the Highway 61 project corridor is characterized by four 12 -foot lanes (two running in each direction) plus an alternating 12 -foot center left turn median lane. From the Vermillion River to $36^{\text {th }}$ Street significant gaps in sidewalk continuity exist, leaving bicyclists and pedestrians with limited options for movement along the corridor. Over 30,000 vehicles use the corridor daily, with significant portions of heavy truck (freight) movement. More than 120 businesses front Highway 61 as well, making this an extremely active corridor of commerce for the region.

Left unaddressed, roadway congestion will dramatically increase. Based on traffic forecasts, the average travel time through the corridor for

| Travel Time on the Hwy $\mathbf{6 1}$ Corridor |  |
| :--- | :--- |
| Southbound from $\mathbf{4}^{\text {th }}$ Street to $\mathbf{3 6}^{\text {th }}$ Street |  |
| 2022 | 4.8 Minutes |
| 2040 (without improvements) | 21.9 Minutes | southbound vehicles in the afternoon peak period will more than quadruple. Additionally, Highway 61 acts as a barrier to bike and pedestrian traffic - unsafe crossing locations pose a risk to user safety and will worsen as traffic volumes continue to grow toward 40,000 vehicles per day in 2040. During the five-year period covered in the Corridor Study, there were over 370 crashes reported, resulting in more than $\$ 2.7$ million in damages per year.

Project Description and Benefits: The proposed project, with 2023 estimated construction costs of over \$21 million, will include reconstruction of the corridor for a revitalization of all assets and will be done in conjunction with the City of Hastings to coordinate replacement of critical utilities so that construction disruption is limited. Notable features of the reconstruction include access closures (right-in/rightout restrictions) that enable a new median refuge for crossing pedestrians and bicyclists while calming traffic, new traffic signals, two new roundabouts, and new trail segments to close gaps in the multimodal system. New pavement, stormwater structures, and technology (e.g. lighting) will create a more resilient roadway corridor to serve Hastings and the larger region for decades to come


## HASTINGS HIGHWAY 61 MODERNIZATION

## Existing and Future Traffic Volumes

## Hastings AADT 2019 and 2040

2019: 32,000
2040: 40,000

2019: 28,000
2040: 33,000


## HASTINGS HIGHWAY 61 MODERNIZATION

## Highway 61 Crash Statistics (from Corridor Study)

| Years of Analysis |  |  |
| :--- | :--- | :--- |
| Total Crashes <br> Top Intersections by Total Crashes | 371 | 28 crashes |
|  | Tenth Street | 23 crashes |
|  | Highway 55 | 21 crashes |
|  | Highway 316 | 10 crashes |
| Top Intersections by Injury Crashes | Highway 316 | 7 crashes |
|  | 15th Street | 6 crashes |
|  | Tenth Street | $37 \%$ of crashes |
| Bike/Pedestrian Crashes | Read End | $29 \%$ of crashes |
|  | Angle | $13 \%$ of crashes |

## Safety Data

Crashes over a five year period (2017-2021)


## HASTINGS HIGHWAY 61 MODERNIZATION

Highway 316 Intersection


Future Roundabout at Hwy 316 Intersection


Highway 55 Access Modifications and Turn Lane


## City Hall Area - Pedestrian Crossing Improvements



## HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 1 (4 ${ }^{\text {TH }}$ TO $8^{\text {TH }}$ STREETS)

## A H Hationing

D. $\begin{aligned} & \text { DEPARTMENT OF } \\ & \text { TRANSPORTATION }\end{aligned}$


HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 2 ( $9^{\text {TH }}$ TO $15^{\text {TH }}$ STREETS)
A Hation


HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 3 (15 ${ }^{\text {TH }}$ TO $19^{\text {TH }}$ STREETS)

Mn PEPARTMENT OF


HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 4 (19 ${ }^{\text {TH }}$ TO $24^{\text {TH }}$ STREETS)

M PRERTMEN TOF


## HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 5 (21TT ST TO HWY 316)

## 

M品 PEPARTMENT OF


HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 6 (24TH TO 33RD STREETS)
A Hation
Mn PEPARTMENT OF


## HASTINGS HIGHWAY 61 MODERNIZATION - SHEET 7 (33 ${ }^{\text {RD }}$ TO $36^{\text {TH }}$ STREETS)



MnDOT Metro District

11/29/2023

Ryan Stempski, P.E.
Public Works Director/City Engineer
City of Hastings Public Works
1225 Progress Drive
Hastings, MN 55033

## Re: MnDOT Letter for City of Hastings

Metropolitan Council/Transportation Advisory Board 2024 Regional Solicitation Funding Request for Highway 61 Corridor Improvements

Dear Ryan Stempski,

This letter documents MnDOT Metro District's recognition for the City of Hastings to pursue funding for the Metropolitan Council/Transportation Advisory Board's (TAB) 2024 Regional Solicitation for the Highway 61 Corridor Improvements.

Reconstruction of the US 61 corridor from 36th Street to 4th Street follows the vision plan created in a partnership study between MnDOT and the City. The project will eliminate on-street parking, reduce private and public access, reconstruct a retaining wall, add turn lanes to key intersections, construct roundabouts at the US 61 intersections with 36th Street and with TH 316, construct multiuse trails on both sides from 36th Street to TH 316, and improve pedestrian and biking facilities and crossings.

As the agency with jurisdiction over US 61, MnDOT will allow the City of Hastings to seek improvements proposed in the application. If funded, details of how the project is delivered and any future maintenance agreement with the City will need to be determined during the project's development to define how the improvements will be maintained for the project's useful life.

MnDOT does not anticipate partnering on local projects beyond current agreements. If your project receives funding, continue to work with MnDOT Area staff to coordinate and review needs and opportunities for cooperation.

MnDOT Metro District looks forward to continued cooperation with The City of Hastings as this project moves forward and as we work together to improve safety and travel options within the Metro Area.

If you have questions or require additional information at this time, please reach out to your Area Manager at Bryant.Ficek@state.mn.us or 651-443-2564

Sincerely,

# Sheila Digtatly signed by Sheila Kauppi <br> Kauppi ${ }_{10}^{\text {Dite:2023:3.1.29 }}$ 

Sheila Kauppi, PE
Metro District Engineer
CC:
Bryant Ficek, Metro South Area Manager
Aaron Tag, Metro Program Director
Dan Erickson, Metro State Aid Engineer

December 1, 2023
Ryan Stempski
Public Works Director/City Engineer
City of Hastings Public Works
1225 Progress Drive
Hastings, MN 55033
Dear Mr. Stempski,
I am writing this letter to demonstrate the Prairie Island Indian Community's support for the City of Hastings Trunk Highway 61 Improvements Project. We are supportive of this project and the City's application to the Metropolitan Council's Regional Solicitation funding program.

Trunk Highway 61 is an important part of Southeast Minnesota's transportation network that touches many communities throughout the region. Trunk Highway 61 is a busy corridor that already experiences congestion and vehicle delays during peak hours. The City of Hastings has determined that Trunk Highway 61 will turn to gridlock by 2040 if improvements are not made. Travel time from 4th Street to 36th Street will increase from 5 minutes to over 25 minutes. This delay is unacceptable and would negatively impact Prairie Island business operations at Treasure Island Resort and Casino as well as community events such as Pow Wows.

We support this project as it will reduce traffic congestion and improve safety along Trunk Highway 61. The project will benefit multiple businesses within the Community. The proposed Trunk Highway 61 Improvements are endorsed by the Prairie Island Indian Community, and we are supportive of the City's funding pursuits to construct the project.

Sincerely,
$9+5$
Grant Johnson
Tribal Administrator
Prairie Island Indian Community


[^0]:    1. What engagement methods and tools were used?
    2. How did you engage specific communities and populations likely to be directly impacted by the project?
    3. What techniques did you use to reach populations traditionally not involved in community engagement related to transportation projects?
    4. How were the project?s purpose and need identified?
    5. How was the community engaged as the project was developed and designed?
    6. How did you provide multiple opportunities for of Black, Indigenous, and People of Color populations, low-income populations, persons with disabilities, youth, older adults, and
    residents in affordable housing to engage at different points of project development?
    7. How did engagement influence the project plans or recommendations? How did you share back findings with community and re-engage to assess responsiveness of these changes?
    8. If applicable, how will NEPA or Title VI regulations will guide engagement activities?
[^1]:    Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing No Build
    TAY

[^2]:    Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing Year Build
    TAY

[^3]:    Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing No Build
    TAY

[^4]:    Hastings Highway 61 Modernization TH 61 Corridor 11:11 am 12/11/2023 Existing Year Build
    TAY

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    TAY

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