# Technical Report Traffic 

### 1.0 Introduction

### 1.1 Purpose of Report

This Traffic Technical Report has been prepared in support of the Bottineau Transitway Project Draft Environmental Impact Statement (Draft EIS). The objective of this report is to evaluate the Project's potential traffic impacts within the study area. This includes the following:

- Evaluate the Project's potential impacts on traffic operations at existing and proposed signalized intersections at or adjacent to the transitway.
- Identify proposed roadway improvements to address operational issues identified in the traffic analysis.

A project description and further context for this and all other technical reports can be found in the Project Description Technical Memorandum.

### 2.0 Technical Analysis

### 2.1 Regulatory Context/Methodology

### 2.1.1 Legal and Regulatory Context

There are no specific legal or regulatory requirements governing how traffic impacts are identified, analyzed, and addressed as part of the National Environmental Policy Act (NEPA) review. The analysis of impacts conducted for the Bottineau Transitway Project, and documented in this technical report, adheres to industry standards, as described below in Methodology.

### 2.1.2 Study Area

The analysis of traffic operations for the Bottineau Transitway Project included existing and proposed signalized intersections along the Bottineau Transitway alternative alignments. In addition, several unsignalized crossings of the transitway that are planned to be controlled with automatic gates have been included in the analysis.

### 2.1.3 Methodology

The approach to the traffic operations analysis is derived from the established methodologies documented in the Highway Capacity Manual (HCM). The HCM contains a series of analysis techniques for evaluating the operations of transportation facilities under specified conditions. The Bottineau Transitway traffic models have been developed using Synchro/SimTraffic and VISSIM, software packages that implement the HCM methodologies. The inputs into the models include lane geometrics, existing and forecast turning movement volumes, intersection traffic control devices, and signal timing characteristics. The level of service thresholds, as defined by the HCM, are shown in Table 1. Based on standard practice in the traffic engineering industry, as well as guidance from the American Association of State Highway and Transportation Officials (AASHTO) and conformance with MnDOT and Hennepin County practice, level of service D/E is considered to be the threshold of acceptable level of intersection operations during the peak hour for urban and suburban areas.

Table 1. Intersection Level of Service Definitions

| Level of Service <br> (LOS) | Average Delay (seconds/vehicle) <br> Signalized <br> Intersection |  |
| :---: | :---: | :---: |
|  | $<10$ | Unsignalized <br> Intersection |
| B | $10-20$ | $<10$ |
| C | $20-35$ | $10-15$ |
| D | $35-55$ | $15-25$ |
| E | $55-80$ | $25-35$ |
| F | $>80$ | $35-55$ |

The traffic operations analysis has also incorporated the requirements and standards documented in the Minnesota Manual on Uniform Traffic Control Devices (MN MUTCD) relative to requirements for signal preemption and gate operations.

All full access intersections with the transitway, (i.e., locations where all vehicular movements are allowed) are assumed to be signalized to provide safe movement of transit light rail vehicles (LRV) and motorized vehicles. In addition, at-grade roadway crossings with transit LRV speeds greater than 35 miles per hour (mph) are required to be equipped with automatic gates, based on the MN MUTCD standards.

### 2.2 Environmental Consequences

### 2.2.1 Operating Phase Impacts

## No-Build Alternative

The 2030 No-Build analysis results provide a baseline comparison to determine the impacts of the Bottineau Transitway Project. The intersections shown in Table 2 were chosen based on the capacity or near-capacity operations in the existing conditions and the potential for unacceptable operations in the future (LOS E or F). Therefore, intersections that were shown to have acceptable operations (LOS D or better) in the 2030 Build conditions were not also modeled for the 2030 No-Build conditions since no improvements would be required in the 2030 Build conditions and the 2030 No-Build traffic operations would be expected to be the same or better.

The results of the 2030 No-Build analysis for the PM peak hour are shown in Table 2. For more detailed analysis and results discussion, see the Trunk Highway 55 (Olson Memorial Highway) Corridor and Bottineau Segment D2 Traffic Analysis Technical Memoranda in the Appendix.

Table 2. 2030 No-Build PM Peak -Delay and LOS Results

| Intersection | Operations |  |
| :---: | :---: | :---: |
|  | Delay (sec/veh) | LOS |
| CSAH 81 (Broadway Ave) at CSAH 2 <br> (Penn Ave)/McNair Ave | 84 | F |
| TH 55 at CSAH 2 (Penn Ave) | 150+ | F |
| TH 55 at W Lyndale Ave (I-94 West Ramps) | 29 | C |
| TH 55 at E Lyndale Ave (l-94 East Ramps) | 26 | C |

## Transportation System Management Alternative

A traffic operations analysis was not performed for the Transportation System Management Alternative (TSM) because the TSM alternative is very comparable to the No-Build alternative from a traffic operations perspective. Therefore, a TSM alternative would not provide additional information relative to identifying impacts of the Bottineau Transitway Project.

## Build Alternatives

The analysis approach of traffic impacts resulting from the Build Alternatives is discussed in the following sections for each segment of the alignment. Intersections identified as operating with transit priority would allow an approaching LRV to influence the signal timing (e.g., extended green phase), but no signal phases would be skipped. Intersections identified as operating with preemption indicates that an approaching LRV would never stop at the intersection, which would result in some signal phases being skipped during a preemption event, in order to provide for the LRT phase. The focus of the operations analysis was the PM peak hour because it has the highest volumes and would therefore represent a worst-case condition. For further discussion of priority and preemption operations, see the Transit/Traffic Signal Operating Schemes Technical Memorandum in the Appendix.

## Alignment A

A summary of the Alignment A modeling assumptions is shown in Table 3. ${ }^{1}$
Table 3. Alignment A Modeling Summary

| Intersection | No-Build Conditions | Build Conditions <br>  <br> Arbor Lakes Pkwy <br> Speed <br> (mph) | Intersection <br> Operations |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 35 | Signalized; <br> Transit <br> Priority | Proposed parkway assumed <br> to be designed with adequate <br> geometrics to accommodate <br> future transit operations; <br> Not included in modeling. |  |
| Arbor Lakes Pkwy <br> at Jefferson Hwy | Arbor Lakes Parkway <br> construction | 35 | Signalized; <br> Transit <br> Priority | Proposed parkway assumed <br> to be designed with adequate <br> geometrics to accommodate <br> future transit operations; <br> Not included in modeling. |
| CSAH 130 <br> (Brooklyn Blvd) at <br> Northland Dr | Signalized; <br> No Changes | 35 | Signalized; <br> Transit <br> Priority | Left-turn lanes added on <br> CSAH 130 (Brooklyn Blvd); <br> Low traffic volumes; <br> Not included in modeling. |
| CSAH 130 <br> (Brooklyn Blvd) at <br> Boone Ave | Signalized; <br> No Changes | 35 | Signalized; <br> Transit <br> Priority | Left-turn lanes added on <br> CSAH 130 (Brooklyn Blvd). |

[^0]Table 3 continued. Alignment A Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| CSAH 81 <br> (Bottineau Blvd) at CSAH 130 <br> (Brooklyn Blvd) | Signalized; <br> Through lanes added on CSAH 81 (Bottineau Blvd); <br> Second left-turn lanes added on northbound, eastbound, and westbound approaches | 55 | Signalized; Grade Separated Transitway |  |
| CSAH 81 <br> (Bottineau Blvd) at 73rd Ave | Signalized; Through lanes added on CSAH 81 (Bottineau BIvd) | 55 | Signalized; Preemption |  |
| CSAH 81 <br> (Bottineau Blvd) at 71st Ave/ CSAH 8 (Broadway Ave) | Signalized; <br> Through lanes added on CSAH 81 (Bottineau <br> Blvd) and westbound 71st Ave; <br> Second left-turn lanes added on southbound and eastbound approaches; <br> Right-turn lane added on westbound approach | 55 | Signalized; Preemption |  |

The results of the 2030 analysis for the PM peak hour are shown in Table 4. The CSAH 81 (Bottineau Boulevard)/CSAH 130 (Brooklyn Boulevard) intersection is expected to operate at or near capacity (LOS E). However, this is not due to any effect caused by the operations of the Bottineau Transitway Project because the transitway is proposed to be grade-separated over CSAH 81 (Bottineau Boulevard)/CSAH 130 (Brooklyn Boulevard), which will eliminate any potential influence of transit operation on overall intersection operations at this location. The grade-separated transitway crossing of CSAH 130 (Brooklyn Boulevard) at CSAH 81 (Bottineau Boulevard) is warranted because this intersection is expected to operate near unacceptable operations even with significant capacity improvements as part of the CSAH 81 (Bottineau Boulevard) reconstruction project. The other intersections analyzed are expected to have overall acceptable operations during the peak hour. For more detailed analysis and results discussion, see the Bottineau Alignment A Traffic Analysis Memorandum in the Appendix.

Table 4. 2030 Build PM Peak - Alignment A Delay and LOS Results

| Intersection | Operations |  | Comments |  |
| :--- | :---: | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) |  |  |
| CSAH 130 (Brooklyn <br> Blvd) at Boone Ave | Transit <br> Priority | 41 | D |  |
| CSAH 81 (Bottineau <br> Blvd) at CSAH 130 <br> (Brooklyn Blvd) | Grade <br> Separated <br> Transitway | 60 | E | No impact due to grade separation of <br> the transitway over CSAH 130 <br> (Brooklyn BIvd) |
| CSAH 81 (Bottineau <br> Blvd) at 73rd Ave | Preemption | 31 | C |  |
| CSAH 81 (Bottineau <br> Blvd) at 71st Ave/ <br> CSAH 8 (Broadway Ave) | Preemption | 50 | D |  |

## Alignment B

Alignment B includes CSAH 103 (Broadway Avenue), which is currently in the planning stages for a roadway reconstruction project. The CSAH 103 (Broadway Avenue) corridor roadway project is a planned improvement by Hennepin County, separate from the Bottineau Transitway Project between CSAH 30 (93rd Avenue) and Candlewood Drive. The proposed improvement includes expanding the roadway from a two-lane undivided to a four-lane divided section, with a median wide enough to accommodate the LRT transitway. A date for the start of construction on the CSAH 103 (Broadway Avenue) has not been established, but it has been assumed that the project would be completed before the start of construction on the Bottineau Transitway project.

A summary of the Alignment B modeling assumptions is shown in Table 5. All modeling was completed in VISSIM based on the preemption operations.

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Table 5. Alignment B Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| CSAH 103 (Broadway Ave) at TH 610 North Ramps | Signalized; No Changes | 45 | Signalized; Transitway does not cross any roadway approaches | Transitway does not cross or impact the ramp operations; Not included in modeling. |
| CSAH 103 (Broadway Ave) at TH 610 South Ramps | Signalized; No Changes | 45 | Signalized; <br> Transitway does not cross any roadway approaches | Transitway does not cross or impact the ramp operations; Not included in modeling. |
| CSAH 103 (Broadway Ave) at 94th Ave | Signalized; No Changes | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at CSAH 30 (93rd Ave) | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Setzler Pkwy | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 89th Ave | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Maplebrook Pkwy | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption | Low traffic volumes; Not included in modeling. |
| CSAH 103 (Broadway Ave) at CSAH 109 (85th Ave) | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 84th Ave | Unsignalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Unsignalized; Right-In/Right-Out Only | No vehicular movements across the transitway; <br> Not included in modeling |

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Table 5 continued. Alignment B Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| CSAH 103 (Broadway Ave) at College Park Drive | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Candlewood Dr | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Shopping Center Driveway | Signalized | 45 | Unsignalized; Right-In/ Right-Out Only | No vehicular movements across the transitway; Not included in modeling |
| CSAH 103 (Broadway Ave) at CSAH 152 (Brooklyn Blvd) | Signalized | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 76th Ave | Signalized | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 75th Ave | Unsignalized | 45 | Signalized; Preemption | Low traffic volumes; Not included in modeling. |
| Transitway at Jolly La | N/A | 45 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |
| Transitway at Lakeland Ave | N/A | 45 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |
| CSAH 103 (Broadway Ave) at 73rd Ave | Signalized; Through lanes added on CSAH 81 (Bottineau Blvd) | 45 | Signalized; Preemption |  |
| CSAH 81 (Bottineau Blvd) at 71st Ave/CSAH 8 (Broadway Ave) | Signalized; <br> Through lanes added on CSAH 81 <br> (Bottineau Blvd) and westbound 71st Ave; Second left-turn lanes added on southbound and eastbound approaches; Rightturn lane added on westbound approach | 45 | Signalized; Preemption | Analyzed as part of Alignment A with the same roadway and transitway configuration; Not reanalyzed as part of Alignment B. |

The modeling results for Alignment B are shown in Table 6. For more detailed analysis and results discussion, see the CSAH 103 (Broadway Avenue) Traffic Study Travel Demand Forecasts Technical Memorandum and the Broadway Avenue Modeling Assumptions Technical Memorandum in the Appendix.

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Table 6. 2030 Build PM Peak - Alignment B Delay and LOS Results

| Intersection | Operations |  |  |
| :--- | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) | LOS |
| CSAH 103 (Broadway Ave) at 94th Ave | Preemption | 28 | C |
| CSAH 103 (Broadway Ave) at CSAH 30 <br> (93rd Ave) | Preemption | 42 | D |
| CSAH 103 (Broadway Ave) at Setzler <br> Pkwy | Preemption | 17 | B |
| CSAH 103 (Broadway Ave) at CSAH 109 <br> (85th Ave) | Preemption | 47 | D |
| CSAH 103 (Broadway Ave) at College <br> Park Dr | Preemption | 22 | C |
| CSAH 103 (Broadway Ave) at <br> Candlewood Dr | Preemption | 17 | B |
| CSAH 103 (Broadway Ave) at CSAH 152 <br> (Brooklyn Blvd) | Preemption | 53 | D |
| CSAH 103 (Broadway Ave) at 76th Ave | Preemption | 28 | C |
| CSAH 81 (Bottineau BIvd) at 73rd Ave | Preemption | 12 | B |

## Alignment C

The operational analysis for Alignment C was completed using VISSIM. A summary of the Alignment C modeling assumptions is shown in Table 7.

Table 7. Alignment C Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| CSAH 81 <br> (Bottineau Blvd) at I-94 North Ramps | Signalized; <br> Through lanes added on CSAH 81 (Bottineau Blvd); Second left-turn lane added on southbound and westbound approaches; Second right-turn lane added on westbound approach | 55 | Signalized; Transitway does not cross any roadway approaches | Transitway does not cross or impact the ramp operations; Not included in modeling. |
| CSAH 81 <br> (Bottineau Blvd) at I-94 South Ramps | Signalized; <br> Through lanes added on CSAH 81 (Bottineau Blvd); Second left-turn lane added on southbound and westbound approaches | 55 | Signalized; Transitway does not cross any roadway approaches | Transitway does not cross or impact the ramp operations; Not included in modeling. |

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Table 7 continued. Alignment C Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| CSAH 81 <br> (Bottineau Blvd) at 63rd Ave | Signalized; <br> Second left-turn lane added on southbound, eastbound, and westbound approaches; <br> Through lanes added on CSAH 81 (Bottineau Blvd); Right-turn lanes added on 63rd Ave | 55 | Signalized; Preemption |  |
| $\text { CSAH } 81$ <br> (Bottineau Blvd) at CSAH 10 (Bass Lake Rd) | Signalized; <br> Second left-turn lane and additional through lane added both directions on CSAH 81 (Bottineau Blvd) | 55 | Signalized; Preemption |  |
| Transitway at Corvallis Ave | Unsignalized BNSF Crossing; <br> No Changes | 55 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |
| Transitway at CSAH 8 (Broadway Ave) | Unsignalized BNSF Crossing; <br> No Changes | 55 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |
| Transitway at 45½ Ave | Unsignalized BNSF Crossing; <br> No Changes | 55 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |
| Transitway at TH 100 | N/A | 55 | Grade Separated Transitway | Transitway crossing grade separated; <br> Not included in modeling. |
| Transitway at CSAH 9 (42nd Ave) | Unsignalized BNSF Crossing; <br> No Changes | 55 | Unsignalized; Automatic Gates | Included in modeling due to concerns of proximity to adjacent traffic signal |
| CSAH 9 (42nd Ave) at CSAH 8 <br> (Broadway Ave) | Signalized; No Changes | N/A | Signalized; No Changes | Signalized intersection is approximately 530 feet from Transitway |
| Transitway at 41st Ave/Noble Ave | Unsignalized BNSF Crossing; <br> No Changes | 55 | Unsignalized; Automatic Gates | Low traffic volumes; Not included in modeling. |

The results of the 2030 analysis for the PM peak hour are shown in Table 8. For more detailed analysis and results discussion, see the Bottineau Alignment C Traffic Analysis Memorandum in the Appendix.

Table 8. 2030 Build PM Peak - Alignment C Delay and LOS Results

| Intersection | Operations |  |  |
| :--- | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) | LOS |
| CSAH 81 (Bottineau Blvd) at 63rd Ave | Preemption | 53 | D |
| CSAH 81 (Bottineau BIvd) at CSAH 10 <br> (Bass Lake Rd) | Preemption | 29 | C |
| CSAH 9 (42nd Avenue) at Transitway | Unsignalized; <br> Automatic Gates | 2 | A |

The queues at the CSAH 9 (42nd Avenue) at CSAH 8 (Broadway Avenue) intersection were also evaluated to determine whether there would be any safety issues due to vehicle queues from the signal extending to the at-grade Transitway crossing. The modeling showed that the maximum eastbound queue on CSAH 9 (42nd Avenue) from the CSAH 8 (Broadway Avenue) intersection would be expected to be approximately 210 feet compared to a storage distance of 350 feet. Therefore, no impacts are expected due to the Bottineau Transitway and no improvements are proposed.

## Alignment D1

The operational analysis for Alignment D1 was completed using Synchro/SimTraffic. A summary of the Alignment D1 modeling assumptions is shown in Table 9.

Table 9. Alignment D1 Modeling Summary

| Intersection | No-Build | Build Conditions <br> Conditions |  | Transitway <br> Speed <br> (mph) |
| :---: | :---: | :---: | :---: | :---: |
|  | N/A | Intersection <br> Operations | Comments |  |
| TH 55 at <br> Thomas Ave | Unsignalized; <br> No Changes | 35 | Grade <br> Separated <br> Transitway | Transitway crossing grade <br> separated; |
| TH 55 at included in modeling. |  |  |  |  |
| TSAH 2 (Penn <br> Ave) | Signalized; <br> No Changes | 35 | Signalized; <br> Transit <br> Priority | Low traffic volumes; <br> Not included in modeling. |
| Nignalized; | Median modifications, approach <br> alignment, and striping <br> Transit <br> Priority | improvements assumed on CSAH 2 <br> (Penn Ave). |  |  |

The results of the 2030 analysis for the PM peak hour are shown in Table 10. Although the TH $55 /$ CSAH 2 (Penn Avenue) intersection is expected to operate at LOS E, this would actually be an improvement over the 2030 No-Build conditions (as summarized in Table 2) due to significantly less delay. The improvement in intersection operations is due to intersection geometric improvements proposed as part of the Bottineau Transitway Project that allow the northbound/southbound phases to operate concurrently, rather than split phased. The intersection geometric improvements are proposed to include median modifications, realignment of the northbound and southbound approach lanes, and additional striping to guide left-turning vehicles through the intersection.

Table 10. 2030 Build PM Peak - Alignment D1 Delay and LOS Results

| Intersection | Operations |  |  |
| :--- | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) | LOS |
| TH 55 at CSAH 2 (Penn Ave) | Priority | 60 | E |

## Alignment D2

The operational analysis for Alignment D2 was completed using Synchro/SimTraffic and VISSIM. Leftturn movements along the alignment were assumed to be prohibited where left-turn lanes could not be provided, which occurred at the following intersections:

- CSAH 81 (Broadway Avenue) and 29th Avenue (approximately 60 left-turn vehicles in the PM peak hour)
- CSAH 81 (Broadway Avenue) and 26th Avenue (approximately 80 left-turn vehicles in the PM peak hour)
- CSAH 81 (Broadway Avenue) and Penn Avenue (CSAH 2) - eastbound only (approximately 30 leftturn vehicles in the PM peak hour)

As a result of the reduced capacity and left-turn restrictions on CSAH 81 (Broadway Avenue), some drivers may choose to divert to other routes. The Metropolitan Council travel demand model was used to determine the magnitude of expected traffic diversions in 2030 due to the capacity reductions and turn restrictions. The diversion is forecast to be approximately 20 percent of through traffic along CSAH 81 (Broadway Avenue) and CSAH 2 (Penn Avenue). The travel demand model shows that approximately 500 to 1,000 vehicles per day would be expected to divert from CSAH 81 (Broadway Avenue) and CSAH 2 (Penn Avenue) to sections of TH 100, I-94, Lowry Avenue, and CSAH 2 (Penn Avenue) north of CSAH 81 (81).Broadway Avenue). A summary of the Alignment D2 modeling assumptions is shown in Table 11.

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Table 11. Alignment D2 Intersections/Segments Analysis Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| France Ave/Oakdale Ave at 34th Ave | Unsignalized; No Changes | 35 | Signalized; Transit Priority |  |
| CSAH 81 (Bottineau Blvd) at Abbott Ave | Signalized; No Changes | 35 | Signalized; Transit Priority |  |
| CSAH 81 (Broadway Ave) at Victory Memorial Parkway/CSAH 153 (Lowry Avenue) | Signalized; No Changes | 30 | Signalized; Grade Separated Transitway | Transitway crossing is grade separated; Not included in modeling. |
| CSAH 81 (Broadway Ave) at 29th Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | CSAH 81 (Broadway Ave) reduced to one lane in each direction and left turn movements banned. |
| CSAH 81 (Broadway Ave) at 26th Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | CSAH 81 (Broadway Ave) reduced to one lane in each direction and left turn movements banned. |
| CSAH 81 (Broadway Ave) CSAH 2 (Penn Ave) | Signalized; No Changes | 30 | Signalized; Transit Priority | West leg of CSAH 81 (Broadway Ave) reduced to one lane in each direction and eastbound left turn movement banned; McNair Ave approach closed. |
| CSAH 2 (Penn Ave) at Golden Valley Rd | Signalized; No Changes | 30 | Signalized; <br> Transit Priority | Left turn lanes added on CSAH 2 (Penn Ave) |
| CSAH 2 (Penn Ave) at 16th Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | Left turn lanes added on CSAH 2 (Penn Ave); <br> Low traffic volumes; Not included in modeling. |
| CSAH 2 (Penn Ave) at 14th Ave | Signalized; No Changes | 30 | Unsignalized; Right-In/ Right-Out Only | No vehicular movements across the transitway; <br> Not included in modeling |
| CSAH 2 (Penn Ave) at Plymouth Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | Left turn lanes added on CSAH 2 (Penn Ave) |
| CSAH 2 (Penn Ave) at 12th Ave | Signalized; No Changes | 30 | Unsignalized; Right-In/ Right-Out Only | No vehicular movements across the transitway; <br> Not included in modeling |
| CSAH 2 (Penn Ave) at Oak Park Ave | Signalized; No Changes | 30 | Unsignalized; Right-In/ Right-Out Only | Left turn lanes added on CSAH 2 (Penn Ave); Low traffic volumes; Not included in modeling. |
| CSAH 2 (Penn Ave) at TH 55 | Signalized; No Changes | 35 | Signalized; <br> Transit Priority | Median modifications, approach alignment, and striping improvements assumed on CSAH 2 (Penn Ave). |

The results of the 2030 analysis for the PM peak hour are shown in Table 12. The TH 55/CSAH 2 (Penn Avenue) intersection is expected to operate at LOS E, but with significantly less delay than in the 2030 No-Build conditions (see Table 2). The improvement in operations is due to intersection geometric improvements that allow the northbound/southbound phases to operate concurrently, rather than split phased. These improvements include median modification, realignment of approach lanes, and additional striping. For more detailed analysis and results discussion, see the Bottineau Segment D2 Traffic Analysis Memorandum in the Appendix.

Table 12. 2030 Build PM Peak - Alignment D2 Delay and LOS Results

| Intersection | Operations |  |  |
| :--- | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) | LOS |
| France Ave/Oakdale Ave at 34th Ave | Priority | 11 | B |
| CSAH 81 (Broadway Ave) at 29th Ave | Priority | 7 | A |
| CSAH 81 (Broadway Ave) at 26th Ave | Priority | 19 | B |
| CSAH 81 (Broadway Ave) at CSAH 2 <br> (Penn Ave) | Priority | 56 | E |
| CSAH 2 (Penn Ave) at Golden Valley Rd | Priority | 32 | C |
| CSAH 2 (Penn Ave) at Plymouth Ave | Priority | 49 | D |
| CSAH 2 (Penn Ave) at TH 55 | Priority | 79 | E |

## Alignment D Common Section

The analysis of the intersections in the Alignment D Common Section was completed using VISSIM. Analysis of the intersections on TH 55 from Van White Memorial Parkway through the I-94 ramps is currently being discussed with MnDOT. This discussion includes operation of the TH 55/l-94 ramp intersections with one controller instead of two, and any queuing issues that might impact freeway operations. The operational results for these intersections will be incorporated when available. A summary of the Alignment D Common Section modeling assumptions is shown in Table 13.

Table 13. Alignment D Common Section Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| TH 55 at Morgan Ave | Signalized; No Changes | 35 | Signalized; <br> Transit <br> Priority | Low traffic volumes; Not included in modeling. |
| TH 55 at Humboldt Ave | Signalized; No Changes | 35 | Signalized; <br> Transit <br> Priority | Low traffic volumes; Not included in modeling. |
| TH 55 at Van White Memorial Pkwy | Signalized; No Changes | 35 | Signalized; <br> Transit <br> Priority |  |
| TH 55 at Bryant Ave | Signalized; No Changes | 30 | Signalized; <br> Transit <br> Priority |  |
| TH 55 at W Lyndale Ave (I-94 West Ramps) | Signalized; No Changes | 30 | Signalized; <br> Transit <br> Priority | One through lane removed in each direction on TH 55. |
| TH 55 at W Lyndale Ave (I-94 West Ramps) | Signalized; No Changes | 30 | Signalized; Transit Priority | One through lane removed in each direction on TH 55. |

## Alignment D at The Interchange

As part of the modeling analysis for The Interchange project, four alternatives were analyzed for the connection of the Southwest and Bottineau Transitways into The Interchange. The results of the analysis showed that acceptable traffic operations (LOS D or better) could be achieved at the TH 55/7th Street/6th Avenue intersection with one LRT line crossing the intersection at-grade, but that some roadway improvements would be necessary. The modeling showed that at-grade crossings of both LRT lines would result in unacceptable traffic operations, therefore this alternative was not considered to be feasible. The design of The Interchange project was based on the following considerations:

- Track elevations approaching and on The Interchange site
- LRT platform locations and grades
- Location of the LRT crossings relative to the TH 55/7th Street/6th Avenue intersection
- Safety of LRT crossings relative to LRT/vehicle conflicts and control at crossings
- Overall traffic operations at the TH 55/7th Street/6th Avenue intersection

The Interchange preferred alternative provides for a Southwest Transitway crossing of 7th Street either above or below grade and a Bottineau Transitway crossing of the TH 55/7th Street/6th Avenue intersection either at-grade or above grade. The Environmental Assessment for The Interchange documents in detail the stakeholder input and public process that was used to identify the preferred alternative for The Interchange.

As part of the Bottineau Transitway Project, the analysis was carried forward to identify the roadway improvements needed to accommodate an at-grade crossing of the TH 55/7th Street/6th Avenue intersection since this would be the worst case scenario from a traffic operations perspective. A summary of the modeling assumptions for the connection to The Interchange is shown in Table 14.

DRAFT ENVIRONMENTAL IMPACT STATEMENT

Table 14. Alignment $D$ at The Interchange Modeling Summary

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| TH 55 at Border Ave/ Oak Lake Ave | No Change |  | Signalized; Transit Priority |  |
| TH 55/6th Ave at 7th St | Grade Separated Southwest Transitway Crossing; Construction of The Interchange |  | Signalized; <br> Transit Priority; Roadway widening on 7th Street to provide 2 exclusive northbound left-turn lanes and 1 southbound left-turn lane |  |
| 6th Ave at Bradford Ave/HERC Driveway | Grade Separated Southwest Transitway Crossing; Construction of The Interchange |  | Signalized; Grade Separated Southwest and Bottineau Transitways Crossings; Construction of The Interchange | Grade separated crossings; not included in modeling. |

The results of the 2030 analysis for the PM peak hour are shown in Table 15. For more detailed analysis and results discussion, see The Interchange Traffic Technical Memorandum \#3 and the TH $55 / 7^{\text {th }}$ Street $/ 6^{\text {th }}$ Avenue Intersection Traffic Analysis Memorandum in the Appendix.

Table 15. 2030 Build PM Peak - Alignment D at The Interchange Delay and LOS Results

| Intersection | Operations |  |  |
| :--- | :---: | :---: | :---: |
|  | Operating <br> Scheme | Delay <br> (sec/veh) | LOS |
| TH 55 at Border Ave/Oak Lake Ave | Priority | 20 | C |
| TH 55/6th Ave at 7th St | Priority | 38 | D |

## Summary of Impacts by Alternative

A summary of the critical assumptions in each alignment is shown in Table 16.
Table 16. Modeling Summary for Each Alignment

| Intersection | Build Conditions <br>  |  | No-Build Conditions | Transitway <br> Speed <br> $(\mathrm{mph})$ |
| :---: | :---: | :---: | :---: | :---: | | Intersection |
| :---: |
| Operations |$\quad$ Comments

## BottineauTransitway

DRAFT ENVIRONMENTAL IMPACT STATEMENT

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| Alignment A |  |  |  |  |
| Arbor Lakes Pkwy at Jefferson Hwy | Arbor Lakes Parkway construction | 35 | Signalized; <br> Transit Priority | Proposed parkway assumed to be designed with adequate geometrics to accommodate future transit operations; Not included in modeling. |
| CSAH 81 (Bottineau <br> Blvd) at CSAH 130 <br> (Brooklyn Blvd) | Signalized; <br> Through lanes added on CSAH 81 (Bottineau Blvd); <br> Second left-turn lanes added on northbound, eastbound, and westbound approaches | 55 | Signalized; Grade <br> Separated Transitway |  |
| CSAH 81 (Bottineau Blvd) at 73rd Ave | Signalized; Through lanes added on CSAH 81 (Bottineau BIvd) | 55 | Signalized; Preemption |  |
| CSAH 81 (Bottineau Blvd) at 71st Ave/ CSAH 8 (Broadway Ave) | Signalized; <br> Through lanes added on CSAH 81 (Bottineau <br> Blvd) and westbound 71st Ave; <br> Second left-turn lanes added on southbound and eastbound approaches; <br> Right-turn lane added on westbound approach | 55 | Signalized; Preemption |  |

Table 16 continued. Modeling Summary for Each Alignment

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \text { Transitway } \\ & \text { Speed } \\ & \text { (mph) } \\ & \hline \end{aligned}$ | Intersection Operations |  |
| Alignment B |  |  |  |  |
| CSAH 103 (Broadway Ave) at CSAH 30 (93rd Ave) | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Setzler Pkwy | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 89th Ave | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at CSAH 109 (85th Ave) | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at College Park Drive | Signalized; Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at Candlewood Dr | Signalized; <br> Through lanes added on CSAH 103 (Broadway Ave) | 45 | Signalized; Preemption |  |
| CSAH 103 (Broadway Ave) at 73rd Ave | Signalized; Through lanes added on CSAH 81 (Bottineau Blvd) | 45 | Signalized; Preemption |  |
| Alignment C |  |  |  |  |
| CSAH 81 <br> (Bottineau Blvd) at 63rd Ave | Signalized; <br> Second left-turn lane added on southbound, eastbound, and westbound approaches; Through lanes added on CSAH 81 (Bottineau Blvd); <br> Right-turn lanes added on 63rd Ave | 55 | Signalized; Preemption |  |
| CSAH 81 <br> (Bottineau Blvd) at CSAH 10 (Bass Lake Rd) | Signalized; Second left-turn lane and additional through lane added both directions on CSAH 81 (Bottineau Blvd) | 55 | Signalized; Preemption |  |

Table 16 continued. Modeling Summary for Each Alignment

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway Speed (mph) | Intersection Operations |  |
| Alignment D1 |  |  |  |  |
| $\begin{gathered} \text { TH } 55 \text { at } \\ \text { CSAH } 2 \text { (Penn Ave) } \end{gathered}$ | Signalized; No Changes | 35 | Signalized; <br> Transit Priority | Median modifications, approach alignment, and striping improvements assumed on CSAH 2 (Penn Ave). |
| Alignment D2 |  |  |  |  |
| CSAH 81 (Broadway Ave) at 29th Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | CSAH 81 (Broadway Ave) reduced to one lane in each direction and left turn movements banned. |
| CSAH 81 (Broadway Ave) at 26th Ave | Signalized; No Changes | 30 | Signalized; <br> Transit Priority | CSAH 81 (Broadway Ave) reduced to one lane in each direction and left turn movements banned. |
| CSAH 81 (Broadway Ave) at CSAH 2 (Penn Ave) | Signalized; No Changes | 30 | Signalized; Transit Priority | West leg of CSAH 81 <br> (Broadway Ave) reduced to one lane in each direction and eastbound left turn movement banned; McNair Ave approach closed. |
| CSAH 2 (Penn Ave) at Golden Valley Rd | Signalized; No Changes | 30 | Signalized; Transit Priority | Left turn lanes added on CSAH 2 (Penn Ave) |
| CSAH 2 (Penn Ave) at Plymouth Ave | Signalized; No Changes | 30 | Signalized; Transit Priority | Left turn lanes added on CSAH 2 (Penn Ave) |
| CSAH 2 (Penn Ave) at TH 55 | Signalized; No Changes | 35 | Signalized; Transit Priority | Median modifications, approach alignment, and striping improvements assumed on CSAH 2 (Penn Ave). |
| Alignment D Common Section |  |  |  |  |
| TH 55 at W Lyndale Ave (I-94 West Ramps) | Signalized; No Changes | 30 | Signalized; Transit Priority | One through lane removed in each direction on TH 55. |
| TH 55 at W Lyndale Ave (I-94 West Ramps) | Signalized; No Changes | 30 | Signalized; Transit Priority | One through lane removed in each direction on TH 55. |

Table 16 continued. Modeling Summary for Each Alignment

| Intersection | No-Build Conditions | Build Conditions |  | Comments |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Transitway <br> Speed <br> (mph) | Intersection Operations |  |
| Alignment D at The Interchange |  |  |  |  |
| TH 55/6th Ave at 7th St | Grade Separated Southwest Transitway Crossing; Construction of The Interchange |  | Signalized; <br> Transit Priority; <br> Roadway widening on 7th Street to provide 2 exclusive northbound left-turn lanes and 1 southbound left-turn lane |  |

The summary of intersections expected to operate at LOS E or LOS F operations in the 2030 PM peak hour is found in Table 17 In general, all intersections are expected to have acceptable operations under any of the build alternatives. The LOS E/F operations at the CSAH 81 (Broadway Ave)/CSAH 2 (Penn Ave) and CSAH 2 (Penn Ave)/TH 55 intersections during the peak hour would be expected to occur in 2030 even if the Bottineau Transitway Project is not constructed.

Table 17. Impacts By Alternative

| Alternative | Intersections Expected to Operate at LOS E/F |
| :--- | :--- |
| No-Build Alternative | CSAH 81 (Broadway Ave) at CSAH 2 (Penn Ave) <br> CSAH 2 (Penn Ave) at TH 55 |
| TSM Alternative | No impacts |
| Alternative A-C-D1 | CSAH 2 (Penn Ave) at TH 55 |
| Alternative A-C-D2 | CSAH 81 (Broadway Ave) at CSAH 2 (Penn Ave) <br> CSAH 2 (Penn Ave) at TH 55 |
| Alternative B-C-D1 | CSAH 2 (Penn Ave) at TH 55 <br> Alternative B-C-D2CSAH 81 (Broadway Ave) at CSAH 2 (Penn Ave) <br> CSAH 2 (Penn Ave) at TH 55 |

### 2.2.2 Construction Phase Impacts

Construction phase impacts were not explicitly evaluated relative to traffic operations. It is anticipated that construction of the transitway under any of the alternatives would result in traffic disruption, including lane closures, short-term intersection and roadway closures, and detours that would cause localized increases in congestion.

### 2.2.3 Indirect/Secondary Impacts

## No-Build Alternative

No indirect or secondary traffic impacts are expected relative to traffic operations as anticipated
under the No-Build alternative.

## Transportation System Management Alternative

No indirect or secondary traffic impacts are anticipated under the Transportation System Management alternative.

Build Alternatives
No indirect or secondary traffic impacts are anticipated under any of the Build alternatives.

### 2.3 Avoidance, Minimization, and/or Mitigation Measures

In general, all intersections are expected to have acceptable operations under any of the build alternatives. The CSAH 81 (Broadway Avenue)/CSAH 2 (Penn Avenue) and TH 55/CSAH 2 (Penn Avenue) intersections are expected to operate at LOS F under the 2030 No-Build conditions (without the project), therefore no mitigation is required as a result of the Bottineau Transitway project.

The TH 55/7th Street/6th Avenue intersection would require geometric improvements to maintain acceptable LOS operations. $7^{\text {th }}$ St would need to be widened to construct a second exclusive northbound left-turn lane, providing additional capacity to maintain acceptable LOS operations with the projected traffic growth.

No permits relative to traffic operations are required. Operation of the traffic signals along the transitway alignment will continue to be under the jurisdiction of the roadway authority (e.g., city or Hennepin County).

No specific construction mitigation is proposed. The details of construction staging would be developed in future stages of project design. Maintenance of traffic (MOT) plans would be required to be developed by the contractor, and submitted for approval to the roadway authorities. The MOT plans would address construction phasing, maintenance of traffic during construction of the transitway, any road closures, and any traffic detours.

### 3.0 Cumulative Impacts

The convergence of the Southwest and Bottineau Transitways at The Interchange, and the potential impacts on traffic operations, have previously been analyzed as part of The Interchange project. In addition, the impacts of the completion of Van White Memorial Boulevard on traffic operations were previously analyzed during the Alternatives Analysis of the Bottineau Transitway. No other cumulative impacts relative to traffic operations are expected.

### 4.0 Summary

The results of the analysis of traffic operations as part of the Bottineau Transitway Project are summarized in Table S-1 and S-2. In general, all intersections are expected to have acceptable operations under any of the build alternatives. The CSAH 81 (Broadway Avenue)/CSAH 2 (Penn Avenue) and TH 55/CSAH 2 (Penn Avenue) intersections are expected to operate at LOS F under the 2030 No-Build conditions (without the project), therefore, beyond the geometric improvements at those two intersections, no additional mitigation is proposed as a result of the Bottineau Transitway Project.

Table S-1. Summary of Impacts and Mitigation Measures

| Impact Category | Impacts of Build Alternatives | Avoidance, Minimization, and/or <br> Mitigation Measures |
| :--- | :--- | :--- |
| Traffic | Alternatives A-C-D1 and B-C-D1 <br> The TH 55/CSAH 2 (Penn <br> Avenue) intersection is expected <br> to operate at LOS F in the 2030 <br> PM peak hour. | No mitigation is proposed - this is <br> the same as the No-Build impact. |
|  | Alternatives A-C-D2 and B-C-D2 <br> The CSAH 81 (Broadway <br> Avenue)/ CSAH 2 (Penn Avenue) <br> and TH 55/CSAH 2 (Penn <br> Avenue intersections are <br> expected to operate at LOS E/F <br> in the 2030 PM peak hour. | No mitigation is proposed - this is <br> the same as the No-Build impact. |
| All alternatives <br> The TH 55/7th Street/6th <br> Avenue intersection would <br> require geometric <br> improvements to maintain LOS <br> D operations. | Widen roadway to construct second <br> northbound left-turn lane on 7th <br> Street. |  |

Table S-2. Summary of Construction Impacts and Mitigation Measures

| Impact Category | Construction Impacts of Build <br> Alternatives | Avoidance, Minimization, and/or <br> Mitigation Measures |
| :--- | :--- | :--- |
| Traffic | Construction of the transitway <br> under any of the alternatives <br> would result in traffic disruption <br> including lane closures, short- <br> term intersection and roadway <br> closures, detours, and traffic <br> diversion that would likely cause <br> localized increases in <br> congestion and delay. | No specific mitigation is proposed. <br> The details of construction staging <br> would be developed in future <br> stages of project design. |

## APPENDIX

## Traffic Technical Memoranda

- Bottineau Transitway Traffic Study Report, Trunk Highway 55 (OIson Memorial Highway) Corridor, WSB and Associates, November 10, 2009
- Operations Analysis of LRT near the Terrace Mall, WSB and Associates, June 2011
- Operations Analysis of LRT and BRT at the 42nd Avenue Crossing, WSB and Associates, June 2011
■ The Interchange, 2030 Build Analysis Update Traffic Technical Memorandum \#3, Kimley-Horn and Associates, August 22, 2011
- Bottineau Segment D2 Traffic Analysis, Kimley-Horn and Associates, September 19, 2011
- Transit/Traffic Signal Operating Schemes Technical Memorandum, Kimley-Horn and Associates, December 6, 2011
- TH 55/7th Street/6th Avenue Intersection Traffic Analysis, Kimley-Horn and Associates, February 16, 2012
- CSAH 103 (Broadway Avenue) Traffic Study Travel Demand Forecasts: Four-Lane and Two-Lane Alternatives, SRF Consulting Group, April 11, 2012
- CSAH 81 (Brooklyn Boulevard) Transit Headway Analysis, Kimley-Horn and Associates, May 24, 2012
- Bottineau Segment A Traffic Analysis, SRF Consulting Group, June 18, 2012
- Bottineau Segment C Traffic Analysis, SRF Consulting Group, June 18, 2012
- Modeling Assumptions Bottineau LRT Transitway - Broadway Avenue, WSB and Associates, October 2012


## Study Report

To: Joe Gladke, PE<br>Manager of Engineering and Transit Planning<br>Hennepin County<br>417 N. 5th Street North, Suite 320<br>Minneapolis, MN 55401

From: Chad Ellos, PE cc: Ross Jentink, PE<br>Anthony Heppelmann, PE

Date: November 10, 2009
Re: Bottineau Transitway Traffic Study Report
Trunk Highway 55 (Olson Memorial Highway) Corridor
Hennepin County
WSB Project No. 1484-04

## Project Overview

The purpose of this study is to forecast future traffic volumes and document traffic and light-rail transit (LRT) operations along Trunk Highway (TH) 55 in the City of Minneapolis. Figure 1 shows the project location. It is assumed that LRT will run in the center median along TH 55. Further, the proposed signal systems along TH 55 will be coordinated to allow both preemption and priority traffic signal operating alternatives to assist LRT as it travels through the corridor.

This report documents the existing and future traffic conditions along the Bottineau Corridor in the City of Minneapolis. This report also analyzes the anticipated operational impacts of LRT on the adjacent roadway system.

The study includes an analysis of existing, no-build, and build scenarios. The no-build scenario includes minimal network revisions while the build scenario includes the addition of LRT as well as other network revisions. Twelve key intersections are analyzed in this study. These intersections include:

- TH 55 and Van White Memorial Boulevard
- TH 55 and Bryant Avenue North
- TH 55 and West Lyndale Avenue North
- TH 55 and East Lyndale Avenue North
- TH 55 and Border Avenue/Oak Lake Avenue North
- TH 55/7th Street North and 6th Avenue North
- TH 55/7th Street North and 5th Avenue North
- 7th Street North and Oak Lake Avenue North
- 6th Avenue North and HERC/Metro Transit Access
- 6th Avenue North and 5th Street North
- 6th Avenue North and 4th Street North
- 5th Street North and 5th Avenue North

The existing lane configuration for the key intersections is shown in Figure 2. A description of the existing roadway geometry is provided in Appendix A.

## Traffic Forecasts

The major roadways within the study area are TH 55 (Olson Memorial Highway) and 7th Street North. On the west side of I-94, TH 55 is classified as a principal arterial. The dominant land-uses adjacent to TH 55 in this area are multifamily residential and undeveloped. To the east of I-94, TH 55 and 7th Street North are classified as 'A' minor relievers. The dominant land-uses adjacent to TH 55 and 7th Street North in this area are industrial and commercial.

TH 55 carries 25,000 vehicles per day (2006) west of I-94 and 11,100 vehicles per day (2005) between I-94 and 7th Street North. 7th Street North carries 9,100 to 11,800 vehicles per day (2005) between I-94 and I-394. ${ }^{1}$ Figures 3 and 4 display the existing am and pm peak hour turning movements at the key intersections. Turning movement data was collected in November of 2008 after the reopening of the I35W Bridge and the $5^{\text {th }}$ Street Bridge.

Peak hour traffic forecasts were developed for the key intersections within the project area by applying an annual growth rate to am and pm peak hour turning movement counts. The annual growth rate was developed based on a comparison of the average daily traffic (ADT) in the Metropolitan Council's 2000 Base Travel Demand Model and the Metropolitan Council's 2030 Travel Demand Model. These models were run using current (2008) Minneapolis Draft Comprehensive 2030 socioeconomic data forecasts.

Some minor changes to the Metropolitan Council’s 2030 Travel Demand Model were made in this analysis. Van White Memorial Boulevard from TH 55 to I-394 was added to the network. In addition, the traffic volumes on side streets were adjusted as necessary based on a review of growth in the adjacent areas and the model outputs.

The historical growth in traffic along screenlines and the growth in trip generation associated with the Traffic Analysis Zones were reviewed in order to confirm that the growth factors used were consistent with model outputs. Selected screenlines were reviewed to make sure that the generalized nature of the travel demand model is not misrepresenting traffic in a given area. Minor adjustments were made to the location of Traffic Analysis Zone (TAZ) centroids to better represent the loadings onto the roadway network.

A review of the historical traffic volumes in the entire downtown area revealed a slight decrease in traffic over the past eight years. The Travel Demand Model forecasts increasing population, employment, and trips in the downtown Minneapolis area between the year 2000 and 2030. The following is a summary of this information.

[^1]
## Regional Travel Demand Model

The ADT volumes obtained from the Metropolitan Council's 2000 and 2030 Twin Cities Regional Travel Demand Models were compared in order to develop an annual growth rate for each leg of the key intersections in the study area. As a check of the forecast growth in the study area, three screenlines were evaluated in order to compare historical growth rates. These screenlines also allowed for a comparison of the impacts from changes in the 2030 land-use as reflected in the 2008 Minneapolis Draft Comprehensive Plan's socioeconomic data. The screenlines used in this analysis include:

- West of I-94 - selected roadways between I-394 and 7th Street North including:
- I-394
- Glenwood Avenue
- TH 55
- $7^{\text {th }}$ Street North
- East of I-94 - selected roadways along the western edge of downtown between Hennepin Avenue and $2^{\text {nd }}$ Street North including:
- Hennepin Avenue
- I-394
- Glenwood Avenue
- TH 55
- $7^{\text {th }}$ Street North
- $2^{\text {nd }}$ through $4^{\text {th }}$ Streets North
- Washington Avenue
- West River Road
- Downtown - all of the major roadways entering downtown Minneapolis inside I-94, I-35W, Plymouth Avenue North, and the Mississippi River.

Figure 5 shows the location of these screenlines.
The 2030 socioeconomic data for the downtown Minneapolis area in the 2008 Minneapolis Draft Comprehensive Plan represents a considerable change from the 2030 socioeconomic data in the current Metropolitan Council 2030 Travel Demand Model. Table 1 below shows a screenline comparison between the 2030 ADT outputs using Minneapolis’s socioeconomic data and using Metropolitan Council's socioeconomic data.

Table 1: Forecast Annual Traffic Growth Rate between 2000 and 2030 based on Comparison of 2000 and 2030 Metro Council Model ADT's on Selected Screenlines

| Screenline <br> Location | Metropolitan Council's 2030 Model | Metropolitan Council's 2030 Model with Current <br> Minneapolis 2030 Socioeconomic Data |
| :--- | :---: | :---: |
| West of l-94 | -0.04 percent/year | 0.23 percent/year |
| East of I-94 | 0.25 percent/year | 0.45 percent/year |
| Downtown | 0.31 percent/year | 0.47 percent/year |

## Historical Traffic Volumes

Historical traffic growth was also reviewed along the same three screenlines. ADT volumes were taken from Mn/DOT maps between 1998 and 2006 to develop a historical trend for the study area. 2007 traffic volumes were not used due to significant temporary changes in travel patterns as a result of the closure of the I-35W bridge and the $5^{\text {th }}$ Street bridge.

Generally, the historical change in ADT decreased between 1998 and 2006. Traffic volumes west of I-94 decreased by approximately 1.0 percent per year. Traffic volumes east of I-94 decreased by approximately 0.30 percent per year. Traffic volumes entering and exiting downtown decreased by approximately 0.6 percent per year. Figure 6 displays the decreasing trend of traffic entering and exiting the downtown area.

Figure 6: Historical Downtown Traffic (Sum of entering and exiting ADTs)


## Socioeconomic Data

The approved 2000 and 2030 Metropolitan Council socioeconomic data used in the Travel Demand Models and the 2008 Minneapolis Draft Comprehensive Plan socioeconomic data were compared to show how the current projected 2030 socioeconomic data from Minneapolis compares with the previous projections used in the Metropolitan Council's 2030 Travel Demand Model. Three areas were defined for this analysis.

- West of I-94 - the area bound by I-94, I-394, Penn Avenue, and West Broadway Avenue.
- East of I-94 - the area along the western edge of downtown bound by I-94, $15^{\text {th }}$ Street West, LaSalle Avenue-8 ${ }^{\text {th }}$ Street-Hennepin Avenue, Plymouth Avenue North, and the Mississippi River.
- Downtown - all of the area within I-94, I-35W, Plymouth Avenue North, and the Mississippi River.

Figure 5 shows the location of these areas. The results of this comparison are summarized in Tables 2, 3, and 4. The current 2030 socioeconomic data in the approved Metropolitan Council 2030 Travel Demand Model shows that the population and employment of downtown is forecasted to increase by approximately 50 percent and 30 percent respectively between the years 2000 and 2030. The socioeconomic data from the 2008 Minneapolis Draft Comprehensive Plan shows that the population and employment of downtown is forecasted to increase by approximately 130 percent and 20 percent respectively between the years 2000 and 2030. These differences in population and employment result in differences between the traffic forecasts for the two different models.

The 2030 socioeconomic data in the 2008 Minneapolis Draft Comprehensive Plan has greater increases in population and lesser increases in employment than what is currently in the approved 2030 Metropolitan Council Travel Demand Model. The northwest portion of downtown adjacent to the study area is forecasted to experience a much higher increase in population in the 2008 Minneapolis Draft Comprehensive Plan data.

Table 2: Socioeconomic Data

| Area | 2000 Metropolitan Council Data |  |  |  | 2030 Metropolitan Council Data |  |  |  | 2030 MPLS Draft Comp Plan Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { z } \\ & \text { 은 } \\ & \vdots \\ & 0 \\ & 0 \end{aligned}$ |  | EMPLOYMENT |  | Z <br> 은 <br>  <br> 0 <br> 0 <br> 0 <br> 0 |  | EMPLOYMENT |  |  | $\begin{aligned} & \text { on } \\ & \text { 우 } \\ & \text { 쏘 } \\ & \text { O} \\ & \text { 오 } \end{aligned}$ | EMPLOYMENT |  |
|  |  |  |  |  |  |  | $\stackrel{\frac{1}{\mathbf{\|}}}{\underset{\sim}{\mid}}$ |  |  |  |  |  |
| West of I-94 | 12,903 | 4,075 | 195 | 4,675 | 12,413 | 4,098 | 1,284 | 2,693 | 15,011 | 6,154 | 1,731 | 4,676 |
| East of I-94 | 5,636 | 3,439 | 970 | 31,894 | 11,734 | 7,320 | 3,539 | 32,302 | 18,213 | 11,492 | 4,392 | 30,480 |
| Downtown | 20,201 | 11,772 | 9,322 | 137,152 | 31,428 | 20,900 | 14,399 | 179,161 | 46,900 | 28,520 | 14,761 | 155,513 |

Table 3: Change in Socioeconomic Data between 2000 and 2030

| Area | Approved MET Council Model Data (2030 MET Council - 2000 MET Council) |  |  |  | MPLS Draft Comp Plan Data(2030 MPLS Draft Comp Plan-2000 MET Council) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | EMPLOYMENT |  | $\begin{aligned} & \text { z } \\ & \frac{0}{1} \\ & \vdots \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ |  | EMPLOYMENT |  |
|  |  |  |  |  |  |  | $\stackrel{\text { 立 }}{\stackrel{\text { ¢ }}{\text { ¢ }}}$ |  |
| West of I-94 | -490 | 23 | 1,089 | -1,982 | 2,108 | 2,079 | 1,536 | 1 |
| East of I-94 | 6,098 | 3,881 | 2,569 | 408 | 12,577 | 8,053 | 3,422 | -1,414 |
| Downtown | 11,227 | 9,128 | 5,077 | 42,009 | 26,699 | 16,748 | 5,439 | 18,361 |

Table 4: Percent Change in Socioeconomic Data between 2000 and 2030

| Area | Approved MET Council Model Data \% Change ([2030-2000]/2000) |  |  |  | MPLS Draft Comp Plan Data \% Change ([2030-2000]/2000) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { z } \\ & \frac{0}{1} \\ & \vdots \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ |  | EMPLOYMENT |  |  | $\begin{aligned} & 0 \\ & \stackrel{0}{0} \\ & \text { 웄 } \\ & 0 \\ & \text { ㅇㅜㅗ } \end{aligned}$ | EMPLOYMENT |  |
|  |  |  |  |  |  |  |  |  |
| West of I-94 | -4\% | 1\% | 558\% | -42\% | 16\% | 51\% | 788\% | 0\% |
| East of l-94 | 108\% | 113\% | 265\% | 1\% | 223\% | 234\% | 353\% | -4\% |
| Downtown | 56\% | 78\% | 54\% | 31\% | 132\% | 142\% | 58\% | 13\% |

## Year 2030 Forecasts

The 2030 socioeconomic data from the 2008 Minneapolis Draft Comprehensive Plan was used to develop the 2030 traffic forecasts for the study area. Annual growth rates for the key intersections were calculated based on a comparison of the modeled ADT's in the 2000 and 2030 Regional Travel Demand Model with the inclusion of Van White Memorial Boulevard from TH 55 to I-394. The annual growth rates were applied to the existing turning movement counts taken in the fall of 2008. Based on the future socioeconomic data obtained from the 2008 Minneapolis Draft Comprehensive Plan, the annual growth rates for cross street volumes were reviewed relative to the change in trip generation for the areas they serve.

Figures 7 and $\mathbf{8}$ display the year 2030 am and pm peak hour turning movement forecasts at the key intersections. Minor adjustments to the forecasted volumes were necessary in order to balance turning movements at the key intersections. These forecasted volumes will be used for the 2030 no-build and 2030 build roadway/LRT alternatives analyses.

## Operations Modeling Methodology and Assumptions

## VISSIM Methodology

VISSIM is a microscopic, time step and behavior based simulation model developed to model urban traffic and public transit operations. It is approved by $\mathrm{Mn} / \mathrm{DOT}$ as a method to analyze complex transportation systems involving traffic and transit operations under constraints such as lane configuration, traffic composition, traffic signals, transit stops, etc., thus making it a useful tool for the evaluation of various alternatives based on transportation engineering and planning measures of effectiveness.

In contrast to less complex models using constant speeds and deterministic car following logic, VISSIM uses the psycho-physical driver behavior model developed by WIEDEMANN (1974). Stochastic distributions of speed and spacing thresholds replicate individual driver behavior characteristics. VISSIM simulates the traffic flow by moving "driver-vehicles" through a network. Every driver, with specific behavior characteristics, is assigned to a specific vehicle. Accordingly, the driving behavior corresponds to the technical capabilities of the vehicle. This allows drivers on multiple lane roadways to react to preceding vehicles and neighboring vehicles on adjacent travel lanes.

## Level of Service Analysis Criteria

The traffic operations analysis is based on established methodologies documented in the Highway Capacity Manual (TRB, 2000). The results from this analysis are presented in the form of a letter grade from A to F, called level of service (LOS). The letter grade provides a qualitative assessment of the intersection operations based on the amount of delay per vehicle. The LOS system is set up similar to a report card with "A" representing high quality operations and "F" representing poor operations. At LOS A, motorists experience very little delay or interference. On a roadway or intersection with LOS F conditions, motorists would experience severe congestion and extreme delay, i.e., gridlock.

For intersections, LOS is primarily a function of delay which is dependent on am and pm peak hour turning movement volumes, intersection lane configuration, and traffic control. The intersection analysis was completed using average control delay as defined by the HCM. The threshold values for each LOS for unsignalized intersections are slightly less than for signalized intersections. This variance was created because motorists' expectations of the intersection differ with the type of traffic control. The LOS analysis criteria for signalized and unsignalized intersections are shown in Figure 9.

Figure 9: Intersection Level of Service Based on Delay


Source: Highway Capacity Manual (2000)
Although LOS A conditions represents the best possible level of traffic flow, the cost to construct intersections to such a high standard exceeds the benefit to the user. Within an urbanized area, it is generally regarded that LOS D provides an acceptable LOS.

## Alternatives Analysis

Six different VISSIM alternatives were modeled to analyze the existing and future roadway system.

1. Existing (2008)
2. 2030 No-Build
3. 2030 Build - Baseline
4. 2030 Build - LRT with Typical Signal Operations
5. 2030 Build - LRT with Priority Signal Operations
6. 2030 Build - LRT with Preemption Signal Operations

## Geometrics

Alternative 1 contains the existing roadway network comprised of traffic signals that permit left turns on green. Alternative 2 contains traffic signals that permit left turns on green and slight improvements to the roadway network including the addition of right turn lanes on TH 55 eastbound at Van White Memorial Boulevard and Bryant Avenue. Also, a northbound right turn lane and a southbound left turn lane were added to Van White Memorial Boulevard at TH 55. Alternatives 3 through 6 had the roadway network revised to add additional capacity at intersections or to remove unneeded lanes thus providing needed right-of-way for the proposed

LRT. Protected left turn phases were also added to traffic signals along TH 55. The revisions mentioned below for Alternatives 3-6 assume the improvements mentioned for Alternative 2 have already been constructed.

- TH 55 and Bryant Avenue North: The exclusive right turn lane becomes a through-right turn lane.
- TH 55 and West Lyndale Avenue North: A second exclusive right turn lane to I-94 EB was added at the bridge replacing the existing combined through/right turn lane. The exclusive right turn lane onto West Lyndale Avenue south was shifted to the south, thus widening the roadway an additional lane.
- TH 55 Bridge over I-94: One through lane in each direction was removed.
- TH 55 and Border Avenue/Oak Lake Avenue North: One through lane in each direction was removed.
- TH 55/7th Street North and 6th Avenue North: One lane in the eastbound direction was removed leaving a through lane, a through-right turn lane, and a right turn lane. A left turn lane was added to the westbound approach and an exclusive right turn lane will take the place of the existing through-right turn lane. This will reduce the westbound lanes traveling through the intersection from three to two. The three lane approach at northbound 7th Street will be resigned to include a left turn lane, a through-left turn lane, and a through lane.

The LRT is assumed to operate in the center of the roadway along TH 55 from Van White Memorial Boulevard through the intersection of 7th Street North and 6th Avenue North. Once the alignment crosses 7th Street, it veers to the south, crossing the eastbound lanes of 6th Avenue, and then operates adjacent to the roadway. The alignment remains side running as it transitions onto 5th Street and enters the station at Target Field. Far-side stations are located at the intersection of TH 55 and Van White Memorial Boulevard.

## Operating Assumptions

The LRT is assumed to operate at a frequency of seven-and-a-half minutes during the peak hours with a station dwell time of approximately twenty seconds at Van White Memorial Boulevard. The TH 55 traffic signals are semi-actuated, coordinated in the eastbound and westbound directions with the exception of the 7th Street and 6th Avenue intersection operating as a fully actuated signal in Alternative 6. Traffic signals for Alternatives 1, 2, and 6 use VISSIM's NEMA controllers. Traffic signals for Alternatives 3-5 use VISSIM’s RBC controllers.

In Alternative 5, LRT is given priority at traffic signals causing minor street phases to end after the minimums are reached and possibly causing major street through phases to be extended beyond their normal maximums in order to minimize LRT delay. Also, once a priority call ends, calls for additional priority will be ignored for one and one half cycle lengths in an attempt to let the traffic signal recover and fully service the minor approaches. During this recovery time, LRT will not be given priority and will have to wait for a green phase.

In Alternative 6, LRT uses preemption by placing an advanced call to the traffic signal warning that a train is approaching. Phases on the side streets are shortened to allow the LRT to run through the corridor without delay at intersections. Through phases not conflicting with the train are allowed to navigate through the intersection while the preemptive call is active. After the preemptive call is satisfied, the next phase the traffic signal brings up will be the first phase across the next barrier.

## Results

A summary of the levels of service for each alternative is provided below. Tables in Appendix $\mathbf{B}$ provide more detail as to the individual movements at each intersection and the queue lengths recorded. Train travel times in Alternatives 4-6 were measured from a checkpoint just west of Van White Memorial Boulevard to a checkpoint just west of the Target Field station.

## Alternative 1 - Existing (2008) Volumes, Existing Geometry

Table 5 summarizes the existing LOS at the key intersections based on the existing lane geometry, City of Minneapolis provided signal timing, and 2008 existing traffic volumes as shown in Figures 3 and 4. In this alternative, all intersections are currently operating at LOS D or better during the am and pm peak hours. In the am peak hour, the southbound left turning movement at $7^{\text {th }}$ Street and $6{ }^{\text {th }}$ Avenue is operating at LOS E. In the pm peak hour, the eastbound right from TH 55 onto eastbound I-94 operates at LOS E. Also, the northbound left from $7^{\text {th }}$ Street to TH 55 westbound operates at LOS F. These results are consistent with observed conditions.

Table 5: Alternative 1 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | B | 10 | SBT | D | 36 | B | 10 | SBL | D | 36 |
| TH 55 \& Bryant Ave N | A | 3 | NBL | D | 45 | B | 17 | NBT | D | 48 |
| TH 55 \& W Lyndale Ave N | B | 18 | SBL | C | 28 | C | 27 | EBR | E | 64 |
| TH 55 \& E Lyndale Ave N | C | 22 | NBL | C | 32 | C | 24 | EBL | D | 36 |
| TH 55 \& Border/Oak Lake Ave | A | 8 | NBT | D | 42 | B | 17 | NBL | D | 50 |
| TH 55/7th St N \& 6th Ave N | C | 32 | SBL | E | 61 | D | 43 | NBL | F | >100 |
| 6th Ave N \& HERC Access | A | 3 | NBL | D | 44 | A | 4 | SBL | D | 53 |
| 6th Ave N \& 5th St W | A | 1 | WBT | A | 8 | A | 8 | SBT | B | 11 |
| 6th Ave N \& 4th St W | A | 2 | EBT | B | 10 | A | 2 | EBT | A | 9 |
| 5th St W \& 5th Ave N | A | 5 | SBL | D | 38 | A | 5 | SBL | D | 47 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | B | 12 | A | 7 | EBL | E | 37 |
| 7th St N \& Oak Lake Ave | B | 16 | SBL | D | 42 | B | 16 | NBL | D | 43 |

Source: WSB \& Associates

## Alternative 2 - 2030 Volumes, Existing/No-Build Geometry

Table 6 summarizes the LOS based on Alternative 2 lane geometry and forecasted 2030 traffic volumes as shown in Figures 7 and 8. In this alternative, all intersections operate at LOS C or better during the am and pm peak hours with the exception of Van White Memorial Boulevard operating at LOS E during the pm peak hour. The worst movements during the pm peak hour at Van White, Bryant, and West Lyndale are due to vehicles backing up and slowing down in an effort to get onto the I-94 eastbound on-ramp. LOS F conditions are also reported for the northbound lefts at TH 55/Border Ave and for northbound lefts at $7^{\text {th }}$ Street and Oak Lake

Avenue. In Alternative 1, the NBL at $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue was at LOS F conditions using existing/provided signal timings. Signal timings were optimized in Alternative 2 and the result was improved LOS conditions for the NBL at $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue.

Table 6: Alternative 2 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | C | 31 | EBR | E | 58 | E | 61 | EBR | F | >100 |
| TH 55 \& Bryant Ave N | B | 12 | NBL | D | 48 | C | 28 | NBR | E | 74 |
| TH 55 \& W Lyndale Ave N | C | 29 | EBR | D | 40 | C | 28 | EBR | E | 76 |
| TH 55 \& E Lyndale Ave N | C | 26 | EBT | C | 31 | C | 25 | WBT | D | 41 |
| TH 55 \& Border/Oak Lake Ave | B | 13 | NBL | D | 50 | C | 23 | NBL | F | >100 |
| TH 55/7th St N \& 6th Ave N | C | 25 | SBT | D | 43 | C | 22 | SBL | D | 36 |
| 6th Ave N \& HERC Access | A | 4 | SBL | D | 46 | A | 4 | SBL | D | 45 |
| 6th Ave N \& 5th St W | A | 1 | WBT | B | 11 | C | 20 | SBT | D | 29 |
| 6th Ave N \& 4th St W | A | 3 | EBT | B | 11 | A | 4 | EBT | B | 12 |
| 5th St W \& 5th Ave N | A | 6 | SBL | D | 44 | A | 6 | SBL | D | 46 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | C | 15 | A | 1 | EBL | A | 9 |
| 7th St N \& Oak Lake Ave | B | 19 | NBL | F | 99 | C | 28 | NBL | F | >100 |

Source: WSB \& Associates

Many vehicle queue lengths are longer than the available storage in the am and pm peak hours and are related to the delay at intersections. Movements that have a maximum queue greater than the storage provided are listed in Table 7.

Table 7: Alternative 2 Peak Hour Queues Exceeding Capacity

| Intersection | AM Peak Hour |  | PM Peak Hour |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Movement \| Storage (ft)| <br> Queue (ft) |  | Movement \| Storage (ft) | <br> Queue (ft) |  |  |  |
| TH 55 \& Van White Mem Blvd | EBT | 950 | 1,045 | EBT | 950 | 1,660 |
| TH 55 \& Bryant Ave N |  |  |  | EBT | 750 | 1,670 |
| TH 55 \& W Lyndale Ave N | EBT | 500 | 655 | EBT | 500 | 900 |
| TH 55 \& E Lyndale Ave N | EBL | 200 | 250 | EBL | 200 | 260 |
|  | EBT | 200 | 325 | WBT | 430 | 545 |
| TH 55/7th St N \& 6th Ave N | SBT | 450 | 540 | SBT | 450 | 540 |

Source: WSB \& Associates

## Alternative 3-2030 Volumes, Baseline Build Geometry with Protected Lefts

Table 8 summarizes the LOS based on Alternative 3 lane geometry and forecasted 2030 traffic volumes. In this alternative, all intersections operate at LOS D or better during the am and pm peak hours. By adding protected left turn phases to the traffic signals along the LRT corridor, some intersections experienced additional delay. Delay was reduced by adding capacity along TH 55 between Van White and West Lyndale and through the addition of a second right turn lane onto the I-94 eastbound on-ramp. Also, setting the signal at $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue to a split-phase for northbound and southbound allowed for two northbound left turn lanes. This helped reduce delay at $7^{\text {th }}$ Street and at Border Avenue due to a better flow of vehicles through the coordinated system. The northbound lefts at $7^{\text {th }}$ Street and Oak Lake continue to have a poor LOS due to poor operations at $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue causing queues to back up from $6^{\text {th }}$ Avenue to Oak Lake Avenue.

Table 8: Alternative 3 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control <br> Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  | LOS \| Control <br> Delay (sec/veh) |  | Worst Movement \| LOS | Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | C | 25 | EBL | E | 70 | C | 24 | SBL | E | 76 |
| TH 55 \& Bryant Ave N | C | 21 | EBL | E | 74 | B | 13 | WBL | F | 96 |
| TH 55 \& W Lyndale Ave N | C | 29 | SBL | D | 46 | C | 20 | SBL | D | 48 |
| TH 55 \& E Lyndale Ave N | C | 24 | NBL | D | 41 | C | 26 | NBL | D | 50 |
| TH 55 \& Border/Oak Lake Ave | C | 30 | WBL | E | 71 | C | 33 | WBL | E | 73 |
| TH 55/7th St N \& 6th Ave N | D | 43 | WBL | F | >100 | D | 37 | WBL | E | 63 |
| 6th Ave N \& HERC Access | A | 5 | NBL | D | 39 | A | 4 | NBL | D | 37 |
| 6th Ave N \& 5th St W | A | 1 | WBT | B | 10 | C | 19 | SBT | D | 28 |
| 6th Ave N \& 4th St W | A | 3 | EBT | B | 12 | A | 4 | WBL | B | 11 |
| 5th St W \& 5th Ave N | A | 6 | SBL | B | 10 | A | 6 | SBL | B | 10 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | B | 14 | A | 1 | EBL | A | 9 |
| 7th St N \& Oak Lake Ave | C | 27 | NBL | F | >100 | C | 21 | NBL | F | 94 |

Source: WSB \& Associates
Alternative 4-2030 Volumes with LRT, Typical Signal Operations, Baseline Build Geometry
Table 9 summarizes the LOS based on Alternative 4 lane geometry and forecasted 2030 traffic volumes. In this alternative, all intersections operate at LOS D or better during the am and pm peak hours. The worst delay movements in both the am and pm peak hours are mostly turning and through movements that conflict with the high volume movements and the LRT movement through the intersections. The westbound left turn from TH 55 onto Van White is at LOS F in the am peak hour. This approach could benefit from dual left turn lanes but additional right-ofway would be needed to shift through lanes further to the north. During the am peak hour, the average LRT travel times eastbound and westbound were 4.5 minutes and 4.1 minutes. During the pm peak hour, the average LRT travel times eastbound and westbound were 4.8 minutes and 5.3 minutes.

Table 9: Alternative 4 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | C | 29 | WBL | F | 81 | C | 27 | SBL | E | 76 |
| TH 55 \& Bryant Ave N | B | 12 | NBT | E | 63 | A | 9 | WBL | E | 77 |
| TH 55 \& W Lyndale Ave N | D | 35 | WBL | E | 56 | C | 27 | SBL | E | 55 |
| TH 55 \& E Lyndale Ave N | C | 26 | NBL | D | 48 | D | 39 | WBT | E | 55 |
| TH 55 \& Border/Oak Lake Ave | C | 27 | WBL | E | 72 | D | 36 | EBL | E | 69 |
| TH 55/7th St N \& 6th Ave N | D | 48 | WBL | F | >100 | D | 51 | SBT | E | 68 |
| 6th Ave N \& HERC Access | A | 6 | NBL | D | 39 | A | 4 | NBL | D | 38 |
| 6th Ave N \& 5th St W | A | 1 | WBT | B | 10 | C | 20 | SBT | D | 30 |
| 6th Ave N \& 4th St W | A | 3 | WBT | B | 11 | A | 4 | EBT | C | 15 |
| 5th St W \& 5th Ave N | A | 6 | SBL | B | 13 | A | 7 | SBL | B | 11 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | C | 15 | A | 1 | EBL | A | 8 |
| 7th St N \& Oak Lake Ave | D | 36 | EBT | D | 48 | B | 12 | NBL | B | 19 |

Source: WSB \& Associates
Alternative 5-2030 Volumes with LRT, Priority Signal Operations, Baseline Build Geometry
Table 10 summarizes the LOS based on Alternative 5 lane geometry and forecasted 2030 traffic volumes. In this alternative, all intersections operate at LOS D or better during the am and pm
peak hours. The result of adding LRT priority to the traffic signals along the corridor was a slight increase in delay over Alternative 4 results. The issues are still the westbound left turn to Van White, the westbound left turn to I-94 eastbound, and the capacity constrained intersection of $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue. During the am peak hour, the average LRT travel times eastbound and westbound were 5.1 minutes and 4.4 minutes. During the pm peak hour, the average LRT travel times eastbound and westbound were 4.8 minutes and 5.0 minutes. The resulting difference in LRT travel times between Alternative 4 and Alternative 5 are minor. LRT travel time differences ranged from 36 seconds slower to 18 seconds faster.

Table 10: Alternative 5 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | Control Delay (sec/veh) |  |  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | C | 34 | WBL | F | >100 | C | 28 | WBL | F | 93 |
| TH 55 \& Bryant Ave N | B | 19 | NBL | E | 66 | B | 10 | WBL | E | 75 |
| TH 55 \& W Lyndale Ave N | D | 35 | WBL | E | 63 | C | 27 | SBL | E | 61 |
| TH 55 \& E Lyndale Ave N | C | 27 | NBL | D | 48 | D | 40 | NBL | E | 57 |
| TH 55 \& Border/Oak Lake Ave | C | 26 | WBL | E | 71 | D | 39 | NBL | F | 99 |
| TH 55/7th St N \& 6th Ave N | D | 43 | WBL | F | >100 | D | 51 | NBT | E | 70 |
| 6th Ave N \& HERC Access | A | 5 | SBL | D | 43 | A | 4 | NBL | D | 38 |
| 6th Ave N \& 5th St W | A | 2 | WBT | B | 10 | C | 21 | SBT | D | 31 |
| 6th Ave N \& 4th St W | A | 3 | EBT | B | 11 | A | 5 | EBR | B | 13 |
| 5th St W \& 5th Ave N | A | 6 | SBL | B | 12 | A | 6 | SBL | B | 12 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | B | 14 | A | 1 | EBL | A | 9 |
| 7th St N \& Oak Lake Ave | C | 24 | EBT | C | 30 | B | 13 | NBL | C | 21 |

Source: WSB \& Associates

## Alternative 6-2030 Volumes with LRT, Preemption Signal Operations, Baseline Build Geometry

Table 11 summarizes the LOS based on Alternative 6 lane geometry and forecasted 2030 traffic volumes. In this alternative, all intersections operate at LOS D or better during the am and pm peak hours. The result of adding LRT preemption to the traffic signals along the corridor was a slight decrease in delay over Alternative 4 results. Compared to Alternative 5, preemption operates slightly better in the am peak hour and nearly the same as priority in the pm peak hour. The same issues are still the westbound left turn to Van White, the westbound left turn to I-94 eastbound, and the capacity constrained intersection of $7^{\text {th }}$ Street and $6{ }^{\text {th }}$ Avenue. Since the LRT receives preemption at all traffic signals, the train travel time between checkpoints was constant at 3.1 minutes. This results in approximately one to two minutes faster LRT travel times compared to Alternative 4 and Alternative 5.

Table 11: Alternative 6 Peak Hour Intersection LOS

| Intersection | AM Peak Hour |  |  |  |  | PM Peak Hour |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | <br> Control Delay (sec/veh) |  |  | LOS \| Control Delay (sec/veh) |  | Worst Movement \| LOS | Control Delay (sec/veh) |  |  |
| TH 55 \& Van White Mem Blvd | C | 26 | WBL | F | >100 | C | 30 | WBL | F | >100 |
| TH 55 \& Bryant Ave N | B | 11 | NBL | E | 72 | B | 13 | WBL | F | 87 |
| TH 55 \& W Lyndale Ave N | C | 24 | WBL | F | 81 | C | 21 | WBL | E | 59 |
| TH 55 \& E Lyndale Ave N | C | 22 | NBL | D | 40 | D | 38 | WBT | E | 58 |
| TH 55 \& Border/Oak Lake Ave | C | 26 | WBL | D | 48 | D | 35 | WBL | E | 68 |
| TH 55/7th St N \& 6th Ave N | D | 41 | WBL | E | 75 | D | 47 | WBL | E | 74 |
| 6th Ave N \& HERC Access | A | 9 | SBL | E | 56 | A | 9 | SBL | E | 57 |
| 6th Ave N \& 5th St W | A | 2 | WBT | B | 12 | C | 22 | SBT | D | 32 |
| 6th Ave N \& 4th St W | A | 3 | EBT | B | 11 | A | 5 | WBL | C | 15 |
| 5th St W \& 5th Ave N | A | 7 | SBL | D | 46 | A | 7 | SBL | D | 51 |
| TH 55/7th St N \& 5th Ave N | A | 1 | EBL | B | 13 | A | 1 | EBL | A | 7 |
| 7th St N \& Oak Lake Ave | B | 13 | SBL | C | 33 | C | 20 | NBL | E | 55 |

Source: WSB \& Associates

## Conclusions

The purpose of this study is to determine the feasibility of LRT operating at-grade in the center median of TH 55 from Van White Boulevard to $5^{\text {th }}$ Street North. In order to determine the feasibility of at-grade LRT operations WSB developed 2030 traffic forecasts for the nine intersections included in the study. These forecasts were based on applying a growth factor to existing traffic counts for the study area. The growth factor was based on the forecast increase in ADT's between 2000 and 2030 using the Metropolitan Council’s Regional Travel Demand Model and the City of Minneapolis's 2008 Comprehensive Plan socio-economic forecasts. The analysis indicated an average traffic growth rate of approximately $0.5 \%$ per year on TH 55 . The analysis of these traffic forecasts for six different alternatives indicated the following.

- Currently, all intersections in the study area are operating at LOS D or better during the am and pm peak hours. However, there are selected traffic movements that operate at LOS "E" or "F". The two most congested intersections are the TH 55 and Lyndale Avenue intersection and the TH 55 and $7^{\text {th }}$ Street North Intersection. The traffic demand on eastbound TH 55 to turn right onto southbound Lyndale Avenue and eastbound I-94 results in queues that extend back through the Bryant Avenue intersection and LOS "E" for this traffic movement. Also the northbound and southbound left turns on $7{ }^{\text {th }}$ Avenue at TH 55 currently operate at level of service "E" or worse in the peak hours.
- Traffic operations on TH 55 are expected to be worse by 2030 as traffic volumes grow. Traffic on TH 55 is expected to increase at approximately 0.5 \% per year. Van White Memorial Boulevard traffic levels are expected to increase significantly upon completion of Van White Memorial Boulevard between I-394 and TH 55 and anticipated development. The forecasted population and traffic growth along Van White Memorial Boulevard south of the study area will create the need for intersection revisions at Van White Memorial Boulevard and TH 55. These revisions, including exclusive right and left-turn lanes on Van White Boulevard, were assumed in the analysis for the 2030 conditions. A second left turn lane from TH 55 westbound to Van White southbound may also be needed to mitigate the delay for this left-turn movement by 2030. The analysis indicates that another right-turn lane is needed on eastbound TH 55 for the right-
turns to southbound Lyndale Avenue and to the eastbound I-94 on-ramp in order to provide an acceptable level of service. Without this extra right-turn lane traffic backs up on TH 55 through the Van White intersection and results in LOS "E" for the eastbound TH 55 traffic.
- The major traffic operations impact of introducing center running LRT on TH 55 is the increased delay experienced by vehicles turning onto, off of, or crossing TH 55. Since through traffic can move concurrently with LRT there will be very little impact on TH 55 through traffic. The model indicates that the biggest problem will occur in the westbound direction as a result of vehicles turning left from TH 55 to southbound Lyndale Avenue / eastbound I-94 ramp. The intersection of $7^{\text {th }}$ Street and $6^{\text {th }}$ Avenue is currently experiencing at-capacity conditions. This intersection consists of two major roadways that funnel traffic into and out-of downtown Minneapolis. Approaches constantly struggle for green time during the peak hours. This makes coordination with the rest of the corridor difficult.
- Priority vs. Preemption - Although the analysis shows that the overall intersection level of service is almost the same between priority and preemption signal operations, preemption does make the critical left-turn movement from westbound TH 55 to southbound Lyndale Avenue / eastbound I-94 ramp worse, creating queues that extend back to $7^{\text {th }}$ Street.
- Because the length of 3-car trains is more than the storage area on TH 55 between southbound Lyndale Avenue and northbound Lyndale Avenue, it is necessary to modify the signal timing for these intersections so trains do not become trapped between the intersections. This adds to the delays for left-turns at these intersections. Dual left turn lanes may be needed in both the eastbound and westbound directions to address this problem if preemption is considered on TH 55.
- Train Travel Times - LRT priority causes the train to experience random delay at some traffic signals within the study area. LRT preemption causes no delay to trains within the study area. The train travel time through the study area is 3.1 minutes with preemption and 4.4 to 5.0 minutes with priority treatment.




Figure 3





## Appendix A

## Existing Roadway Geometry

- TH 55 is a six-lane divided roadway west of I-94 and between I-94 and 7th Street North. When TH 55 combines with 7th Street North, south of 6th Avenue North, it becomes a six lane undivided roadway.
- 7th Street North is a four-lane undivided roadway between I-94 and 6th Avenue North.
- TH 55 and Van White Memorial Boulevard: This is a four-legged signalized intersection. TH 55 eastbound and westbound approaches each have a left turn lane, three through lanes, and right turns allowed from the right most through lane. The Van White Memorial Boulevard southbound approach has a right turn lane and a shared through and left turn lane. The Van White Memorial Boulevard northbound approach has a left turn lane and a shared through and right turn lane.
- TH 55 and Bryant Avenue North: This is a four-legged signalized intersection with Bryant Avenue North meeting TH 55 on a skew. TH 55 eastbound and westbound approaches each have a left turn lane, three through lanes, and right turns allowed from the right most through lane. Bryant Avenue North northbound and southbound approaches each have only a single lane shared between right, through, and left turning vehicles.
- TH 55 and West Lyndale Avenue North: This is a signalized intersection with West Lyndale Avenue North being a southbound one-way roadway. The southbound approach on West Lyndale Avenue North has four through lanes. The two right most through lanes are for traffic destined to I-94 eastbound while the two left most through lanes are for traffic destined to West Lyndale Avenue North southbound. Left turns are allowed from the left most lane while right turns are allowed from the right most lane. The eastbound approach on TH 55 has four through lanes. Right turning vehicles must choose between two destinations, West Lyndale Avenue North southbound or I-94 eastbound. Vehicle turning onto West Lyndale Avenue North southbound utilize a right turn lane preceding the intersection. Vehicle turning onto I-94 eastbound use the right most through lane to turn right once they have entered the intersection. A left-turn movement is not possible for this approach. The westbound approach on TH 55 has a left turn lane and three through lanes. A right-turn movement is not possible for this approach.
- TH 55 and East Lyndale Avenue North: This is a signalized intersection with East Lyndale Avenue North being a northbound one-way roadway. The northbound approach on East Lyndale Avenue North has one left turn lane and three through lanes. Left turns are allowed from the left turn lane and the left most through lane while right turns are allowed from the right most through lane. The eastbound approach on TH 55 has a left turn lane and three through lanes. A right-turn movement is not possible for this approach. The westbound approach on TH 55 has four through lanes and a right turn lane. A left-turn movement is not possible for this approach.
- TH 55 and Border Avenue/Oak Lake Avenue North: This is a four-legged signalized intersection with Border Avenue/Oak Lake Avenue North meeting TH 55 on a skew. Also, there is a southern frontage road that runs east-west within twenty feet of the intersection. Border Avenue is a one-way southbound roadway. The eastbound frontage road approach at Border Avenue is also a one-way roadway. The westbound frontage road approach is a two-way roadway. Figure 5 displays the lane alignments at this intersection. The TH 55 eastbound approach has a left turn lane, four through lanes, and right turns allowed from the right most through lane. The TH 55 westbound approach has a left turn lane, three through lanes, and a right turn lane. Border Avenue northbound and Oak Lake Avenue North southbound each have two through lanes. Left turns are allowed from the left through lane while right turns are allowed from the right through lane.
- TH 55/7th Street North and 6th Avenue North: This is a four-legged signalized intersection with 7th Street North meeting 6th Avenue North on a skew. The TH 55 eastbound approach has two through lanes and two right turn lanes. The 6th Avenue North westbound approach has three through lanes. Left turns are allowed from the left most lane while right turns are allowed from the right most lane. The 7th Street North southbound approach has two through lanes. Left turns are allowed from the left through lane while right turns are allowed from the right through lane. The 7th Street North northbound approach has a left turn lane and two through lanes. There is also a right turn lane prior to the intersection with a yield condition at 6th Avenue North eastbound.
- TH 55/7th Street North and 5th Avenue North: TH 55/7th Street North is the major through roadway at this three-legged side street stop controlled intersection. 5th Avenue North meets TH 55/7th Street North on a skew. The 7th Street North southbound approach has three through lanes with right turns allowed from the right most through lane. The 7th Street North northbound approach has three through lanes with left turns allowed from the left most through lane. The 5th Avenue North eastbound approach has a single shared lane for left and right turning vehicles.
- 7th Street North and Oak Lake Avenue North: This is a four-legged signalized intersection with 7th Street North meeting Oak Lake Avenue North on a skew. The 7th Street North eastbound approach has two through lanes and a bus stop lane that also acts as a right turn lane. Left turning vehicles are allowed from the left most through lane. The 7th Street North westbound approach has two through lanes. Left turns are allowed from the left through lane while right turns are allowed from the right through lane. Oak Lake Avenue North northbound and southbound approaches each have two through lanes. Left turns are allowed from the left through lane while right turns are allowed from the right through lane.
- 6th Avenue North and HERC/Metro Transit Access: This is a four-legged signalized intersection. 6th Avenue North eastbound and westbound approaches each have two through lanes. Left turns are allowed from the left through lane while right turns are allowed from the right through lane. In addition, 6th Avenue North eastbound trucks entering the HERC facility have a right turn lane prior to the intersection. Vehicles that are not trucks entering the facility use the right most through lane to turn right once they have entered the intersection. The HERC Access northbound and the Metro Transit Access southbound approaches each have only a single lane to be shared by right, through, and left turning vehicles. Northbound and southbound through traffic is unlikely due to the Metro Transit Access being restricted to busses only.
- 6th Avenue North and 5th Street North: 6th Avenue North is the major through roadway at this four-legged side street stop controlled intersection. The westbound approach of 5th Street North is a one-way roadway. There is a single shared lane for through and right turning vehicles. The 5th Street North eastbound approach is a two-way roadway with a single shared lane for left and right turning vehicles. The 6th Avenue North northbound approach has a single shared lane for through and left turning vehicles. The 6th Avenue North southbound approach has a single shared lane for through and right turning vehicles. Figure 6 displays the lane alignments at this intersection.
- 6th Avenue North and 4th Street North: 6th Avenue North is the major through roadway at this four-legged side street stop controlled intersection. All approaches have a single shared lane for left, through, and right turning vehicles.
- 5th Street North and 5th Avenue North: Presently, this three-legged intersection is all-way stop controlled due to continued road construction in the area. A signal will be reinstalled once construction is finalized. The 5th Street North westbound approach has a single shared lane for through and right turning vehicles. The 5th Street North eastbound approach has a single shared lane for through and left turning vehicles. The 5th Avenue North southbound approach has a single shared lane for left and right turning vehicles.

APPENDIX B
VISSIM LOS \& QUEUE DATA

Alternative 1
Bottineau Corridor
2008 AM Volumes
Existing Geometry


Alternative 1
Bottineau Corridor
2008 PM Volumes
Existing Geometry


Alternative 2
Bottineau Corridor
2030 AM Forecast Volumes
Existing Geometry


Alternative 2
Bottineau Corridor
2030 PM Forecast Volumes
Existing Geometry

| Intersection |  | Appr | Forecast Volumes |  |  |  | Modeled Volumes |  |  |  | Error Volumes |  |  | Volume Error by Approach |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach ( $\mathrm{Sec} / \mathrm{Veh}$ ) |  |  |  | Traffic Queueing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS by <br> Intersection <br> (Sec/Veh) |  |  |  |  | Through | Left Turn |  | Right Turn |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | L | T | R | total | L |  |  |  |  | T | R | total | L | T |  | R | Total | \% | L | T | R | L | T | R | Delay | LOS | Delay | LOS | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ | $\begin{array}{\|c\|} \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ | $\begin{gathered} \text { Ave } \\ \text { Queue } \end{gathered}$ | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ |
| ( ${ }_{\text {¢ }}$ |  |  | NB | 250 | 50 | 220 | 520 | 249 | 50 | 214 |  |  |  | 513 | -1 | 0 | -6 | -7 | -1\% | NB | 41 | 29 | 14 | D | C | B | 29 | C | 61 | E |  | 94 | 59 | 391 |  |  |
|  |  | WB | 220 | 1370 | 20 | 1610 | 215 | 1346 | 20 | 1581 | -5 | -24 | 0 | -29 | -2\% | WB | 32 | 14 | 21 | C | B | C | 17 | B | 43 | 255 |  | 202 |  |  |  |  |
|  |  | SB | 90 | 30 | 60 | 180 | 87 | 32 | 58 | 177 | -3 | 2 | -2 | -3 | -2\% | SB | 50 | 41 | 6 | D | D | A | 34 | C |  | 78 |  |  |  |  |  | 49 |
|  |  | EB | 50 | 1550 | 230 | 1830 | 40 | 1404 | 197 | 1641 | -10 | -146 | -33 | -189 | -10\% | EB | 72 | 112 | 157 | E | F | F | 116 | F | 801 | 1659 |  | 71 |  |  |  |  |
| ( 7 TH 55 \& Bryant Ave N |  |  | NB | 80 | 30 | 70 | 180 | 81 | 28 | 68 | 177 | 1 | -2 | -2 | -3 | -2\% | NB | 62 | 64 | 74 | E | E | E | 67 | E | 28 | c | 30 | 210 |  |  |  |  |
|  |  | WB | 70 | 1510 | 120 | 1700 | 61 | 1482 | 119 | 1662 | -9 | -28 | -1 | -38 | -2\% | WB | 19 | 6 | 6 | B | A | A | 6 | A |  |  |  | 276 |  | 52 |  |  |
|  |  | SB | 20 | 10 | 20 | 50 | 20 | 11 | 19 | 50 | 0 | 1 | -1 | 0 | 0\% | SB | 59 | 52 | 20 | E | D | C | 43 | D |  |  |  | 90 |  |  |  |  |
|  |  | EB | 10 | 1770 | 80 | 1860 | 7 | 1593 | 72 | 1672 | -3 | -177 | -8 | -188 | -10\% | EB | 14 | 45 | 51 | B | D | D | 45 | D | 661 |  |  | 1506 |  |  |  |  |
| TH 55 \& W Lyndale Ave N |  | NB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | NB | 0 | 0 | 0 | A | A | A | 0 | A | 28 | c |  |  |  |  |  |  |
|  |  | WB | 400 | 1420 | 0 | 1820 | 377 | 1385 | 0 | 1762 | -23 | -35 | 0 | -58 | -3\% | WB | 39 | 1 | 0 | D | A | A | 9 | A |  |  |  | 82 | 85 | 304 |  |  |
|  |  | SB | 30 | 540 | 280 | 850 | 31 | 541 | 271 | 843 | 1 | 1 | -9 | -7 | -1\% | SB | 44 | 44 | 15 | D | D | B | 35 | C |  |  | 69 | 261 |  |  |  |  |
|  |  | EB | 0 | 850 | 808 | 1658 | 0 | 764 | 725 | 1489 | 0 | -86 | -83 | -169 | -10\% | EB | 0 | 20 | 76 | A | C | E | 47 | D |  |  | 492 | 877 |  |  |  |  |
|  | TH 55 \& E Lyndale Ave N | NB | 580 | 800 | 200 | 1580 | 578 | 799 | 205 | 1582 | -2 | -1 | 5 | 2 | 0\% | NB | 26 | 23 | 15 | C | c | B | 23 | C | 25 | c | 73 | 284 |  |  |  |  |
|  |  | WB | 0 | 1240 | 110 | 1350 | 0 | 1185 | 107 | 1292 | 0 | -55 | -3 | -58 | -4\% | WB | 0 | 41 | 9 | A | D | A | 38 | D |  |  | 217 | 521 |  |  |  | 98 |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  |  |  |  |  |  |  |
|  |  | EB | 210 | 670 | 0 | 880 | 208 | 588 | 0 | 796 | -2 | -82 | 0 | -84 | -10\% | EB | 17 | 3 | 0 | B | A | A | 7 | A |  |  |  | 108 |  | 268 |  |  |
| The 55 \& Border/Oak Lake Ave |  | NB | 180 | 100 | 20 | 300 | 172 | 102 | 19 | 293 | -8 | 2 | -1 | -7 | -2\% | NB | 103 | 59 | 11 | F | E | B | 82 | F | 23 | c | 123 | 389 |  |  |  |  |
|  |  | WB | 30 | 900 | 20 | 950 | 29 | 868 | 0 | 897 | -1 | -32 | -20 | -53 | -6\% | WB | 10 | 22 | 0 | B | C | A | 22 | C |  |  | 61 | 429 |  | 50 |  | 101 |
|  |  | SB | 10 | 80 | 270 | 360 | 0 | 75 | 262 | 337 | -10 | -5 | -8 | -23 | -6\% | SB | 16 | 17 | 10 | B | B | B | 12 | B |  |  |  | 330 |  |  |  |  |
|  |  | EB | 190 | 620 | 60 | 870 | 162 | 576 | 57 | 795 | -28 | -44 | -3 | -75 | -9\% | EB | 14 | 6 | 7 | B | A | A | 8 | A |  |  |  | 94 |  | 103 |  |  |
| $\left\|\begin{array}{l} \overline{\mathrm{a}} \\ \stackrel{\rightharpoonup}{\mathrm{j}} \\ \stackrel{y}{2} \end{array}\right\|$ | TH 55/7th St N \& 6th Ave N | NB | 400 | 610 | 40 | 1050 | 388 | 632 | 39 | 1059 | -12 | 22 | -1 | 9 | 1\% | NB | 25 | 14 | 7 | C | B | A | 18 | B | 22 | c |  | 183 | 45 | 321 |  |  |
|  |  | WB | 80 | 510 | 20 | 610 | 81 | 482 | 25 | 588 | 1 | -28 | 5 | -22 | -4\% | WB | 27 | 25 | 9 | c | C | A | 25 | C |  |  | 54 | 239 |  |  |  |  |
|  |  | SB | 30 | 340 | 40 | 410 | 37 | 343 | 32 | 412 | 7 | 3 | -8 | 2 | 0\% | SB | 36 | 30 | 19 | D | c | B | 30 | c |  |  | 26 | 200 |  |  |  |  |
|  |  | EB | 0 | 350 | 300 | 650 | 0 | 308 | 288 | 596 | 0 | -42 | -12 | -54 | -8\% | EB | 0 | 29 | 17 | A | C | B | 23 | c |  |  |  | 162 |  |  |  | 133 |
| $\left.\begin{gathered} \overline{\mathrm{a}} \\ \stackrel{\rightharpoonup}{\bar{j}} \end{gathered} \right\rvert\,$ | 6th Ave N \& HERC Access | NB | 30 | 0 | 20 | 50 | 27 | 0 | 22 | 49 | -3 | 0 | 2 | -1 | -2\% | NB | 35 | 0 | 11 | D | A | B | 24 | C | 4 | A |  | 78 |  |  |  |  |
|  |  | WB | 10 | 560 | 10 | 580 | 11 | 549 | 12 | 572 |  | -11 | 2 | -8 | -1\% | WB | 8 | 2 | 4 | A | A | A | 2 | A |  |  |  | 75 |  |  |  |  |
|  |  | SB | 10 | 0 | 20 | 30 | 11 | 0 | 17 | 28 | 1 | 0 | -3 | -2 | -7\% | SB | 45 | 0 | 17 | D | A | B | 28 | C |  |  |  | 127 |  |  |  |  |
|  |  | EB | 20 | 380 | 20 | 420 | 15 | 353 | 16 | 384 | -5 | -27 | -4 | -36 | -9\% | EB | 1 | 1 | 4 | A | A | A | 1 | A |  |  |  | 54 |  |  |  |  |
| $\left\|\begin{array}{c} \stackrel{0}{0} \\ \stackrel{\rightharpoonup}{2} \\ i \\ i \end{array}\right\|$ | 6th Ave N \& 5th St W | NB | 10 | 150 | 0 | 160 | 8 | 141 | 0 | 149 | -2 | -9 | 0 | -11 | -7\% | NB | 2 | -1 | 0 | A | A | A | -1 | A | 20 | c |  |  |  |  |  |  |
|  |  | WB | 0 | 15 | 25 | 40 | 0 | 22 | 19 | 41 | 0 | 7 | -6 | 1 | 3\% | WB | 0 | 9 | 6 | A | A | A | 8 | A |  |  |  | 40 |  |  |  |  |
|  |  | SB | 0 | 420 | 10 | 430 | 0 | 414 | 10 | 424 | 0 | -6 | 0 | -6 | -1\% | SB | 0 | 29 | 20 | A | D | C | 29 | C |  |  |  |  |  |  |  |  |
|  |  | EB | 10 | 0 | 50 | 60 | 10 | 0 | 48 | 58 | 0 | 0 | -2 | -2 | -3\% | EB | 9 | 0 | 13 | A | A | B | 12 | B |  |  |  | 44 |  |  |  |  |
| $\left\|\begin{array}{l} \frac{0}{0} \\ \stackrel{\rightharpoonup}{0} \\ \stackrel{\rightharpoonup}{5} \end{array}\right\|$ | 6th Ave N \& 4th St W | NB | 20 | 145 | 20 | 185 | 15 | 140 | 15 | 170 | -5 | -5 | -5 | -15 | -8\% | NB | 3 | 1 | 1 | A | A | A | 1 | A | 4 | A |  |  |  |  |  |  |
|  |  | WB | 50 | 30 | 10 | 90 | 48 | 30 | 11 | 89 | -2 | 0 | 1 | -1 | -1\% | WB | 10 | 9 | 9 | B | A | A | 10 | A |  |  |  | 62 |  |  |  |  |
|  |  | SB | 10 | 330 | 10 | 350 | 9 | 329 | 11 | 349 | -1 | - 1 | 1 | -1 | 0\% | SB | 2 | 3 | 2 | A | A | A | 3 | A |  |  |  |  |  |  |  |  |
|  |  | EB | 10 | 10 | 50 | 70 | 10 | 11 | 48 | 69 | 0 | 1 | -2 | -1 | -1\% | EB | 8 | 12 | 11 | A | B | B | 11 | B |  |  |  | 65 |  |  |  |  |
| $\left.\begin{array}{\|c} \overline{\mathrm{a}} \\ \stackrel{\rightharpoonup}{\bar{j}} \end{array} \right\rvert\,$ | 5th St W \& 5th Ave N | NB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | NB | 0 | 0 | 0 | A | A | A | 0 | A | 6 | A |  |  |  |  |  |  |
|  |  | WB | 0 | 110 | 10 | 120 | 0 | 110 | 8 | 118 | 0 | 0 | -2 | -2 | -2\% | WB | 0 | 1 | 2 | A | A | A | 1 | A |  |  |  | 45 |  |  |  |  |
|  |  | SB | 20 | 0 | 90 | 110 | 19 | 0 | 88 | 107 | -1 | 0 | -2 | -3 | -3\% | SB | 46 | 0 | 13 | D | A | B | 19 | B |  |  |  | 141 |  |  |  |  |
|  |  | EB | 190 | 110 | 0 | 300 | 169 | 117 | 0 | 286 | -21 | 7 | 0 | -14 | -5\% | EB | , | 4 | 0 | A | A | A | 3 | A |  |  |  | 167 |  |  |  |  |
|  | TH 55/7th St N \& 5th Ave N | NB | 30 | 1010 | 0 | 1040 | 29 | 1021 | 0 | 1050 | -1 | 11 | 0 | 10 | 1\% | NB | 3 | 0 | 0 | A | A | A | 0 | A | 0 | A |  |  |  |  |  |  |
|  |  | WB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | WB | 0 | 0 | 0 | A | A | A | 0 | A |  |  |  |  |  |  |  |  |
|  |  | SB | 0 | 700 | 20 | 720 | 0 | 694 | 16 | 710 | 0 | -6 | -4 | -10 | -1\% | SB | 0 | 0 | 1 | A | A | A | 0 | A |  |  |  |  |  |  |  |  |
|  |  | EB | 40 | 0 | 10 | 50 | 40 | 0 | 9 | 49 | 0 | 0 | -1 | -1 | -2\% | EB | 9 | 0 | 6 | A | A | A | 8 | A |  |  |  | 46 |  |  |  |  |
| ( 7 th St N \& Oak Lake Ave |  | NB | 100 | 200 | 10 | 310 | 94 | 170 | 0 | 264 | -6 | -30 | -10 | $-46$ | -15\% | NB | 113 | 20 | 0 | F | C | A | 53 | D | 28 | C | 64 | 309 |  |  |  |  |
|  |  | WB | 10 | 550 | 70 | 630 | 0 | 567 | 85 | 652 | -10 | 17 | 15 | 22 | 3\% | WB | 0 | 26 | 24 | A | c | C | 26 | C |  |  | 52 | 274 |  |  |  |  |
|  |  | SB | 40 | 320 | 50 | 410 | 51 | 305 | 54 | 410 | 11 | -15 | 4 | 0 | 0\% | SB | 31 | 31 | 24 | C | C | C | 30 | c |  |  | 44 | 211 |  |  |  |  |
|  |  | EB | 40 | 360 | 30 | 430 | 42 | 361 | 32 | 435 | 2 | 1 | 2 | 5 | 1\% | EB | 14 | 15 | 5 | B | B | A | 14 | B |  |  |  | 126 |  |  |  | 91 |

Alternative 3
Bottineau Corridor
2030 AM Forecast Volumes
Baseline Geometry


Alternative 3
Bottineau Corridor
2030 PM Forecast Volumes
Baseline Geometry


Alternative 4
Bottineau Corridor
2030 AM Forecast Volumes
Baseline Geometry - Trains go with traffic


Alternative 4
Bottineau Corridor
2030 PM Forecast Volumes
Baseline Geometry - Trains go with traffic



Alternative 5
Bottineau Corridor
2030 PM Forecast Volumes



Alternative 6
Bottineau Corridor
2030 PM Forecast Volumes
Average Train Travel Times
EB - 3.1 minutes
Baseline Geometry - Preemption
WB - 3.1 minutes


To
Joe Gladke, PE
Copy: N/A

From: Tony Heppelmann, PE
Chad Ellos, PE
Dean Chamberlain, EIT

Date: June 2011
File: WSB No. 01484-05
Subject: Operations Analysis of LRT near the Terrace Mall
Bottineau Transitway - Robbinsdale
$36^{\text {th }}$ Avenue LRT Alignment
$34^{\text {th }}$ Avenue LRT Alignment

## Introduction

The purpose of this study was to analyze the operational impacts caused by introducing LRT into the area. Three scenarios were analyzed as part of this study. These scenarios include:

- Existing (No Build)
- $36^{\text {th }}$ Avenue LRT Alignment
- $34^{\text {th }}$ Avenue LRT Alignment

Below is a description of each scenario analyzed.

## Existing (No Build)

The Existing scenario represents the 2010 roadway network. Within the study area, eleven key intersections were analyzed, which include:

- $36^{\text {th }}$ Avenue at Halifax Avenue
- $36^{\text {th }}$ Avenue at Grimes Avenue
- $36^{\text {th }}$ Avenue at France Avenue
- $36^{\text {th }}$ Avenue at West Broadway Avenue
- CSAH 81 at $36^{\text {th }}$ Avenue
- CSAH 81 at Abbott Avenue
- $34^{\text {th }}$ Avenue at Halifax Avenue
- $34^{\text {th }}$ Avenue at Grimes Avenue
- $34^{\text {th }}$ Avenue/Oakdale Avenue at France Avenue
- Terrace Mall Access at France Avenue
- $35^{\text {th }}$ Avenue at France Avenue

The study area and existing lane configuration for these key intersections are shown on
Figure 1.

## $36^{\text {th }}$ Avenue LRT Alignment

LRT is proposed to run along the south side of $36^{\text {th }}$ Avenue between the existing BNSF railroad tracks and CSAH 81 using right-of-way that would be acquired from current property owners on the south side of $36^{\text {th }}$ Avenue. The LRT line then turns south to run along CSAH 81 using the right-of-way of the existing West Broadway Avenue, which currently serves as a frontage road for CSAH 81. The LRT line would follow the West Broadway Avenue right-of-way until it meets the intersection of Theodore Wirth Parkway, Victory Memorial Parkway, Lowry Avenue, and Oakdale Avenue (under the existing CSAH 81 bridge). The LRT line would bisect the intersection and proceed to run through the median of CSAH 81 into Minneapolis as shown in Appendix A.

Because of safety and operational concerns, all LRT at-grade roadway crossings would be under signalized control. A number of road accesses would need to close with this option, including: Halifax and Grimes Avenues south of $36^{\text {th }}$ Avenue, West Broadway Avenue frontage road south of $36{ }^{\text {th }}$ Avenue, and West Broadway Avenue frontage road north of Abbott Avenue. To provide access to the Terrace Mall businesses, an additional signalized intersection could be built on CSAH 81 south of $36^{\text {th }}$ Avenue.

Dedicated right-turn and left-turn lanes on approaches paralleling the transitway would need to be defined in order to allow major through movements to proceed through the intersection concurrently with the LRT movements. The intersection of France Avenue and $36{ }^{\text {th }}$ Avenue would need a dedicated westbound left-turn lane and eastbound right-turn lane. Currently these approaches have shared through-left and through-right lanes. The westbound approach would include a left-turn lane and a shared through/right-turn lane. The eastbound approach would include a right-turn lane and a shared through/left-turn lane. These revisions would provide storage for turning vehicles waiting for the LRT movements to clear the intersection.

Some right-turning movements that cross the tracks would not be allowed to turn on red. These include the northbound right-turn at France Avenue and $36^{\text {th }}$ Avenue, the eastbound right-turn at the new entrance to the Terrace Mall and CSAH 81, and the eastbound right-turn at Abbott Avenue and CSAH 81. The remaining right-turn movements that cross the tracks would need to obey LRT actuated blank-out prohibition signs informing vehicles not to cross the tracks when a train is present or approaching. These movements include the eastbound right-
turn at $36^{\text {th }}$ Avenue and France Avenue, the southbound right-turn at the new entrance to the Terrace Mall and CSAH 81, and the southbound right-turn at Abbott Avenue and CSAH 81.

## $34^{\text {th }}$ Avenue LRT Alignment

For this alignment, LRT is proposed to run down the middle of the current $34^{\text {th }}$ Avenue right-ofway from the BNSF railroad tracks to the intersection of France Avenue. The line would then run through the edge of residential backyards and the southern edge of the North Memorial Out-Patient Clinic land to CSAH 81. At CSAH 81, the LRT line would cross over the southbound lanes of CSAH 81 on a grade-separated crossing and proceed to run in the median of CSAH 81 into Minneapolis as shown in Appendix B. This proposed LRT alignment requires further geometric evaluation in order to validate its possible implementation.

To accommodate the LRT line in this alignment, $34^{\text {th }}$ Avenue between Grimes Avenue and France Avenue would be closed. Halifax Avenue would cross the LRT tracks on a gradeseparated structure with the LRT line passing below the existing ground elevation, allowing Halifax Avenue to cross without changing the elevation of the roadway. Between Indiana and Grimes Avenues, $34^{\text {th }}$ Avenue would be realigned adjacent to the LRT tracks with a narrower width and retaining walls to match the existing elevations. Due to safety and operational concerns, the intersection of France Avenue and Oakdale Avenue would be rebuilt as a signalized T-Type intersection. West Broadway Avenue would close between Abbott Avenue and the North Memorial clinic at the south end of the Terrace Mall. The southbound right-turn at France Avenue and $34^{\text {th }}$ Avenue would not be allowed to turn on red.

## Existing Conditions

## Roadway System

The $34^{\text {th }}$ Avenue, $36^{\text {th }}$ Avenue, France Avenue, and CSAH 81 corridors provide access to local neighborhoods and businesses in the City of Robbinsdale and surrounding areas. Commercial retail businesses, office space, medical services, and many single-family residences are located along these corridors and depend on the access provided by these roadways.

CSAH 81 is classified as a minor arterial serving the northwest quadrant of the Twin Cities Metropolitan area. One of CSAH 81's major functions is connecting population centers in the northwest metro area with employment in the City of Minneapolis. CSAH 81 has an average daily traffic (ADT) of 11,700 vehicles according to Mn/DOT 2009 Traffic Flow Maps. $36^{\text {th }}$ Avenue serves as a connector roadway between CSAH 81 and TH 100 (approximately 1 mile to the west) with an ADT of 10,200-16,400 vehicles on various segments in the project area. France Avenue provides an alternate route to North Memorial Medical Center via Oakdale Avenue and has an ADT of 6,900 vehicles in the project area. The remaining streets in the area are local roads providing access to and movement through the nearby residential areas.

## Existing Traffic

In order to provide a base condition for the traffic analysis, turning movement counts were collected at the key intersections. Hennepin County provided peak hour turning movement counts along CSAH 81 at $36^{\text {th }}$ Avenue and Abbott Avenue (June 2009). Turning movements at the other key intersections were collected in November and December 2010. For reference, these counts are provided in Appendix C.

As mentioned in Technical Memorandum 2, a "worst case scenario" was developed to provide a conservative estimate of the peak hour conditions that currently exist on the roadway system. The peak hour traffic volumes for the $36^{\text {th }}$ Avenue and CSAH 81 corridors were derived from the turning movement counts in Appendix C. These peak hour counts were then adjusted to match tube counts on Halifax and Grimes Avenues as well as the two entrances to Terrace Mall on France Avenue taken by WSB in November 2010, which are shown in Appendix D. Finally, the traffic flows were balanced to provide system continuity. The resulting peak hour turning movements at the key intersections are also shown on Figure 1.

## Modeling Methodology

VISSIM computer software was used to model the Existing, the $36^{\text {th }}$ Avenue LRT Alignment, and the $34^{\text {th }}$ Avenue LRT Alignment. VISSIM is a microscopic, time step and behavior based simulation model developed to model urban traffic and public transit operations. It is approved by Mn /DOT as a method to analyze complex transportation systems involving traffic and transit operations under constraints such as lane configuration, traffic composition, traffic signals, transit stops, etc., thus making it a useful tool for the evaluation of various alternatives based on transportation engineering and planning measures of effectiveness.

## Level of Service Definition

The traffic operations analysis results are presented in the form of a letter grade from A to F, called level of service (LOS). The letter grade provides a qualitative assessment of the intersection operations based on the amount of delay per vehicle. The LOS system is set up similar to a report card with "A" representing high quality operations and " $F$ " representing poor operations. At LOS A, motorists experience very little delay or interference. On a roadway or intersection with LOS F conditions, motorists would experience severe congestion and extreme delay, i.e., gridlock. The LOS analysis criteria for signalized and unsignalized intersections are shown in Figure 2. Although LOS A conditions represents the best possible level of traffic flow, the cost to construct intersections to such a high standard exceeds the benefit to the user. Within an urbanized area, it is generally regarded that LOS D or better provides for acceptable operations.

Figure 2: Delay Based LOS Thresholds


## Existing Operations

A summary of the existing level of service conditions is provided below. Appendix E provides more detail as to the individual movements at each intersection and the queue lengths recorded.

Table 1 summarizes the LOS conditions at the key intersections based on the existing lane geometry, Hennepin County and City of Robbinsdale provided signal timing, and 2010 traffic volumes as shown in Figure 1. All intersections are currently operating at LOS C or better during the am and pm peak hours. A few movements in the pm peak hour are operating at LOS D as footnoted in Table 1 and displayed in Appendix E. Adequate storage for queuing vehicles was provided at all approaches. These results are consistent with observed conditions.

Table 1: Intersection LOS - Existing (No Build)

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) |
| Halifax Ave \& 36th Ave | A | 1 | A | 1 |
| Grimes Ave \& 36th Ave | A | 1 | A | 1 |
| France Ave \& 36th Ave | A | 7 | A | 8 |
| W. Broadway Frontage Rd \& 36th Ave | A | 1 | A | 3 |
| CSAH 81 \& 36th Ave | B | 16 | C | $23^{\mathrm{a}}$ |
| CSAH 81 \& Abbott Ave | B | 11 | B | $14^{\mathrm{b}}$ |
| Halifax Ave \& 34th Ave | A | 4 | A | 4 |
| Grimes Ave \& 34th Ave | A | 5 | A | 4 |
| France Ave \& 34th Ave | A | 8 | B | 10 |
| France Ave \& Terrace Mall Access | A | 1 | A | 1 |
| France Ave \& 35th Ave | A | 1 | A | 2 |

${ }^{\text {a }}$ All left-turn movements and the westbound through movement operate at LOS D.
${ }^{\mathrm{b}}$ The eastbound left-turn movement operates at LOS D.

K:101484-05\Admin\Docs\Tech Memos\[LOS Summary Tables.xls]No Build
Source: WSB \& Associates

## Traffic Forecasts

The project is located in a historically built-up area in the City of Robbinsdale, which is an inner suburb of Minneapolis. Based the City's comprehensive plan and other known development plans, no increase in traffic within the study area over the next 20 years was assumed. Historical traffic volumes for the study area, provided in Appendix F, generally supports this assumption by showing that traffic volumes have stayed relatively unchanged.

Year 2030 traffic forecasts for the LRT alignments do include shifting traffic to other intersections based on closures and revisions to the roadway network. The methodology used to reroute traffic based on each LRT alignment is described below.

## $36{ }^{\text {th }}$ Avenue LRT Alignment

Due to the closures previously mentioned for this alignment, affected traffic was rerouted to other roadways according to the following assumptions:

- Halifax and Grimes Avenues south of $36^{\text {th }}$ Avenue: All traffic originating in the area between $34^{\text {th }}$ and $36^{\text {th }}$ Avenues was rerouted onto $35^{\text {th }}$ Avenue to France Avenue. Twothirds of the turning traffic at $36^{\text {th }}$ Avenue originating from the area south of $34^{\text {th }}$ Avenue on both streets was rerouted onto $34^{\text {th }}$ Avenue to France Avenue. The remaining one-third of the turning traffic originating south of $34^{\text {th }}$ Avenue was assumed to have taken $33^{\text {rd }}$ Avenue to France Avenue. This split was assumed because existing traffic control on $34^{\text {th }}$ Avenue allows through traffic to proceed to France Avenue without stopping.
- West Broadway Avenue south of $36^{\text {th }}$ Avenue: Traffic coming from and destined to the east leg of the existing intersection of $36^{\text {th }}$ Avenue and West Broadway Avenue from Terrace Mall was rerouted to the proposed new intersection on CSAH 81. Traffic coming from and destined to the west leg of the intersection was rerouted to the shopping center entrance on France Avenue between $35^{\text {th }}$ and $36^{\text {th }}$ Avenues ( $70 \%$ ), the proposed new intersection on CSAH 81 (20\%), or the shopping center entrance at $35^{\text {th }}$ Avenue and France Avenue (10\%). Factors contributing to these splits were the ease of entering and exiting the shopping center as well as the assumed destination within the shopping center.
- West Broadway Avenue north of Abbott Avenue: Because the traffic on this road is mainly destined for the North Memorial clinic in the south end of the Terrace Mall complex, traffic was diverted to the proposed new intersection on CSAH 81 (75\%) and the shopping center entrance at $35^{\text {th }}$ Avenue and France Avenue via Oakdale Ave (25\%).
- West Broadway Avenue south of Abbott Avenue and at Oakdale Avenue: This segment was not included in the scope of this study.

The results of the shifted traffic are reflected in the forecast turning movements shown in Figure 3.

## $34^{\text {th }}$ Avenue LRT Alignment

Due to the closures previously mentioned for this alignment, affected traffic was rerouted to other roadways according to the following assumptions:

- Grimes Avenue south of $\mathbf{3 4}$ th Avenue: Traffic on Grimes Avenue proceeding to/from the west on $36^{\text {th }}$ Avenue was rerouted to Halifax Avenue to $36^{\text {th }}$ Avenue. Traffic proceeding to/from the east on $36^{\text {th }}$ Avenue and turning to/from the east on $34^{\text {th }}$ Avenue was moved to France Avenue.
- 34th Avenue between Grimes Avenue and France Avenue: Vehicles on $34^{\text {th }}$ Avenue turning to/from the north on France Avenue were rerouted to $35^{\text {th }}$ Avenue to France Avenue, and vehicles on $34^{\text {th }}$ Avenue proceeding straight to/from Oakdale Avenue were moved to $33^{\text {rd }}$ Avenue to France Avenue.
- West Broadway Avenue north of Abbott Avenue: Because the traffic on this road is mainly destined for the North Memorial clinic in the south end of the Terrace Mall complex, traffic was diverted to the proposed new intersection on CSAH 81 (75\%) and the shopping center entrance at $35^{\text {th }}$ Avenue and France Avenue via Oakdale Ave ( $25 \%$ ).

Figure 4 shows the forecast turning movements resulting from these closures.

## Operations Analysis

## General LRT Operating Assumptions

The LRT is assumed to operate at a frequency of 7.5 minutes, in each direction, during the peak hours with a station dwell time of approximately twenty seconds. Random arrival times at the North Memorial Station are based on a mean value of 450 seconds ( 7.5 minutes) with a standard deviation of 90 seconds (representing $\pm 20 \%$ ). The desired speed through the study area is 30 mph . Each LRT train consists of three 94 foot cars with an acceleration rate of 3.0 mph per second and a deceleration rate of 1.5 mph per second. LRT is given priority at traffic signals resulting in minor street phases ending after their minimums are reached and possibly allowing major street through phases to be extended beyond their normal maximums in order to minimize LRT delay. Under traffic signal priority, the LRT vehicles may need to slow down and/or stop at intersections thus experiencing some delay through the study area.

Analysis data from the models was collected for both the traffic and LRT traveling through the study area. A summary of the levels of service conditions for each scenario is provided below. Appendix E provides more detail as to the individual movements at each intersection, the queue lengths recorded, and LRT travel times through the study area.

## Traffic LOS Conditions

$36^{\text {th }}$ Avenue LRT Alignment
Table 2 summarizes the LOS conditions based on the revised lane geometry and redistributed traffic volumes as shown in Figure 3. Redistributed traffic from closed approaches prompted the optimization of signal timings. All intersections operate at LOS B or better during the am and pm peak hours. A few movements in the pm peak hour are operating at LOS D as footnoted in Table 2.

Queue lengths at the new CSAH 81 intersection to the Terrace Mall were reviewed to determine if vehicles were backing up into the CSAH 81 and $36^{\text {th }}$ Avenue intersection affecting signal operations. The maximum queues recorded during the peak hours did not exceed 200 feet and thus were not affecting nearby signal operations. The northbound left-turn storage on France Avenue at $36^{\text {th }}$ Avenue was exceeded a few instances during the pm peak hour. Currently the storage is approximately 150 feet. Left-turning vehicles temporarily block through and right-turning vehicles until the signal turns green allowing the queue to dissipate. This left-turn storage should be monitored and possibly extended should this LRT alignment be implemented. The resulting LOS conditions were similar to the Existing (No Build) conditions.

Table 2: Intersection LOS - $36^{\text {th }}$ Avenue LRT Alignment

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) |
| Halifax Ave \& 36th Ave | A | 1 | A | 1 |
| Grimes Ave \& 36th Ave | A | 1 | A | 1 |
| France Ave \& 36th Ave | B | 12 | B | 14 |
| W. Broadway Frontage Rd \& 36th Ave | A | 1 | A | 1 |
| CSAH 81 \& 36th Ave | B | $16^{\mathrm{a}}$ | B | $19^{\mathrm{d}}$ |
| CSAH 81 \& Abbott Ave | A | $9^{\mathrm{b}}$ | B | $12^{\mathrm{e}}$ |
| Halifax Ave \& 34th Ave | A | 2 | A | 1 |
| Grimes Ave \& 34th Ave | A | 2 | A | 1 |
| France Ave \& 34th Ave | A | 9 | B | 11 |
| France Ave \& Terrace Mall Access | A | 1 | A | 2 |
| France Ave \& 35th Ave | A | 2 | A | 3 |
| CSAH 81 \& New Terrace Mall Access | A | $9^{\mathrm{c}}$ | B | $14^{\mathrm{f}}$ |

${ }^{\text {a }}$ All left-turn movements and the westbound through movement operate at LOS D.
${ }^{\mathrm{b}}$ The northbound left, eastbound left, and eastbound right-turn movements operate at LOS D.
${ }^{\text {c }}$ The northbound left, eastbound left, and eastbound right-turn movements operate at LOS D.
${ }^{\mathrm{d}}$ The NB, WB, and SB left-turn movements and the WB through movement operate at LOS D.
${ }^{\text {e }}$ The northbound left, eastbound left, and eastbound right-turn movements operate at LOS D.
${ }^{\text {f }}$ The northbound left, eastbound left, and eastbound right-turn movements operate at LOS D.

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Source: WSB \& Associates

## $34^{\text {th }}$ Avenue LRT Alignment

Table 3 summarizes the LOS conditions based on the revised lane geometry and redistributed traffic volumes as shown in Figure 4. Redistributed traffic from closed approaches again prompted the optimization of signal timings. All intersections operate at LOS C or better during the am and pm peak hours. A few movements in the pm peak hour are operating at LOS D as footnoted in Table 3. The northbound left-turn storage on France Avenue at $36^{\text {th }}$ Avenue was exceeded a few instances during the pm peak hour. Left-turning vehicles temporarily block through and right-turning vehicles until the signal turns green allowing the queue to dissipate. This left-turn storage should be monitored and possibly extended should this LRT alignment be implemented. The resulting LOS conditions were similar to the Existing (No Build) conditions.

Table 3: Intersection LOS - $34^{\text {th }}$ Avenue LRT Alignment

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Delay <br> (sec/veh) |
| Halifax Ave \& 36th Ave | A | 1 | A | 1 |
| Grimes Ave \& 36th Ave | A | 1 | A | 1 |
| France Ave \& 36th Ave | A | 7 | A | 9 |
| W. Broadway Frontage Rd \& 36th Ave | A | 1 | A | 3 |
| CSAH 81 \& 36th Ave | B | 17 | C | $21^{\mathrm{b}}$ |
| CSAH 81 \& Abbott Ave | A | $9^{\mathrm{a}}$ | B | $16^{\text {c }}$ |
| Halifax Ave \& 34th Ave | A | 1 | A | 1 |
| France Ave \& Oakdale Ave | B | 11 | B | 11 |
| France Ave \& Terrace Mall Access | A | 1 | A | 1 |
| France Ave \& 35th Ave | A | 2 | A | 2 |

${ }^{\text {a }}$ The northbound left-turn movement operates at LOS D.
${ }^{\mathrm{b}}$ The WB, SB, and EB left-turn movements and the WB through movement operate at LOS D.
${ }^{\text {c }}$ The northbound and eastbound left-turn movements operate at LOS D.

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Source: WSB \& Associates

## LRT Travel Times

The LRT train travel time through the study area was collected for comparison between the $36^{\text {th }}$ Avenue and $34^{\text {th }}$ Avenue alignments. The travel time was calculated from a point just north of 36 th Avenue to just south of Abbott Avenue.
$36^{\text {th }}$ Avenue LRT Alignment
The distance following the $36^{\text {th }}$ Avenue LRT alignment was approximately 4,645 feet. On average, it took an LRT train 157 seconds to travel through the study area in the am peak hour and 151 seconds in the pm peak hour. Approximately 50 seconds ${ }^{1}$ was related to stopping at the North Memorial Station and 17 seconds (am peak) and 11 seconds ( pm peak) were related to intersection delay.

## $34^{\text {th }}$ Avenue LRT Alignment

The distance following the $34^{\text {th }}$ Avenue LRT alignment was approximately 5,475 feet. On average, it took an LRT train 172 seconds to travel through the study area in the am peak hour and 173 seconds in the pm peak hour. Approximately 50 seconds ${ }^{2}$ was related to stopping at

[^2]the North Memorial Station and 8 seconds (am peak) and 9 seconds ( pm peak) were related to intersection delay.

The $34^{\text {th }}$ Avenue alignment is approximately 830 feet longer than the $36^{\text {th }}$ Avenue alignment. On average, an LRT train traveling on the $34^{\text {th }}$ Avenue alignment takes an additional 15 seconds in the am peak and 22 seconds in the pm peak hour to travel through the study area as compared to traveling on the $36^{\text {th }}$ Avenue alignment. These average travel times are also provided in Appendix E.

## Additional Analyses

## Sensitivity Analysis

Since the traffic forecasts were essentially the same as the existing counts, a sensitivity analysis was also performed to determine if an increase in forecasted traffic would result in acceptable conditions. Even though traffic volumes in the study area are not anticipated to increase in the future, traffic on TH 100 is forecasted to increase. Congestion on TH 100 could lead to an increase in vehicles using CSAH 81 and $36^{\text {th }}$ Avenue during peak traffic hours. Revised 2030 forecasts for this analysis were developed assuming an annual increase of one percent per year. These forecasts were then rerouted using the same methodology as stated previously based on closures and revisions to the roadway network for each LRT alignment scenario.

The results of the sensitivity analysis were similar to the original analysis, producing acceptable conditions under both the $36^{\text {th }}$ Avenue and $34^{\text {th }}$ Avenue scenarios. Tables of these results are provided in Appendix G.

## Weekend Traffic Analysis

Terrace Mall management was concerned about traffic operations on the weekend since customer volume using the Terrace Mall shopping center was highest on the weekend. Since no current weekend traffic data was available, the following methodology was used to compare the highest weekend hourly traffic to the weekday PM peak hour.

Using Automatic Traffic Recorder (ATR) data from the TH 100 and $36^{\text {th }}$ Avenue freeway ramps, it was concluded that the highest hourly weekday traffic in the area occurs between 4:30 pm and 5:30 pm, the highest hourly Saturday traffic between $3: 30 \mathrm{pm}$ and $4: 30 \mathrm{pm}$ and the highest hourly Sunday traffic between noon and 1:00 pm. Saturday's peak hour was $78 \%$ of the weekday PM peak hour and Sunday's peak hour was $76 \%$ of the weekday PM peak hour. It was concluded that traffic in the area on Saturday and Sunday is generally $80 \%$ of what could be expected during the weekday PM peak hour.

A review of the ITE trip generation rates for Saturday traffic revealed that the weekday PM peak hour generated approximately the same number of trips as the Saturday peak hour. Therefore, since the roadway network on Saturdays is expected to experience approximately $20 \%$ less
traffic and the trips produced during the peak hour are essentially the same as the weekday PM peak hour, the operations and LOS conditions at the area intersection is expected to be the same or better than those reported for the weekday PM peak hour. Refer to Appendix $\mathbf{H}$ for ATR and ITE data used in this analysis.

## Conclusions

- Both the $36^{\text {th }}$ Avenue and $34^{\text {th }}$ Avenue LRT scenarios produce acceptable LOS conditions and queue lengths at all the area intersections.
- The sensitivity analysis, which included revising the 2030 forecasts to include an annual traffic increase of one percent, produced acceptable LOS conditions and queue lengths at all intersection.
- The $34^{\text {th }}$ Avenue LRT alignment requires further geometric evaluation in order to validate its possible implementation.
- The highest hourly traffic volumes on the weekend are not anticipated to exceed the weekday PM peak hour traffic volumes (highest weekend peak hour is approximately $80 \%$ of the weekday PM peak hour).
- With the $36^{\text {th }}$ Avenue LRT alignment, a new intersection on CSAH 81 between $36^{\text {th }}$ Avenue and Abbott Avenue will experience acceptable LOS conditions. Queue lengths at this intersection are expected to be short and thus not effecting operations at the CSAH $81 / 36^{\text {th }}$ Avenue intersection.


Figure 1
No Build
Existing Turning Movements and Lane Configuration


Figure 3
36 ${ }^{\text {th }}$ Avenue LRT Alignment 2030 Turning Movement Forecasts and Lane Configuration


Figure 4
34 ${ }^{\text {th }}$ Avenue LRT Alignment 2030 Turning Movement Forecasts and Lane Configuration

## Appendix A

## $36^{\text {th }}$ Avenue LRT Alignment



## Appendix B

## $34^{\text {th }}$ Avenue LRT Alignment



## Appendix C

## Turning Movement Counts

(Taken by WSB or supplied by Hennepin County)

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

36th Ave \& Halifax Ave Turning Movement Robbinsdale, MN

File Name : 36th ave \& halifax ave
Site Code : 00000001
Start Date : 11/30/2010
Page No : 1

Groups Printed- Unshifted

|  | Halifax Ave From North |  |  |  |  | 36th Ave From East |  |  |  |  | Halifax Ave From South |  |  |  |  | 36th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:15 AM | 2 | 0 | 0 | 0 | 2 | 0 | 87 | 1 | 0 | 88 | 0 | 0 | 8 | 0 | 8 | 0 | 95 | 0 | 0 | 95 | 193 |
| 07:30 AM | 6 | 1 | 1 | 0 | 8 | 1 | 95 | 1 | 0 | 97 | 1 | 0 | 6 | 0 | 7 | 3 | 111 | 0 | 0 | 114 | 226 |
| 07:45 AM | 1 | 2 | 1 | 0 | 4 | 0 | 89 | 0 | 0 | 89 | 2 | 0 | 6 | 0 | 8 | 4 | 113 | 0 | 0 | 117 | 218 |
| Total | 9 | 3 | 2 | 0 | 14 | 1 | 271 | 2 | 0 | 274 | 3 | 0 | 20 | 0 | 23 | 7 | 319 | 0 | 0 | 326 | 637 |
| 08:00 AM | 1 | 0 | 0 | 0 | 1 | 2 | 74 | 0 | 0 | 76 | 1 | 0 | 3 | 0 | 4 | 1 | 106 | 1 | 0 | 108 | 189 |
| 08:15 AM | 0 | 0 | 2 | 0 | 2 | 1 | 62 | 1 | 0 | 64 | 1 | 0 | 9 | 0 | 10 | 1 | 108 | 1 | 0 | 110 | 186 |
| 08:30 AM | 2 | 0 | 0 | 0 | 2 | 2 | 76 | 0 | 0 | 78 | 0 | 0 | 5 | 0 | 5 | 3 | 96 | 0 | 0 | 99 | 184 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 3 | 0 | 2 | 0 | 5 | 5 | 212 | 1 | 0 | 218 | 2 | 0 | 17 | 0 | 19 | 5 | 310 | 2 | 0 | 317 | 559 |

*** BREAK ***

| 04:15 PM | 1 | 0 | 1 | 1 | 3 | 2 | 123 | 0 | 0 | 125 | 1 | 0 | 3 | 0 | 4 | 6 | 99 | 2 | 0 | 107 | 239 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 2 | 0 | 2 | 0 | 4 | 2 | 132 | 0 | 0 | 134 | 1 | 0 | 3 | 0 | 4 | 6 | 116 | 1 | 0 | 123 | 265 |
| 04:45 PM | 2 | 0 | 0 | 0 | 2 | 1 | 132 | 2 | 0 | 135 | 3 | 0 | 4 | 0 | 7 | 15 | 127 | 1 | 0 | 143 | 287 |
| Total | 5 | 0 | 3 | 1 | 9 | 5 | 387 | 2 | 0 | 394 | 5 | 0 | 10 | 0 | 15 | 27 | 342 | 4 | 0 | 373 | 791 |
| 05:00 PM | 1 | 0 | 0 | 0 | 1 | 3 | 133 | 0 | 0 | 136 | 1 | 0 | 7 | 0 | 8 | 12 | 99 | 1 | 0 | 112 | 257 |
| 05:15 PM | 5 | 0 | 0 | 0 | 5 | 0 | 128 | 0 | 0 | 128 | 1 | 0 | 7 | 0 | 8 | 11 | 102 | 2 | 0 | 115 | 256 |
| 05:30 PM | 2 | 0 | 0 | 0 | 2 | 2 | 96 | 1 | 0 | 99 | 0 | 0 | 5 | 0 | 5 | 2 | 103 | 4 | 0 | 109 | 215 |
| Grand Total | 25 | 3 | 7 | 1 | 36 | 16 | 1227 | 6 | 0 | 1249 | 12 | 0 | 66 | 0 | 78 | 64 | 1275 | 13 | 0 | 1352 | 2715 |
| Apprch \% | 69.4 | 8.3 | 19.4 | 2.8 |  | 1.3 | 98.2 | 0.5 | 0 |  | 15.4 | 0 | 84.6 | 0 |  | 4.7 | 94.3 | 1 | 0 |  |  |
| Total \% | 0.9 | 0.1 | 0.3 | 0 | 1.3 | 0.6 | 45.2 | 0.2 | 0 | 46 | 0.4 | 0 | 2.4 | 0 | 2.9 | 2.4 | 47 | 0.5 | 0 | 49.8 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

36th Ave \& Grimes Ave
Turning Movement Robbinsdale, MN

File Name : 36th ave \& grimes ave
Site Code : 00000002
Start Date : 12/1/2010
Page No : 1

Groups Printed- Unshifted

|  | Grimes Ave From North |  |  |  |  | 36th Ave From East |  |  |  |  | Grimes Ave From South |  |  |  |  | 36th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:15 AM | 7 | 1 | 2 | 0 | 10 | 1 | 70 | 0 | 0 | 71 | 0 | 1 | 5 | 0 | 6 | 2 | 104 | 3 | 0 | 109 | 196 |
| 07:30 AM | 4 | 1 | 5 | 0 | 10 | 3 | 97 | 1 | 0 | 101 | 0 | 0 | 8 | 0 | 8 | 3 | 130 | 4 | 0 | 137 | 256 |
| 07:45 AM | 6 | 0 | 2 | 0 | 8 | 1 | 94 | 1 | 0 | 96 | 1 | 0 | 2 | 0 | 3 | 2 | 126 | 4 | 0 | 132 | 239 |
| Total | 17 | 2 | 9 | 0 | 28 | 5 | 261 | 2 | 0 | 268 | 1 | 1 | 15 | 0 | 17 | 7 | 360 | 11 | 0 | 378 | 691 |


| 08:00 AM | 1 | 1 | 2 | 0 | 4 | 1 | 74 | 1 | 0 | 76 | 2 | 0 | 3 | 0 | 5 | 3 | 93 | 3 | 0 | 99 | 184 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 08:15 AM | 6 | 0 | 1 | 0 | 7 | 0 | 63 | 1 | 0 | 64 | 0 | 1 | 1 | 0 | 2 | 0 | 95 | 4 | 0 | 99 | 172 |
| 08:30 AM | 10 | 1 | 3 | 0 | 14 | 0 | 64 | 0 | 0 | 64 | 0 | 1 | 6 | 0 | 7 | 5 | 72 | 4 | 0 | 81 | 166 |
| *** BREAK *** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 17 | 2 | 6 | 0 | 25 | 1 | 201 | 2 | 0 | 204 | 2 | 2 | 10 | 0 | 14 | 8 | 260 | 11 | 0 | 279 | 522 |

*** BREAK ***

| 04:15 PM | 3 | 2 | 2 | 0 | 7 | 3 | 118 | 4 | 0 | 125 | 1 | 0 | 4 | 0 | 5 | 8 | 103 | 4 | 0 | 115 | 252 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 1 | 3 | 1 | 0 | 5 | 4 | 112 | 1 | 0 | 117 | 2 | 0 | 4 | 0 | 6 | 7 | 112 | 2 | 0 | 121 | 249 |
| 04:45 PM | 5 | 0 | 3 | 0 | 8 | 1 | 144 | 1 | 0 | 146 | 0 | 1 | 6 | 0 | 7 | 6 | 130 | 10 | 0 | 146 | 307 |
| Total | 9 | 5 | 6 | 0 | 20 | 8 | 374 | 6 | 0 | 388 | 3 | 1 | 14 | 0 | 18 | 21 | 345 | 16 | 0 | 382 | 808 |
| 05:00 PM | 5 | 5 | 2 | 0 | 12 | 4 | 120 | 0 | 0 | 124 | 4 | 1 | 2 | 0 | 7 | 5 | 98 | 7 | 0 | 110 | 253 |
| 05:15 PM | 8 | 2 | 3 | 0 | 13 | 2 | 108 | 1 | 0 | 111 | 4 | 0 | 3 | 0 | 7 | 4 | 110 | 4 | 0 | 118 | 249 |
| 05:30 PM | 3 | 1 | 1 | 0 | 5 | 3 | 91 | 2 | 0 | 96 | 1 | 2 | 3 | 0 | 6 | 5 | 126 | 5 | 0 | 136 | 243 |
| Grand Total | 59 | 17 | 27 | 0 | 103 | 23 | 1155 | 13 | 0 | 1191 | 15 | 7 | 47 | 0 | 69 | 50 | 1299 | 54 | 0 | 1403 | 2766 |
| Apprch \% | 57.3 | 16.5 | 26.2 | 0 |  | 1.9 | 97 | 1.1 | 0 |  | 21.7 | 10.1 | 68.1 | 0 |  | 3.6 | 92.6 | 3.8 | 0 |  |  |
| Total \% | 2.1 | 0.6 | 1 | 0 | 3.7 | 0.8 | 41.8 | 0.5 | 0 | 43.1 | 0.5 | 0.3 | 1.7 | 0 | 2.5 | 1.8 | 47 | 2 | 0 | 50.7 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN
36th Ave \& France Ave
File Name: 36th ave \& france
Turning Movement
Site Code : 00000003
Start Date : 11/30/2010
Page No : 1

Groups Printed- Unshifted

|  | France Ave From North |  |  |  |  | 36th Ave From East |  |  |  |  | France Ave From South |  |  |  |  | 36th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:15 AM | 4 | 14 | 14 | 2 | 34 | 4 | 50 | 3 | 0 | 57 | 5 | 4 | 22 | 0 | 31 | 32 | 55 | 1 | 1 | 89 | 211 |
| 07:30 AM | 3 | 22 | 7 | 4 | 36 | 2 | 46 | 7 | 1 | 56 | 7 | 15 | 37 | 0 | 59 | 51 | 57 | 5 | 0 | 113 | 264 |
| 07:45 AM | 3 | 16 | 6 | 0 | 25 | 3 | 48 | 13 | 0 | 64 | 10 | 17 | 33 | 0 | 60 | 46 | 56 | 9 | 1 | 112 | 261 |
| Total | 10 | 52 | 27 | 6 | 95 | 9 | 144 | 23 | 1 | 177 | 22 | 36 | 92 | 0 | 150 | 129 | 168 | 15 | 2 | 314 | 736 |
| 08:00 AM | 5 | 19 | 7 | 3 | 34 | 3 | 37 | 8 | 0 | 48 | 6 | 8 | 28 | 1 | 43 | 40 | 66 | 5 | 0 | 111 | 236 |
| 08:15 AM | 1 | 21 | 6 | 1 | 29 | 3 | 26 | 3 | 0 | 32 | 10 | 11 | 31 | 0 | 52 | 46 | 61 | 2 | 1 | 110 | 223 |
| 08:30 AM | 7 | 9 | 6 | 0 | 22 | 6 | 45 | 6 | 1 | 58 | 5 | 7 | 23 | 1 | 36 | 37 | 52 | 6 | 0 | 95 | 211 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 13 | 49 | 19 | 4 | 85 | 12 | 108 | 17 | 1 | 138 | 21 | 26 | 82 | 2 | 131 | 123 | 179 | 13 | 1 | 316 | 670 |

*** BREAK ***

| 04:15 PM | 13 | 19 | 12 | 1 | 45 | 5 | 50 | 4 | 0 | 59 | 11 | 29 | 59 | 1 | 100 | 32 | 52 | 7 | 0 | 91 | 295 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 4 | 25 | 13 | 0 | 42 | 7 | 71 | 5 | 0 | 83 | 8 | 28 | 61 | 0 | 97 | 28 | 76 | 9 | 0 | 113 | 335 |
| 04:45 PM | 12 | 16 | 4 | 4 | 36 | 6 | 74 | 10 | 2 | 92 | 7 | 34 | 43 | 2 | 86 | 36 | 66 | 10 | 0 | 112 | 326 |
| Total | 29 | 60 | 29 | 5 | 123 | 18 | 195 | 19 | 2 | 234 | 26 | 91 | 163 | 3 | 283 | 96 | 194 | 26 | 0 | 316 | 956 |
| 05:00 PM | 11 | 23 | 13 | 1 | 48 | 5 | 58 | 1 | 2 | 66 | 11 | 29 | 71 | 0 | 111 | 29 | 59 | 8 | 2 | 98 | 323 |
| 05:15 PM | 7 | 17 | 6 | 2 | 32 | 10 | 66 | 5 | 0 | 81 | 12 | 18 | 43 | 1 | 74 | 33 | 57 | 9 | 0 | 99 | 286 |
| 05:30 PM | 7 | 22 | 12 | 2 | 43 | 10 | 48 | 4 | 1 | 63 | 7 | 15 | 41 | 1 | 64 | 24 | 58 | 4 | 0 | 86 | 256 |
| Grand Total | 77 | 223 | 106 | 20 | 426 | 64 | 619 | 69 | 7 | 759 | 99 | 215 | 492 | 7 | 813 | 434 | 715 | 75 | 5 | 1229 | 3227 |
| Apprch \% | 18.1 | 52.3 | 24.9 | 4.7 |  | 8.4 | 81.6 | 9.1 | 0.9 |  | 12.2 | 26.4 | 60.5 | 0.9 |  | 35.3 | 58.2 | 6.1 | 0.4 |  |  |
| Total \% | 2.4 | 6.9 | 3.3 | 0.6 | 13.2 | 2 | 19.2 | 2.1 | 0.2 | 23.5 | 3.1 | 6.7 | 15.2 | 0.2 | 25.2 | 13.4 | 22.2 | 2.3 | 0.2 | 38.1 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

36th Ave \& W Broadway Ave
Turning Movement
Robbinsdale, MN

File Name : 36th ave \& w broadway ave
Site Code : 00000004
Start Date : 12/1/2010
Page No : 1

Groups Printed- Unshifted

|  | Ent to Parking lot From North |  |  |  |  | 36th Ave From East |  |  |  |  | W Broadway Ave From South |  |  |  |  | 36th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:15 AM | 0 | 0 | 0 | 1 | 1 | 0 | 46 | 25 | 0 | 71 | 7 | 0 | 4 | 0 | 11 | 10 | 56 | 0 | 0 | 66 | 149 |
| 07:30 AM | 0 | 0 | 0 | 2 | 2 | 0 | 63 | 34 | 0 | 97 | 5 | 0 | 3 | 0 | 8 | 8 | 80 | 0 | 0 | 88 | 195 |
| 07:45 AM | 0 | 0 | 0 | 2 | 2 | 0 | 62 | 44 | 0 | 106 | 4 | 0 | 2 | 0 | 6 | 15 | 75 | 0 | 0 | 90 | 204 |
| Total | 0 | 0 | 0 | 5 | 5 | 0 | 171 | 103 | 0 | 274 | 16 | 0 | 9 | 0 | 25 | 33 | 211 | 0 | 0 | 244 | 548 |


| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 32 | 0 | 76 | 8 | 0 | 3 | 0 | 11 | 8 | 57 | 1 | 0 | 66 | 153 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 08:15 AM | 0 | 0 | 1 | 1 | 2 | 0 | 44 | 30 | 0 | 74 | 10 | 0 | 4 | 0 | 14 | 13 | 57 | 0 | 0 | 70 | 160 |
| 08:30 AM | 0 | 0 | 0 | 1 | 1 | 0 | 46 | 32 | 0 | 78 | 12 | 0 | 11 | 0 | 23 | 5 | 44 | 0 | 0 | 49 | 151 |
| *** BREAK *** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 0 | 0 | 1 | 2 | 3 | 0 | 134 | 94 | 0 | 228 | 30 | 0 | 18 | 0 | 48 | 26 | 158 | 1 | 0 | 185 | 464 |

*** BREAK ***

| 04:15 PM | 0 | 1 | 0 | 0 | 1 | 0 | 63 | 27 | 0 | 90 | 43 | 0 | 19 | 0 | 62 | 16 | 73 | 2 | 0 | 91 | 244 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 2 | 58 | 22 | 0 | 82 | 36 | 0 | 17 | 0 | 53 | 12 | 72 | 0 | 0 | 84 | 219 |
| 04:45 PM | 2 | 0 | 1 | 1 | 4 | 0 | 68 | 20 | 0 | 88 | 42 | 2 | 14 | 0 | 58 | 16 | 86 | 3 | 0 | 105 | 255 |
| Total | 2 | 1 | 1 | 1 | 5 | 2 | 189 | 69 | 0 | 260 | 121 | 2 | 50 | 0 | 173 | 44 | 231 | 5 | 0 | 280 | 718 |
| 05:00 PM | 1 | 0 | 2 | 0 | 3 | 0 | 68 | 20 | 0 | 88 | 52 | 0 | 15 | 0 | 67 | 14 | 70 | 2 | 1 | 87 | 245 |
| 05:15 PM | 2 | 0 | 2 | 0 | 4 | 2 | 61 | 21 | 0 | 84 | 30 | 1 | 13 | 0 | 44 | 6 | 85 | 1 | 0 | 92 | 224 |
| 05:30 PM | 1 | 0 | 1 | 1 | 3 | 2 | 65 | 20 | 0 | 87 | 23 | 0 | 8 | 0 | 31 | 9 | 90 | 1 | 0 | 100 | 221 |
| Grand Total | 6 | 1 | 7 | 9 | 23 | 6 | 688 | 327 | 0 | 1021 | 272 | 3 | 113 | 0 | 388 | 132 | 845 | 10 | 1 | 988 | 2420 |
| Apprch \% | 26.1 | 4.3 | 30.4 | 39.1 |  | 0.6 | 67.4 | 32 | 0 |  | 70.1 | 0.8 | 29.1 | 0 |  | 13.4 | 85.5 | 1 | 0.1 |  |  |
| Total \% | 0.2 | 0 | 0.3 | 0.4 | 1 | 0.2 | 28.4 | 13.5 | 0 | 42.2 | 11.2 | 0.1 | 4.7 | 0 | 16 | 5.5 | 34.9 | 0.4 | 0 | 40.8 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

34th Ave \& Halifax Ave Turning Movement Robbinsdale, MN

File Name : 34th ave \& halfax ave
Site Code : 00000007
Start Date : 11/30/2010
Page No : 1

Groups Printed- Unshifted

|  | Halifax Ave From North |  |  |  |  | 34th Ave From East |  |  |  |  | Halifax Ave From South |  |  |  |  | 34th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 07:15 AM | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 0 | 2 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 9 |
| 07:30 AM | 0 | 3 | 5 | 0 | 8 | 0 | 1 | 0 | 0 | 1 | 2 | 2 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 13 |
| 07:45 AM | 1 | 2 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 0 | 1 | 2 | 0 | 3 | 11 |
| Total | 1 | 5 | 7 | 0 | 13 | 1 | 2 | 0 | 0 | 3 | 2 | 12 | 0 | 0 | 14 | 0 | 1 | 2 | 0 | 3 | 33 |
| 08:00 AM | 1 | 0 | 1 | 0 | 2 | 1 | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 2 | 7 |
| 08:15 AM | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 9 |
| 08:30 AM | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 0 | 1 | 0 | 0 | 1 | 6 |
| Total | 2 | 2 | 1 | 0 | 5 | 1 | 0 | 1 | 0 | 2 | 0 | 12 | 0 | 0 | 12 | 0 | 3 | 0 | 0 | 3 | 22 |


| 04:15 PM | 1 | 3 | 1 | 0 | 5 | 0 | 1 | 1 | 0 | 2 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 1 | 3 | 1 | 0 | 5 | 2 | 0 | 1 | 0 | 3 | 0 | 2 | 0 | 0 | 2 | 0 | 2 | 1 | 0 | 3 | 13 |
| 04:45 PM | 1 | 5 | 3 | 0 | 9 | 1 | 1 | 4 | 0 | 6 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 16 |
| Total | 3 | 11 | 5 | 0 | 19 | 3 | 2 | 6 | 0 | 11 | 1 | 3 | 0 | 0 | 4 | 0 | 3 | 1 | 0 | 4 | 38 |


| 05:00 PM | 3 | 4 | 0 | 0 | 7 | 1 | 0 | 1 | 0 | 2 | 1 | 4 | 0 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 15 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 05:15 PM | 1 | 4 | 1 | 0 | 6 | 1 | 1 | 0 | 0 | 2 | 2 | 3 | 0 | 0 | 5 | 0 | 1 | 0 | 0 | 1 | 14 |
| 05:30 PM | 1 | 2 | 1 | 0 | 4 | 2 | 3 | 0 | 0 | 5 | 1 | 2 | 0 | 0 | 3 | 0 | 1 | 0 | 0 | 1 | 13 |
| Grand Total | 11 | 28 | 15 | 0 | 54 | 9 | 8 | 8 | 0 | 25 | 7 | 36 | 0 | 0 | 43 | 0 | 9 | 4 | 0 | 13 | 135 |
| Apprch \% | 20.4 | 51.9 | 27.8 | 0 |  | 36 | 32 | 32 | 0 |  | 16.3 | 83.7 | 0 | 0 |  | 0 | 69.2 | 30.8 | 0 |  |  |
| Total \% | 8.1 | 20.7 | 11.1 | 0 | 40 | 6.7 | 5.9 | 5.9 | 0 | 18.5 | 5.2 | 26.7 | 0 | 0 | 31.9 | 0 | 6.7 | 3 | 0 | 9.6 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN
34th Ave \& Grimes Ave
File Name: 34th ave \& grimes ave
Turning Movement
Site Code : 00000006
Start Date : 12/1/2010
Page No : 1

Groups Printed- Unshifted

|  | Grimes Ave From North |  |  |  |  | 34th Ave From East |  |  |  |  | Grimes Ave From South |  |  |  |  | 34th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 07:15 AM | 0 | 1 | 0 | 0 | 1 | 2 | 0 | 1 | 0 | 3 | 1 | 1 | 0 | 0 | 2 | 0 | 8 | 1 | 0 | 9 | 15 |
| 07:30 AM | 2 | 5 | 2 | 0 | 9 | 0 | 0 | 1 | 0 | 1 | 2 | 7 | 0 | 0 | 9 | 0 | 5 | 0 | 0 | 5 | 24 |
| 07:45 AM | 0 | 2 | 0 | 0 | 2 | 1 | 1 | 0 | 0 | 2 | 1 | 1 | 0 | 0 | 2 | 0 | 5 | 0 | 0 | 5 | 11 |
| Total | 2 | 8 | 2 | 0 | 12 | 3 | 1 | 2 | 0 | 6 | 4 | 9 | 0 | 0 | 13 | 0 | 18 | 1 | 0 | 19 | 50 |
| 08:00 AM | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 1 | 4 | 0 | 0 | 5 | 0 | 1 | 0 | 0 | 1 | 11 |
| 08:15 AM | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 4 | 3 | 0 | 0 | 7 | 0 | 2 | 0 | 0 | 2 | 11 |
| 08:30 AM | 0 | 6 | 0 | 0 | 6 | 1 | 0 | 1 | 0 | 2 | 3 | 6 | 1 | 0 | 10 | 0 | 3 | 0 | 0 | 3 | 21 |
| Total | 0 | 11 | 1 | 0 | 12 | 1 | 0 | 2 | 0 | 3 | 8 | 13 | 1 | 0 | 22 | 0 | 6 | 0 | 0 | 6 | 43 |


| $04: 15 ~ P M ~$ | 0 | 7 | 2 | 0 | 9 | 0 | 3 | 2 | 0 | 5 | 5 | 4 | 0 | 0 | 9 | 0 | 0 | 1 | 0 | 1 | 24 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $04: 30 ~ P M ~$ | 1 | 7 | 2 | 0 | 10 | 1 | 0 | 2 | 0 | 3 | 0 | 3 | 0 | 1 | 4 | 0 | 1 | 0 | 2 | 3 | 20 |
| $04: 45 \mathrm{PM}$ | 1 | 2 | 0 | 1 | 4 | 0 | 1 | 0 | 0 | 1 | 1 | 7 | 0 | 2 | 10 | 1 | 0 | 1 | 3 | 5 | 20 |
| Total | 2 | 16 | 4 | 1 | 23 | 1 | 4 | 4 | 0 | 9 | 6 | 14 | 0 | 3 | 23 | 1 | 1 | 2 | 5 | 9 | 64 |


| 05:00 PM | 1 | 4 | 1 | 0 | 6 | 0 | 5 | 5 | 0 | 10 | 0 | 3 | 0 | 0 | 3 | 0 | 1 | 0 | 0 | 1 | 20 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 05:15 PM | 2 | 6 | 0 | 0 | 8 | 0 | 1 | 1 | 0 | 2 | 1 | 5 | 0 | 0 | 6 | 0 | 2 | 1 | 0 | 3 | 19 |
| 05:30 PM | 0 | 4 | 2 | 0 | 6 | 2 | 6 | 0 | 0 | 8 | 0 | 5 | 0 | 5 | 10 | 0 | 2 | 0 | 2 | 4 | 28 |
| Grand Total | 7 | 49 | 10 | 1 | 67 | 7 | 17 | 14 | 0 | 38 | 19 | 49 | 1 | 8 | 77 | 1 | 30 | 4 | 7 | 42 | 224 |
| Apprch \% | 10.4 | 73.1 | 14.9 | 1.5 |  | 18.4 | 44.7 | 36.8 | 0 |  | 24.7 | 63.6 | 1.3 | 10.4 |  | 2.4 | 71.4 | 9.5 | 16.7 |  |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

34th Ave \& France Ave
Turning Movement
Robbinsdale, MN

File Name : 34th ave \& france ave
Site Code : 00000005
Start Date : 12/2/2010
Page No : 1

Groups Printed- Unshifted

|  | France Ave From North |  |  |  |  | 34th Ave From East |  |  |  |  | France Ave From South |  |  |  |  | 34th Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 07:15 AM | 3 | 6 | 43 | 1 | 53 | 27 | 1 | 2 | 0 | 30 | 1 | 10 | 0 | 0 | 11 | 1 | 2 | 3 | 0 | 6 | 100 |
| 07:30 AM | 1 | 6 | 43 | 5 | 55 | 47 | 0 | 1 | 2 | 50 | 3 | 11 | 0 | 2 | 16 | 0 | 6 | 5 | 3 | 14 | 135 |
| 07:45 AM | 1 | 6 | 60 | 0 | 67 | 41 | 0 | 1 | 1 | 43 | 3 | 16 | 0 | 1 | 20 | 0 | 6 | 2 | 0 | 8 | 138 |
| Total | 5 | 18 | 146 | 6 | 175 | 115 | 1 | 4 | 3 | 123 | 7 | 37 | 0 | 3 | 47 | 1 | 14 | 10 | 3 | 28 | 373 |
| 08:00 AM | 0 | 5 | 45 | 1 | 51 | 36 | 0 | 3 | 2 | 41 | 0 | 14 | 0 | 1 | 15 | 0 | 4 | 1 | 0 | 5 | 112 |
| 08:15 AM | 1 | 12 | 33 | 0 | 46 | 22 | 2 | 0 | 0 | 24 | 3 | 9 | 0 | 0 | 12 | 0 | 2 | 1 | 0 | 3 | 85 |
| 08:30 AM | 0 | 7 | 41 | 0 | 48 | 27 | 0 | 2 | 0 | 29 | 1 | 11 | 0 | 0 | 12 | 0 | 3 | 1 | 0 | 4 | 93 |
| Total | 1 | 24 | 119 | 1 | 145 | 85 | 2 | 5 | 2 | 94 | 4 | 34 | 0 | 1 | 39 | 0 | 9 | 3 | 0 | 12 | 290 |


| 04:15 PM | 3 | 19 | 44 | 0 | 66 | 60 | 3 | 1 | 2 | 66 | 0 | 18 | 0 | 0 | 18 | 0 | 2 | 3 | 0 | 5 | 155 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 3 | 7 | 37 | 1 | 48 | 68 | 2 | 2 | 5 | 77 | 3 | 18 | 0 | 4 | 25 | 0 | 2 | 4 | 2 | 8 | 158 |
| 04:45 PM | 4 | 23 | 41 | 3 | 71 | 60 | 2 | 3 | 4 | 69 | 0 | 12 | 0 | 0 | 12 | 1 | 0 | 1 | 3 | 5 | 157 |
| Total | 10 | 49 | 122 | 4 | 185 | 188 | 7 | 6 | 11 | 212 | 3 | 48 | 0 | 4 | 55 | 1 | 4 | 8 | 5 | 18 | 470 |
| 05:00 PM | 1 | 14 | 47 | 0 | 62 | 61 | 4 | 0 | 2 | 67 | 0 | 10 | 0 | 2 | 12 | 1 | 1 | 1 | 0 | 3 | 144 |
| 05:15 PM | 4 | 11 | 47 | 0 | 62 | 64 | 4 | 0 | 2 | 70 | 2 | 15 | 0 | 0 | 17 | 0 | 3 | 0 | 0 | 3 | 152 |
| 05:30 PM | 2 | 14 | 39 | 1 | 56 | 60 | 2 | 2 | 2 | 66 | 3 | 13 | 0 | 2 | 18 | 0 | 2 | 4 | 1 | 7 | 147 |
| Grand Total | 23 | 130 | 520 | 12 | 685 | 573 | 20 | 17 | 22 | 632 | 19 | 157 | 0 | 12 | 188 | 3 | 33 | 26 | 9 | 71 | 1576 |
| Apprch \% | 3.4 | 19 | 75.9 | 1.8 |  | 90.7 | 3.2 | 2.7 | 3.5 |  | 10.1 | 83.5 | 0 | 6.4 |  | 4.2 | 46.5 | 36.6 | 12.7 |  |  |
| Total \% | 1.5 | 8.2 | 33 | 0.8 | 43.5 | 36.4 | 1.3 | 1.1 | 1.4 | 40.1 | 1.2 | 10 | 0 | 0.8 | 11.9 | 0.2 | 2.1 | 1.6 | 0.6 | 4.5 |  |

# Hennepin Caunty 

Public Works
Transportation Department
1600 Prairie Drive
Medina, Minnesota 55340-5421
File Name: 8033700
Site Code : 08033700
Start Date : 6/30/2009
Page No : 1

Groups Printed- Unshifted - Bank 1-Bank 2

|  | CSAH 81 From North |  |  |  |  | 36th Ave. N. From East |  |  |  |  | CSAH 81 From South |  |  |  |  | 36th Ave. N. From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App, Tolal | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 06:00 AM | 22 | 63 | 2 | 0 | 87 | 5 | 16 | 1 | 0 | 22 | 0 | 28 | 4 | 0 | 32 | 17 | 6 | 2 | 0 | 25 | 166 |
| 06:15 AM | 29 | 96 | 1 | 0 | 126 | 7 | 10 | 1 | 0 | 18 | 0 | 16 | 3 | 0 | 19 | 21 | 0 | 7 | 0 | 28 | 191 |
| 06:30 AM | 24 | 133 | 0 | 0 | 157 | 5 | 31 | 1 | 0 | 37 | 0 | 36 | 6 | 0 | 42 | 23 | 7 | 20 | 0 | 50 | 286 |
| 06:45 AM | 13 | 153 | 1 | 0 | 167 | 4 | 16 | 0 | 1 | 21 | 0 | 32 | 10 | 0 | 42 | 25 | 4 | 6 | 0 | 35 | 265 |
| Total | 88 | 445 | 4 | 0 | 537 | 21 | 73 | 3 | 1 | 98 | 0 | 112 | 23 | 0 | 135 | 86 | 17 | 35 | 0 | 138 | 908 |
| 07:00 AM | 28 | 86 | 2 | 0 | 116 | 5 | 27 | 0 | 0 | 32 | 0 | 45 | 5 | 0 | 50 | 31 | 9 | 5 | 0 | 45 | 243 |
| 07:15 AM | 30 | 128 | 2 | 0 | 160 | 4 | 21 | 0 | 1 | 26 | 0 | 38 | 8 | 0 | 46 | 27 | 10 | 9 | 0 | 46 | 278 |
| 07:30 AM | 52 | 147 | 2 | 0 | 201 | 8 | 37 | 1 | 0 | 46 | 0 | 73 | 14 | 0 | 87 | 25 | 7 | 12 | 0 | 44 | 378 |
| 07:45 AM | 30 | 145 | 2 | 0 | 177 | 8 | 28 | 2 | 0 | 38 | 0 | 68 | 11 | 0 | 79 | 34 | 7 | 12 | 0 | 53 | 347 |
| Total | 140 | 506 | 8 | 0 | 654 | 25 | 113 | 3 | 1 | 142 | 0 | 224 | 38 | 0 | 262 | 117 | 33 | 38 | 0 | 188 | 1246 |
| 08:00 AM | 36 | 139 | 4 | 0 | 179 | 3 | 19 | 0 | 0 | 22 | 1 | 62 | 13 | 0 | 76 | 34 | 10 | 17 | 0 | 61 | 338 |
| 08:15 AM | 35 | 114 | 2 | 1 | 152 | 3 | 28 | 1 | 0 | 32 | 0 | 50 | 7 | 1 | 58 | 20 | 6 | 15 | 0 | 41 | 283 |
| 08:30 AM | 16 | 102 | 2 | 0 | 120 | 4 | 23 | 0 | 0 | 27 | 2 | 58 | 17 | 0 | 77 | 18 | 10 | 19 | 0 | 47 | 271 |
| 08:45 AM | 27 | 109 | 3 | 0 | 139 | 6 | 27 | 1 | 0 | 34 | 2 | 52 | 12 | 0 | 66 | 26 | 10 | 13 | 0 | 49 | 288 |
| Total | 114 | 464 | 11 | 1 | 590 | 16 | 97 | 2 | 0 | 115 | 5 | 222 | 49 | 1 | 277 | 98 | 36 | 64 | 0 | 198 | 1180 |

*** BREAK ***

| 11:00 AM | 16 | 85 | 9 | 1 | 111 | 7 | 26 | 3 | 1 | 37 | 2 | 94 | 29 | 1 | 126 | 26 | 18 | 18 | 0 | 62 | 336 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 11:15 AM | 16 | 88 | 3 | 0 | 107 | 11 | 20 | 2 | 0 | 33 | 2 | 94 | 22 | 0 | 118 | 20 | 17 | 28 | 0 | 65 | 323 |
| 11:30 AM | 24 | 83 | 6 | 1 | 114 | 7 | 19 | 2 | 1 | 29 | 1 | 93 | 21 | 1 | 116 | 21 | 14 | 23 | 1 | 59 | 318 |
| 11:45 AM | 19 | 81 | 6 | 0 | 106 | 14 | 23 | 1 | 0 | 38 | 2 | 107 | 23 | 0 | 132 | 17 | 25 | 27 | 0 | 69 | 345 |
| Total | 75 | 337 | 24 | 2 | 438 | 39 | 88 | 8 | 2 | 137 | 7 | 388 | 95 | 2 | 492 | 84 | 74 | 96 | 1 | 255 | 1322 |
| 12:00 PM | 27 | 81 | 2 | 0 | 110 | 7 | 15 | 0 | 0 | 22 | 1 | 93 | 22 | 0 | 116 | 21 | 19 | 38 | 0 | 78 | 326 |
| 12:15 PM | 27 | 89 | 5 | 0 | 121 | 10 | 19 | 0 | 0 | 29 | 2 | 83 | 27 | 0 | 112 | 28 | 21 | 37 | 0 | 86 | 348 |
| 12:30 PM | 25 | 89 | 8 | 0 | 122 | 13 | 19 | 1 | 0 | 33 | 2 | 107 | 20 | 0 | 129 | 32 | 25 | 34 | 0 | 91 | 375 |
| 12:45 PM | 38 | 118 | 7 | 0 | 163 | 2 | 14 | 1 | 0 | 17 | 0 | 88 | 12 | 0 | 100 | 26 | 23 | 24 | 0 | 73 | 353 |
| Total | 117 | 377 | 22 | 0 | 516 | 32 | 67 | 2 | 0 | 101 | 5 | 371 | 81 | 0 | 457 | 107 | 88 | 133 | 0 | 328 | 1402 |

*** BREAK ***

| 03:00 PM | 29 | 89 | 6 | 0 | 124 | 7 | 22 | 3 | 1 | 33 | 1 | 147 | 24 | 0 | 172 | 29 | 34 | 50 | 0 | 113 | 442 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 03:15 PM | 27 | 92 | 10 | 0 | 129 | 8 | 13 | 1 | 0 | 22 | 2 | 149 | 38 | 1 | 190 | 34 | 27 | 42 | 0 | 103 | 444 |
| 03:30 PM | 27 | 102 | 11 | 0 | 140 | 6 | 23 | 0 | 0 | 29 | 0 | 177 | 34 | 0 | 211 | 35 | 33 | 49 | 1 | 118 | 498 |
| 03:45 PM | 28 | 105 | 11 | 0 | 144 | 9 | 17 | 0 | 0 | 26 | 0 | 152 | 34 | 0 | 186 | 23 | 29 | 42 | 0 | 94 | 450 |
| Total | 111 | 388 | 38 | 0 | 537 | 30 | 75 | 4 | 1 | 110 | 3 | 625 | 130 | 1 | 759 | 121 | 123 | 183 | 1 | 428 | 1834 |
| 04:00 PM | 27 | 101 | 9 | 0 | 137 | 7 | 12 | 0 | 0 | 19 | 2 | 156 | 39 | 5 | 202 | 43 | 31 | 48 | 2 | 124 | 482 |
| 04:15 PM | 30 | 91 | 11 | 0 | 132 | 10 | 21 | 2 | 2 | 35 | 2 | 151 | 39 | 0 | 192 | 36 | 30 | 65 | 3 | 134 | 493 |
| 04:30 PM | 26 | 99 | 12 | 0 | 137 | 9 | 20 | 2 | 0 | 31 | 4 | 172 | 38 | 0 | 214 | 21 | 48 | 46 | 0 | 115 | 497 |
| 04:45 PM | 26 | 92 | 17 | 0 | 135. | 14 | 28 | 0 | 0 | 42 | 3 | 165 | 46 | 0 | 214 | 35 | 34 | 45 | 0 | 114 | 505 |
| Total | 109 | 383 | 49 | 0 | 541 | 40 | 81 | 4 | 2 | 127 | 11 | 644 | 162 | 5 | 822 | 135 | 143 | 204 | 5 | 487 | 1977 |
| 05:00 PM | 25 | 98 | 10 | 1 | 134 | 12 | 37 | 1 | 1 | 51 | 1 | 172 | 56 | 0 | 229 | 32 | 52 | 61 | 0 | 145 | 559 |
| 05:15 PM | 24 | 96 | 14 | 0 | 134 | 12 | 25 | 0 | 1 | 38 | 7 | 161 | 41 | 1 | 210 | 34 | 41 | 56 | 1 | 132 | 514 |
| 05:30 PM | 27 | 77 | 20 | 0 | 124 |  | 21 | 0 | 5 | 34 | 1 | 158 | 63 | 0 | 222 | 27 | 49 | 41 | 0 | 117 | 497 |
| 05:45 PM | 20 | 99 | 14 | 0 | 133 | 6 | 29 | 1 | 3 | 39 | 2 | 103 | 32 | 0 | 137 | 32 | 53 | 43 | 2 | 130 | 439 |
| Total | 96 | 370 | 58 | 1 | 525 | 38 | 112 | 2 | 10 | 162 | 11 | 594 | 192 | 1 | 798 | 125 | 195 | 201 | 3 | 524 | 2009 |
| Grand Total | 850 | 3270 | 214 | 4 | 4338 | 241 | 706 | 28 | 17 | 992 | 42 | 3180 | 770 | 10 | 4002 | 873 | 709 | 954 | 10 | 2546 | 11878 |
| Apprch \% | 19.6 | 75.4 | 4.9 | 0.1 |  | 24.3 | 71.2 | 2.8 | 1.7 |  | 1 | 79.5 | 19.2 | 0.2 |  | 34.3 | 27.8 | 37.5 | 0.4 |  |  |
| Total \% | 7.2 | 27.5 | 1.8 | 0 | 36.5 | 2 | 5.9 | 0.2 | 0.1 | 8.4 | 0.4 | 26.8 | 6.5 | 0.1 | 33.7 | 7.3 | 6 | 8 | 0.1 | 21.4 |  |
| Unshifted | 830 | 3172 | 210 | 4 | 4216 | 237 | 696 | 23 | 17 | 973 | 38 | 3087 | 747 | 10 | 3882 | 835 | 695 | 928 | 10 | 2468 | 11539 |
| \% Unshitted | 97.6 | 97 | 98.1 | 100 | 97.2 | 98.3 | 98.6 | 82.1 | 100 | 98.1 | 90.5 | 97.1 | 97 | 100 | 97 | 95.6 | 98 | 97.3 | 100 | 96.9 | 97.1 |
| Bank 1 | 20 | 88 | 4 | 0 | 112 | 4 | 9 | 5 | 0 | 18 | 4 | 87 | 20 | 0 | 111 | 34 | 14 | 22 | 0 | 70 | 311 |
| \% Bank 1 | 2.4 | 2.7 | 1.9 | 0 | 2.6 | 1.7 | 1.3 | 17.9 | 0 | 1.8 | 9.5 | 2.7 | 2.6 | 0 | 2.8 | 3.9 | 2 | 2.3 | 0 | 2.7 | 2.6 |

## Hennepin Caunty

Public Works
Transportation Department
1600 Prairie Drive
Medina, Minnesota 55340-5421
File Name: 8033700
Site Code : 08033700
Start Date : 6/30/2009
Page No : 2

|  | CSAH 81 From North |  |  |  |  | 36th Ave. N. From East |  |  |  |  | CSAH 81 From South |  |  |  |  | 36th Ave. N. From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Toxa | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| Bank 2 | 0 | 10 | 0 | 0 | 10 | 0 | 1 | 0 | 0 | 1 | 0 | 6 | 3 | 0 | 9 | 4 | 0 | 4 | 0 | 8 | 28 |
| \% Bank 2 | 0 | 0.3 | 0 | 0 | 0.2 | 0 | 0.1 | 0 | 0 | 0.1 | 0 | 0.2 | 0.4 | 0 | 0.2 | 0.5 | 0 | 0.4 | 0 | 0.3 | 0.2 |

## Hennepin Caunty

Public Works
Transportation Department
1600 Prairie Drive
Medina, Minnesota 55340-5421
File Name : 8004200-1
Site Code : 08004200
Start Date : 6/25/2009 Page No : 1

|  | CSAH 81 <br> From North |  |  |  |  | Abbott Ave. From East |  |  |  |  | $\text { CSAH } 81$ <br> From South |  |  |  |  | Abbott Ave. From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Tolad | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Tolad | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 06:00 AM | 54 | 43 | 0 | 0 | 97 | 0 | 0 | 0 | 0 | 0 | 0 | 29 | 5 | 0 | 34 | 1 | 0 | 4 | 0 | 5 | 136 |
| 06:15 AM | 60 | 55 | 0 | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 3 | 0 | 26 | 2 | 0 | 9 | 0 | 11 | 152 |
| 06:30 AM | 89 | 46 | 0 | 0 | 135 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 5 | 1 | 30 | 2 | 0 | 7 | 0 | 9 | 174 |
| 06:45 AM | 99 | 77 | 0 | 0 | 176 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 1 | 0 | 21 | 2 | 0 | 9 | 0 | 11 | 208 |
| Total | 302 | 221 | 0 | 0 | 523 | 0 | 0 | 0 | 0 | 0 | 0 | 96 | 14 | 1 | 111 | 7 | 0 | 29 | 0 | 36 | 670 |
| 07:00 AM | 76 | 86 | 0 | 0 | 162 | 0 | 0 | 0 | 0 | 0 | 0 | 25 | 9 | 0 | 34 | 4 | 0 | 16 | 0 | 20 | 216 |
| 07:15 AM | 61 | 86 | 0 | 0 | 147 | 0 | 0 | 0 | 0 | 0 | 0 | 34 | 13 | 0 | 47 | 7 | 0 | 23 | 0 | 30 | 224 |
| 07:30 AM | 69 | 125 | 0 | 5 | 199 | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 8 | 0 | 52 | 2 | 0 | 27 | 0 | 29 | 280 |
| 07:45 AM | 81 | 121 | 0 | 7 | 209 | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 16 | 0 | 60 | 5 | 0 | 40 | 0 | 45 | 314 |
| Total | 287 | 418 | 0 | 12 | 717 | 0 | 0 | 0 | 0 | 0 | 0 | 147 | 46 | 0 | 193 | 18 | 0 | 106 | 0 | 124 | 1034 |
| 08:00 AM | 60 | 134 | 0 | 1 | 195 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 13 | 0 | 61 | 5 | 0 | 28 | 0 | 33 | 289 |
| 08:15 AM | 54 | 113 | 0 | 0 | 167 | 0 | 0 | 0 | 0 | 0 | 0 | 42 | 12 | 0 | 54 | 6 | 0 | 19 | 0 | 25 | 246 |
| 08:30 AM | 42 | 99 | 0 | 4 | 145 | 0 | 0 | 0 | 0 | 0 | 0 | 51 | 7 | 0 | 58 | 7 | 0 | 13 | 0 | 20 | 223 |
| 08:45 AM | 53 | 98 | 0 | 2 | 153 | 0 | 0 | 0 | 0 | 0 | 0 | 65 | 8 | 0 | 73 | 4 | 0 | 22 | 0 | 26 | 252 |
| Total | 209 | 444 | 0 | 7 | 660 | 0 | 0 | 0 | 0 | 0 | 0 | 206 | 40 | 0 | 246 | 22 | 0 | 82 | 0 | 104 | 1010 |

*** BREAK ***

| $11: 00 \mathrm{AM}$ | 30 | 78 | 0 | 1 | 109 | 0 | 0 | 0 | 0 | 0 | 0 | 62 | 8 | 0 | 70 | 8 | 0 | 37 | 0 | 45 | 224 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 11:15 AM | 26 | 72 | 0 | 3 | 101 | 0 | 0 | 0 | 0 | 0 | 0 | 61 | 3 | 0 | 64 | 13 | 0 | 43 | 0 | 56 | 221 |
| $11: 30 \mathrm{AM}$ | 36 | 80 | 0 | 4 | 120 | 0 | 0 | 0 | 0 | 0 | 0 | 81 | 6 | 0 | 87 | 7 | 0 | 48 | 0 | 55 | 262 |
| $11: 45 \mathrm{AM}$ | 36 | 78 | 0 | 0 | 114 | 0 | 0 | 0 | 0 | 0 | 0 | 71 | 13 | 0 | 84 | 7 | 0 | 37 | 0 | 44 | 242 |
| Total | 128 | 308 | 0 | 8 | 444 | 0 | 0 | 0 | 0 | 0 | 0 | 275 | 30 | 0 | 305 | 35 | 0 | 165 | 0 | 200 | 949 |


| 12:00 PM | 38 | 79 | 0 | 2 | 119 | 0 | 0 | 0 | 1 | 1 | 0 | 93 | 7 | 0 | 100 | 6 | 0 | 52 | 0 | 58 | 278 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:15 PM | 35 | 95 | 0 | 0 | 130 | 0 | 0 | 0 | 2 | 2 | 0 | 95 | 8 | 0 | 103 | 7 | 0 | 34 | 1 | 42 | 277 |
| 12:30 PM | 47 | 90 | 0 | 2 | 139 | 0 | 0 | 0 | 0 | 0 | 0 | 79 | 10 | 0 | 89 | 9 | 0 | 38 | 0 | 47 | 275 |
| 12:45 PM | 37 | 99 | 0 | 1 | 137 | 0 | 0 | 0 | 1 | 1 | 0 | 88 | 7 | 0 | 95 | 12 | 0 | 30 | 0 | 42 | 275 |
| Total | 157 | 363 | 0 | 5 | 525 | 0 | 0 | 0 | 4 | 4 | 0 | 355 | 32 | 0 | 387 | 34 | 0 | 154 | 1 | 189 | 1105 |

## *** BREAK ***

| 03:00 PM | 45 | 74 | 0 | 1 | 120 | 0 | 0 | 0 | 1 | 1 | 0 | 98 | 14 | 0 | 112 | 14 | 0 | 85 | 0 | 99 | 332 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $03: 15 \mathrm{PM}$ | 48 | 92 | 0 | 4 | 144 | 0 | 0 | 0 | 0 | 0 | 0 | 104 | 12 | 0 | 116 | 13 | 0 | 65 | 0 | 78 | 338 |
| $03: 30 \mathrm{PM}$ | 28 | 139 | 0 | 1 | 168 | 0 | 0 | 0 | 0 | 0 | 0 | 99 | 6 | 0 | 105 | 13 | 0 | 105 | 0 | 118 | 391 |
| $03: 45 \mathrm{PM}$ | 35 | 119 | 0 | 0 | 154 | 0 | 0 | 0 | 0 | 0 | 0 | 113 | 12 | 0 | 125 | 11 | 0 | 55 | 0 | 66 | 345 |
| Total | 156 | 424 | 0 | 6 | 586 | 0 | 0 | 0 | 1 | 1 | 0 | 414 | 44 | 0 | 458 | 51 | 0 | 310 | 0 | 361 | 1406 |


| 04:00 PM | 28 | 98 | 0 | 1 | 127 | 0 | 0 | 0 | 0 | 0 | 0 | 121 | 14 | 0 | 135 | 7 | 2 | 71 | 0 | 80 | 342 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 30 | 104 | 0 | 4 | 138 | 0 | 0 | 0 | 0 | 0 | 0 | 144 | 8 | 0 | 152 | 7 | 0 | 62 | 0 | 69 | 359 |
| 04:30 PM | 18 | 118 | 0 | 7 | 143 | 0 | 0 | 0 | 0 | 0 | 0 | 148 | 7 | 0 | 155 | 13 | 0 | 79 | 0 | 92 | 390 |
| 04:45 PM | 24 | 111 | 0 | 2 | 137 | 0 | 0 | 0 | 0 | 0 | 0 | 141 | 14 | 0 | 155 | 13 | 0 | 55 | 0 | 68 | 360 |
| Total | 100 | 431 | 0 | 14 | 545 | 0 | 0 | 0 | 0 | 0 | 0 | 554 | 43 | 0 | 597 | 40 | 2 | 267 | 0 | 309 | 1451 |


| 05:00 PM | 23 | 113 | 0 | 0 | 136 | 0 | 0 | 0 | 0 | 0 | 0 | 135 | 9 | 0 | 144 | 14 | 0 | 81 | 0 | 95 | 375 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 05:15 PM | 23 | 126 | 0 | 0 | 149 | 0 | 0 | 0 | 1 | 1 | 0 | 139 | 12 | 0 | 151 | 11 | 0 | 52 | 0 | 63 | 364 |
| 05:30 PM | 20 | 124 | 0 | 2 | 146 | 0 | 0 | 0 | 0 | 0 | 0 | 129 | 14 | 0 | 143 | 12 | 0 | 47 | 0 | 59 | 348 |
| 05:45 PM | 25 | 115 | 0 | 2 | 142 | 0 | 0 | 0 | 2 | 2 | 0 | 109 | 11 | 0 | 120 | 9 | 0 | 22 | 0 | 31 | 295 |
| Total | 91 | 478 | 0 | 4 | 573 | 0 | 0 | 0 | 3 | 3 | 0 | 512 | 46 | 0 | 558 | 46 | 0 | 202 | 0 | 248 | 1382 |
| Grand Total | 1430 | 3087 | 0 | 56 | 4573 | 0 | 0 | 0 | 8 | 8 | 0 | 2559 | 295 | 1 | 2855 | 253 | 2 | 1315 | 1 | 1571 | 9007 |
| Apprch \% | 31.3 | 67.5 | 0 | 1.2 |  | 0 | 0 | 0 | 100 |  | 0 | 89.6 | 10.3 | 0 |  | 16.1 | 0.1 | 83.7 | 0.1 |  |  |
| Total \% | 15.9 | 34.3 | 0 | 0.6 | 50.8 | 0 | 0 | 0 | 0.1 | 0.1 | 0 | 28.4 | 3.3 | 0 | 31.7 | 2.8 | 0 | 14.6 | 0 | 17.4 |  |
| Unshifted | 1382 | 2968 | 0 | 56 | 4406 | 0 | 0 | 0 | 8 | 8 | 0 | 2478 | 283 | 1 | 2762 | 250 | 2 | 1298 | 1 | 1551 | 8727 |
| \% Unshifted | 96.6 | 96.1 | 0 | 100 | 96.3 | 0 | 0 | 0 | 100 | 100 | 0 | 96.8 | 95.9 | 100 | 96.7 | 98.8 | 100 | 98.7 | 100 | 98.7 | 96.9 |
| Bank 1 | 47 | 107 | 0 | 0 | 154 | 0 | 0 | 0 | 0 | 0 | 0 | 70 | 12 | 0 | 82 | 3 | 0 | 16 | 0 | 19 | 255 |
| \% Bank 1 | 3.3 | 3.5 | 0 | 0 | 3.4 | 0 | 0 | 0 | 0 | 0 | 0 | 2.7 | 4.1 | 0 | 2.9 | 1.2 | 0 | 1.2 | 0 | 1.2 | 2.8 |

## Hennepin Caunty

Public Works
Transportation Department 1600 Prairie Drive
Medina, Minnesota 55340-5421

File Name : 8004200-1
Site Code : 08004200
Start Date : 6/25/2009
Page No : 2

Groups Printed- Unshifted - Bank 1 - Bank 2

|  | CSAH 81 <br> From Noth |  |  |  |  | Abbott Ave. From East |  |  |  |  | CSAH 81 From South |  |  |  |  | Abbott Ave. From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Right | Thru | Left | Peds | App. Toxal | Right | Thru | Left | Peds | App. Tolal | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Tolad | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| Bank 2 | 1 | 12 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 1 | 0 | 1 | 25 |
| \% Bank 2 | 0.1 | 0.4 | 0 | 0 | 0.3 | 0 | 0 | 0 | 0 | 0 | 0 | 0.4 | 0 | 0 | 0.4 | 0 | 0 | 0.1 | 0 | 0.1 | 0.3 |

Appendix D
Tube Counts
(Taken by WSB)

|  |  | Halifax Ave <br> (South of 36th Ave) | Grimes Ave <br> (South of 36th Ave) | Terrace Mall Driveway To France Ave (Between 35th and 36th Aves) | Terrace Mall Driveway To France Ave (At 35th Ave) | W Broadway Ave (North of Abbott Ave) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM MAX |  | 55 | 47 | 55 | 192 | 112 |
| PM MAX |  | 80 | 70 | 58 | 157 | 146 |
| 00:00 | TO 01:00 | 7 | 10 | 0 | 5 | 5 |
| 00:15 | TO 01:15 | 6 | 10 | 0 | 5 | 5 |
| 00:30 | TO 01:30 | 6 | 9 | 0 | 4 | 6 |
| 00:45 | TO 01:45 | 8 | 9 | 0 | 3 | 5 |
| 01:00 | TO 02:00 | 8 | 8 | 0 | 2 | 5 |
| 01:15 | TO 02:15 | 8 | 7 | 0 | 2 | 5 |
| 01:30 | TO 02:30 | 7 | 5 | 0 | 2 | 4 |
| 01:45 | TO 02:45 | 6 | 4 | 0 | 2 | 4 |
| 02:00 | TO 03:00 | 6 | 3 | 0 | 1 | 4 |
| 02:15 | TO 03:15 | 6 | 2 | 0 | 1 | 3 |
| 02:30 | TO 03:30 | 6 | 3 | 0 | 2 | 3 |
| 02:45 | TO 03:45 | 5 | 4 | 0 | 1 | 3 |
| 03:00 | TO 04:00 | 5 | 5 | 1 | 1 | 2 |
| 03:15 | TO 04:15 | 5 | 5 | 1 | 3 | 2 |
| 03:30 | TO 04:30 | 6 | 6 | 1 | 6 | 3 |
| 03:45 | TO 04:45 | 6 | 5 | 1 | 8 | 7 |
| 04:00 | TO 05:00 | 7 | 4 | 0 | 8 | 11 |
| 04:15 | TO 05:15 | 6 | 4 | 0 | 12 | 15 |
| 04:30 | TO 05:30 | 6 | 3 | 0 | 12 | 22 |
| 04:45 | TO 05:45 | 9 | 4 | 0 | 19 | 25 |
| 05:00 | TO 06:00 | 14 | 5 | 0 | 26 | 31 |
| 05:15 | TO 06:15 | 15 | 6 | 0 | 50 | 33 |
| 05:30 | TO 06:30 | 23 | 8 | 1 | 67 | 34 |
| 05:45 | TO 06:45 | 31 | 8 | 4 | 82 | 42 |
| 06:00 | TO 07:00 | 39 | 12 | 5 | 99 | 48 |
| 06:15 | TO 07:15 | 46 | 14 | 9 | 102 | 58 |
| 06:30 | TO 07:30 | 44 | 16 | 13 | 110 | 68 |
| 06:45 | TO 07:45 | 44 | 23 | 14 | 129 | 74 |
| 07:00 | TO 08:00 | 48 | 25 | 18 | 163 | 81 |
| 07:15 | TO 08:15 | 54 | 28 | 25 | 192 | 84 |
| 07:30 | TO 08:30 | 55 | 30 | 29 | 190 | 85 |
| 07:45 | TO 08:45 | 52 | 29 | 30 | 172 | 89 |
| 08:00 | TO 09:00 | 45 | 32 | 30 | 144 | 82 |
| 08:15 | TO 09:15 | 39 | 33 | 29 | 100 | 81 |
| 08:30 | TO 09:30 | 37 | 35 | 38 | 95 | 83 |
| 08:45 | TO 09:45 | 36 | 36 | 42 | 100 | 81 |
| 09:00 | TO 10:00 | 33 | 38 | 45 | 101 | 87 |
| 09:15 | TO 10:15 | 33 | 36 | 49 | 102 | 94 |
| 09:30 | TO 10:30 | 31 | 37 | 40 | 104 | 90 |
| 09:45 | TO 10:45 | 32 | 39 | 44 | 101 | 93 |
| 10:00 | TO 11:00 | 31 | 41 | 48 | 88 | 95 |
| 10:15 | TO 11:15 | 28 | 45 | 46 | 88 | 93 |
| 10:30 | TO 11:30 | 35 | 47 | 50 | 99 | 102 |
| 10:45 | TO 11:45 | 36 | 47 | 49 | 103 | 102 |
| 11:00 | TO 12:00 | 38 | 43 | 55 | 122 | 107 |
| 11:15 | TO 12:15 | 41 | 42 | 53 | 130 | 112 |
| 11:30 | TO 12:30 | 37 | 43 | 55 | 120 | 113 |
| 11:45 | TO 12:45 | 36 | 43 | 58 | 117 | 119 |
| 12:00 | TO 13:00 | 38 | 42 | 46 | 103 | 120 |
| 12:15 | TO 13:15 | 37 | 48 | 48 | 99 | 123 |
| 12:30 | TO 13:30 | 44 | 47 | 52 | 88 | 122 |
| 12:45 | TO 13:45 | 42 | 51 | 46 | 82 | 116 |
| 13:00 | TO 14:00 | 41 | 54 | 52 | 69 | 115 |
| 13:15 | TO 14:15 | 47 | 59 | 54 | 64 | 114 |
| 13:30 | TO 14:30 | 44 | 59 | 42 | 74 | 115 |
| 13:45 | TO 14:45 | 50 | 55 | 43 | 115 | 122 |
| 14:00 | TO 15:00 | 59 | 56 | 41 | 125 | 127 |
| 14:15 | TO 15:15 | 59 | 47 | 34 | 143 | 131 |
| 14:30 | TO 15:30 | 62 | 46 | 43 | 143 | 132 |
| 14:45 | TO 15:45 | 66 | 47 | 42 | 122 | 125 |
| 15:00 | TO 16:00 | 63 | 46 | 43 | 145 | 126 |
| 15:15 | TO 16:15 | 62 | 47 | 40 | 141 | 131 |
| 15:30 | TO 16:30 | 66 | 50 | 27 | 157 | 134 |
| 15:45 | TO 16:45 | 63 | 53 | 25 | 147 | 146 |
| 16:00 | TO 17:00 | 59 | 55 | 18 | 128 | 146 |
| 16:15 | TO 17:15 | 65 | 58 | 22 | 139 | 144 |
| 16:30 | TO 17:30 | 60 | 56 | 25 | 122 | 143 |
| 16:45 | TO 17:45 | 70 | 53 | 26 | 106 | 131 |


| 17:00 | TO 18:00 | 78 | 54 | 30 | 95 | 119 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:15 | TO 18:15 | 80 | 54 | 25 | 65 | 108 |
| 17:30 | TO 18:30 | 78 | 57 | 23 | 52 | 103 |
| 17:45 | TO 18:45 | 68 | 66 | 18 | 42 | 99 |
| 18:00 | TO 19:00 | 63 | 70 | 16 | 39 | 94 |
| 18:15 | TO 19:15 | 57 | 66 | 15 | 32 | 87 |
| 18:30 | TO 19:30 | 61 | 62 | 15 | 26 | 80 |
| 18:45 | TO 19:45 | 61 | 55 | 13 | 23 | 73 |
| 19:00 | TO 20:00 | 55 | 49 | 8 | 24 | 68 |
| 19:15 | TO 20:15 | 49 | 49 | 9 | 28 | 63 |
| 19:30 | TO 20:30 | 42 | 49 | 11 | 30 | 55 |
| 19:45 | TO 20:45 | 35 | 41 | 12 | 31 | 50 |
| 20:00 | TO 21:00 | 32 | 37 | 14 | 24 | 46 |
| 20:15 | TO 21:15 | 29 | 38 | 12 | 18 | 42 |
| 20:30 | TO 21:30 | 32 | 37 | 9 | 14 | 33 |
| 20:45 | TO 21:45 | 34 | 38 | 9 | 10 | 29 |
| 21:00 | TO 22:00 | 35 | 37 | 8 | 13 | 24 |
| 21:15 | TO 22:15 | 38 | 31 | 11 | 16 | 18 |
| 21:30 | TO 22:30 | 38 | 29 | 9 | 17 | 17 |
| 21:45 | TO 22:45 | 34 | 27 | 7 | 18 | 13 |
| 22:00 | TO 23:00 | 31 | 26 | 5 | 12 | 13 |
| 22:15 | TO 23:15 | 29 | 29 | 2 | 7 | 13 |
| 22:30 | TO 23:30 | 20 | 24 | 1 | 6 | 12 |
| 22:45 | TO 23:45 | 20 | 25 | 1 | 5 | 11 |
| 23:00 | TO 24:00 | 19 | 22 | 1 | 4 | 10 |

## Appendix E

## Detailed LOS Conditions and Queue Lengths

No Build (AM Peak Hour)


No Build (PM Peak Hour)



[^3]

[^4]| Intersection |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queuing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \overline{\mathrm{O}} \\ & \\ & 0 \\ & 0 \end{aligned}$ | Location |  |  |  |  | Left Turn | Through |  |  |  | Right Turn |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave Queue | $\begin{array}{\|c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Ave Queue | Max Queue | Ave Queue | Max Queue |
| $\stackrel{0}{5}$ | Halifax Avenue \& 36th Avenue | NB | 9 | 0 | 8 | A | A | A | 9 | A | 1 | A |  | NB |  |  |  |  |  |  |
|  |  | WB | 5 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 7 | 10 | 6 | A | B | A | 7 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 3 | 0 | 1 | A | A | A | 0 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 7 | 2 | 7 | A | A | A | 7 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 3 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 7 | 9 | 5 | A | A | A | 6 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 3 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 36th Avenue | NB | 12 | 8 | 4 | B | A | A | 10 | A | 7 | A | NB | 6 | 95 | 6 | 95 | 6 | 95 |
|  |  | WB | 10 | 6 | 4 | B | A | A | 6 | A |  |  | WB | 4 | 75 | 4 | 75 | 4 | 75 |
|  |  | SB | 10 | 8 | 4 | B | A | A | 8 | A |  |  | SB | 3 | 59 | 3 | 60 | 3 | 59 |
|  |  | EB | 7 | 7 | 5 | A | A | A | 6 | A |  |  | EB | 8 | 116 | 8 | 116 | 8 | 116 |
|  | W. Broadway Frontage Rd \& 36th Avenue | NB | 8 | 0 | 5 | A | A | A | 6 | A | 1 | A | NB |  | 3 |  | 3 |  | 3 |
|  |  | WB | 2 | 0 | 0 | A | A | A | 1 | A |  |  | WB |  | 87 |  | 87 |  | 87 |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 0 | 3 | A | A | A | 0 | A |  |  | EB |  | 6 |  | 6 |  | 6 |
|  | CSAH 81 \& 36th Avenue | NB | 30 | 17 | 3 | C | B | A | 19 | B | 17 | B | NB | 7 | 74 | 12 | 122 |  |  |
|  |  | WB | 32 | 22 | 5 | C | C | A | 20 | C |  |  | WB | 9 | 95 | 12 | 95 | 8 | 95 |
|  |  | SB | 24 | 22 | 7 | C | C | A | 18 | B |  |  | SB | 1 | 38 | 36 | 241 |  | 4 |
|  |  | EB | 24 | 15 | 6 | C | B | A | 12 | B |  |  | EB | 9 | 68 | 10 | 70 | 4 | 70 |
|  |  <br> Abbott Avenue | NB | 35 | 2 |  | D | A |  | 8 | A | 9 | A | NB | 8 | 84 | 7 | 84 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 6 | 5 |  | A | A | 6 | A |  |  | SB |  |  | 7 | 148 | 6 | 148 |
|  |  | EB | 33 |  | 7 | C |  | A | 30 | C |  |  | EB | 14 | 85 |  |  | 10 | 85 |
|  |  <br> 34th Avenue | NB | 0 | 0 | 0 | A | A | A | 0 | A | 0 | A | NB |  |  |  |  |  |  |
|  |  | WB | 3 | 4 | 2 | A | A | A | 3 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | EB |  |  |  |  |  |  |
| ․․․ | France Avenue \& 34th Avenue | NB | 17 |  | 5 | B |  | A | 14 | B | 11 | B | NB | 4 | 70 |  |  | 2 | 76 |
|  |  | WB | 13 |  | 11 | B |  | B | 11 | B |  |  | WB | 8 | 118 |  |  | 8 | 118 |
|  |  | SB | 11 |  | 8 | B |  | A | 11 | B |  |  | SB | 10 | 172 |  |  | 10 | 171 |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& Terrace Mall Access | NB |  | 0 | 0 |  | A | A | 0 | A | 0 | A | NB |  |  |  |  |  |  |
|  |  | WB | 7 |  | 5 | A |  | A | 5 | A |  |  | WB |  | 7 |  |  |  | 7 |
|  |  | SB | 1 | 0 |  | A | A |  | 0 | A |  |  | SB |  | 5 |  | 5 |  |  |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 35th Avenue | NB | 1 | 1 | 1 | A | A | A | 1 | A | 2 | A | NB |  |  |  |  |  |  |
|  |  | WB | 9 | 10 | 6 | A | B | A | 7 | A |  |  | WB |  | 50 |  | 50 |  | 50 |
|  |  | SB | 1 | 0 | 1 | A | A | A | 0 | A |  |  | SB |  | 33 |  | 33 |  | 33 |
|  |  | EB | 8 | 9 | 5 | A | A | A | 6 | A |  |  | EB |  |  |  |  |  |  |

[^5]| Intersection |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queuing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 을응 | Location |  |  |  |  | Left Turn | Through |  |  |  | Right Turn |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave Queue | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Ave Queue | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Ave Queue | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ |
|  | Halifax Avenue \& 36th Avenue | NB | 11 | 0 | 7 | B | A | A | 10 | B | 1 | A |  | NB |  |  |  |  |  |  |
|  |  | WB | 7 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 8 | 0 | 6 | A | A | A | 6 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 5 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 9 | 5 | 6 | A | A | A | 8 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 2 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 7 | 11 | 6 | A | B | A | 8 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 3 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 36th Avenue | NB | 13 | 9 | 6 | B | A | A | 11 | B | 9 | A | NB | 13 | 190 | 13 | 190 | 13 | 190 |
|  |  | WB | 13 | 8 | 5 | B | A | A | 8 | A |  |  | WB | 7 | 95 | 7 | 95 | 7 | 95 |
|  |  | SB | 11 | 8 | 5 | B | A | A | 8 | A |  |  | SB | 3 | 59 | 3 | 59 | 3 | 59 |
|  |  | EB | 10 | 8 | 6 | B | A | A | 8 | A |  |  | EB | 11 | 107 | 11 | 107 | 11 | 107 |
|  | W. Broadway Frontage Rd \& 36th Avenue | NB | 9 | 8 | 7 | A | A | A | 8 | A | 3 | A | NB |  | 33 |  | 33 |  | 33 |
|  |  | WB | 2 | 0 | 0 | A | A | A | 1 | A |  |  | WB |  | 84 |  | 84 |  | 84 |
|  |  | SB | 7 | 0 | 4 | A | A | A | 5 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 1 | 1 | 3 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 34 | 11 | 5 | C | B | A | 15 | B | 21 | C | NB | 24 | 204 | 19 | 194 |  | 17 |
|  |  | WB | 51 | 37 | 9 | D | D | A | 29 | C |  |  | WB | 11 | 74 | 15 | 74 | 10 | 74 |
|  |  | SB | 48 | 20 | 5 | D | C | A | 20 | C |  |  | SB | 13 | 101 | 24 | 126 |  |  |
|  |  | EB | 41 | 27 | 5 | D | C | A | 28 | C |  |  | EB | 41 | 249 | 42 | 251 | 31 | 251 |
|  |  <br> Abbott Avenue | NB | 42 | 4 |  | D | A |  | 7 | A | 16 | B | NB | 12 | 103 | 12 | 104 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 18 | 4 |  | B | A | 16 | B |  |  | SB |  |  | 21 | 177 | 20 | 176 |
|  |  | EB | 39 |  | 8 | D |  | A | 34 | C |  |  | EB | 37 | 154 |  |  | 34 | 154 |
|  | Halifax Avenue \& 34th Avenue | NB | 0 | 0 | 0 | A | A | A | 0 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 5 | 0 | 1 | A | A | A | 5 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 6 | 7 | 3 | A | A | A | 6 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 34th Avenue | NB | 18 |  | 8 | B |  | A | 17 | B | 11 | B | NB | 5 | 83 |  |  | 3 | 88 |
|  |  | WB | 14 |  | 10 | B |  | B | 10 | B |  |  | WB | 13 | 188 |  |  | 13 | 188 |
|  |  | SB | 10 |  | 10 | B |  | B | 10 | B |  |  | SB | 11 | 186 |  |  | 11 | 186 |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  |  <br> Terrace Mall Access | NB |  | 1 | 0 |  | A | A | 1 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 8 |  | 7 | A |  | A | 7 | A |  |  | WB |  | 28 |  |  |  | 28 |
|  |  | SB | 2 | 0 |  | A | A |  | 0 | A |  |  | SB |  | 6 |  | 6 |  |  |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  |  <br> 35th Avenue | NB | 2 | 1 | 1 | A | A | A | 1 | A | 2 | A | NB |  | 22 |  | 22 |  | 22 |
|  |  | WB | 10 | 7 | 7 | B | A | A | 9 | A |  |  | WB | 1 | 60 | 1 | 60 | 1 | 60 |
|  |  | SB | 2 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  | 36 |  | 36 |  | 36 |
|  |  | EB | 9 | 10 | 6 | A | B | A | 8 | A |  |  | EB |  | 44 |  | 44 |  | 44 |

* Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
approximately 5,475 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
Source: WSB \& Associates



## Appendix F

## Historical Traffic Counts

(Two-way tube counts provided by Hennepin County)


## Appendix G

## Sensitivity Analysis Results

(Revised Forecasts: Annual growth rate of one percent)

| Intersection |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach (Sec/Veh) |  | $\begin{aligned} & \text { LOS by } \\ & \text { Intersection } \\ & \text { (Sec/Veh) } \end{aligned}$ |  | Appr | Average \& Maximum Traffic Queuing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\bar{y}$ | Location |  |  |  |  | Left Turn | Through |  |  |  | Right Turn |  |
| 0 |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave Queue | Max Queue | Ave Queue | Max Queue | Ave Queue | Max Queue |
| $\begin{aligned} & \stackrel{\rightharpoonup}{\ddot{x}} \\ & \stackrel{\rightharpoonup}{E v} \end{aligned}$ |  <br> 36th Avenue | NB |  |  |  |  |  |  |  |  |  |  |  | NB |  |  |  |  |  |  |
|  |  | WB |  | 0 | 0 |  | A | A | 0 | A | 1 | A | WB |  |  |  |  |  |  |
|  |  | SB | 11 |  | 8 | B |  | A | 10 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 4 | 1 |  | A | A |  | 1 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB |  |  |  |  |  |  |  |  |  |  | NB |  |  |  |  |  |  |
|  |  | WB |  | 0 | 1 |  | A | A | 0 | A | 1 | A | WB |  |  |  |  |  |  |
|  |  | SB | 11 |  | 6 | B |  | A | 8 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 5 | 2 |  | A | A |  | 2 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 24 | 19 | 19 | C | B | B | 22 | C |  |  | NB | 54 | 244 | 54 | 245 | 54 | 245 |
|  |  | WB | 31 | 6 | 5 | C | A | A | 10 | B | 13 | B | WB | 11 | 112 | 11 | 113 | 11 | 112 |
|  |  | SB | 20 | 17 | 7 | C | B | A | 17 | B |  |  | SB | 10 | 97 | 9 | 94 | 10 | 96 |
|  |  | EB | 9 | 8 | 9 | A | A | A | 9 | A |  |  | EB | 18 | 233 | 19 | 232 | 20 | 233 |
|  | W. Broadway Frontage Rd \& 36th Avenue | NB |  |  |  |  |  |  |  |  |  |  | NB |  |  |  |  |  |  |
|  |  | WB |  | 0 | 0 |  | A | A | 0 | A | 0 | A | WB |  |  |  | 80 |  | 80 |
|  |  | SB | 9 |  | 0 | A |  | A | 9 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 1 | 0 |  | A | A |  | 0 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 54 | 8 | 4 | D | A | A | 14 | B | 19 | B | NB | 12 | 93 | 7 | 85 |  |  |
|  |  | WB | 42 | 33 | 7 | D | C | A | 33 | C |  |  | WB | 22 | 118 | 24 | 119 | 15 | 102 |
|  |  | SB | 47 | 18 | 6 | D | B | A | 17 | B |  |  | SB | 4 | 64 | 30 | 239 |  |  |
|  |  | EB | 41 | 31 | 10 | D | C | B | 22 | C |  |  | EB | 20 | 100 | 22 | 103 | 12 | 103 |
| $\|\mathbf{A}\|$ |  <br> Abbott Avenue | NB | 48 | 2 |  | D | A |  | 6 | A | 10 | B | NB | 7 | 65 | 7 | 65 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 4 | 9 |  | A | A | 6 | A |  |  | SB |  |  | 11 | 176 | 10 | 176 |
|  |  | EB | 38 |  | 46 | D |  | D | 38 | D |  |  | EB | 43 | 136 |  |  | 36 | 139 |
|  |  <br> 34th Avenue | NB | 0 | 0 | 4 | A | A | A | 4 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 4 | A | A | A | 4 | A |  |  | SB |  | 9 |  | 9 |  | 9 |
|  |  | EB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 34th Avenue | NB | 0 | 0 | 4 | A | A | A | 4 | A | 2 | A | NB |  | 25 |  | 25 |  | 25 |
|  |  | WB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 6 | 0 | 5 | A | A | A | 6 | A |  |  | SB |  | 9 |  | 9 |  | 9 |
|  |  | EB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 34th Avenue | NB | 0 | 10 | 7 | A | B | A | 10 | A | 11 | B | NB | 5 | 102 | 6 | 100 | 3 | 76 |
|  |  | WB | 13 | 13 | 10 | B | B | B | 10 | B |  |  | WB | 13 | 145 | 18 | 154 | 18 | 148 |
|  |  | SB | 13 | 14 | 12 | B | B | B | 13 | B |  |  | SB | 15 | 173 | 15 | 173 | 14 | 173 |
|  |  | EB | 9 | 9 | 0 | A | A | A | 9 | A |  |  | EB | 2 | 46 | 2 | 46 | 1 | 43 |
|  |  <br> Terrace Mall Access | NB |  | 1 | 0 |  | A | A | 1 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 12 |  | 6 | B |  | A | 7 | A |  |  | WB |  | 35 |  |  |  | 35 |
|  |  | SB | 3 | 1 |  | A | A |  | 1 | A |  |  | SB |  | 81 |  | 81 |  |  |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 35th Avenue | NB | 2 | 1 | 1 | A | A | A | 1 | A | 3 | A | NB |  | 19 |  | 19 |  | 19 |
|  |  | WB | 15 | 11 | 8 | C | B | A | 10 | B |  |  | WB | 2 | 75 | 2 | 75 | 2 | 75 |
|  |  | SB | 3 | 1 | 1 | A | A | A | 1 | A |  |  | SB | 1 | 84 | 1 | 84 | 1 | 84 |
|  |  | EB | 12 | 13 | 8 | B | B | A | 11 | B |  |  | EB | 1 | 46 | 1 | 46 | 1 | 46 |
|  <br> New Terrace Mall Access |  | NB | 42 | 2 |  | D | A |  | 10 | A | 9 | A | NB | 18 | 103 | 18 | 102 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 6 | 3 |  | A | A | 5 | A |  |  | SB |  |  | 18 | 227 | 16 | 227 |
|  |  | EB | 39 |  | 42 | D |  | D | 41 | D |  |  | EB | 30 | 136 |  |  | 30 | 137 |

[^6]

[^7]| Intersection |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queuing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \bar{y} \\ & \text { By } \\ & \text { O} \end{aligned}$ | Location |  |  |  |  | Left Turn | Through |  |  |  | Right Turn |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \\ \hline \end{array}$ | $\begin{gathered} \hline \text { Max } \\ \text { Queue } \end{gathered}$ | Ave Queue | Max Queue | Ave Queue | Max Queue |
|  |  <br> 36th Avenue | NB | 12 | 0 | 10 | B | A | B | 12 | B | 1 | A |  | NB |  | 33 |  | 33 |  | 33 |
|  |  | WB | 8 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 9 | 12 | 7 | A | B | A | 8 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 5 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 10 | 10 | 6 | B | B | A | 8 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 3 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 7 | 10 | 6 | A | B | A | 7 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 3 | 1 | 0 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 36th Avenue | NB | 13 | 9 | 5 | B | A | A | 11 | B | 8 | A | NB | 8 | 93 | 9 | 93 | 9 | 93 |
|  |  | WB | 14 | 7 | 5 | B | A | A | 8 | A |  |  | WB | 5 | 76 | 6 | 76 | 5 | 76 |
|  |  | SB | 12 | 9 | 5 | B | A | A | 9 | A |  |  | SB | 4 | 64 | 4 | 64 | 4 | 64 |
|  |  | EB | 9 | 8 | 6 | A | A | A | 7 | A |  |  | EB | 11 | 115 | 11 | 115 | 11 | 115 |
|  | W. Broadway Frontage Rd \& 36th Avenue | NB | 8 | 0 | 5 | A | A | A | 6 | A | 2 | A | NB |  |  |  |  |  |  |
|  |  | WB | 3 | 1 | 0 | A | A | A | 2 | A |  |  | WB |  | 102 |  | 102 |  | 102 |
|  |  | SB | 6 | 0 | 0 | A | A | A | 6 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 1 | 0 | 3 | A | A | A | 1 | A |  |  | EB |  | 18 |  | 18 |  | 18 |
|  |  <br> 36th Avenue | NB | 31 | 21 | 4 | C | C | A | 22 | C | 18 | B | NB | 8 | 84 | 20 | 191 |  |  |
|  |  | WB | 31 | 23 | 6 | C | C | A | 21 | C |  |  | WB | 11 | 87 | 14 | 86 | 11 | 87 |
|  |  | SB | 19 | 22 | 7 | B | C | A | 18 | B |  |  | SB | 1 | 68 | 43 | 278 |  |  |
|  |  | EB | 26 | 15 | 7 | C | B | A | 13 | B |  |  | EB | 13 | 86 | 14 | 88 | 6 | 89 |
|  |  <br> Abbott Avenue | NB | 34 | 2 |  | C | A |  | 8 | A | 10 | B | NB | 10 | 80 | 9 | 81 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 8 | 6 |  | A | A | 7 | A |  |  | SB |  |  | 10 | 189 | 9 | 188 |
|  |  | EB | 32 |  | 7 | C |  | A | 29 | C |  |  | EB | 17 | 101 |  |  | 13 | 102 |
|  |  <br> 34th Avenue | NB | 0 | 0 | 0 | A | A | A | 0 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 6 | 7 | 5 | A | A | A | 6 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 6 | 0 | 0 | A | A | A | 6 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 34th Avenue | NB | 29 |  | 12 | C |  | B | 24 | C | 15 | B | NB | 11 | 108 |  |  | 2 | 76 |
|  |  | WB | 21 |  | 14 | C |  | B | 14 | B |  |  | WB | 14 | 180 |  |  | 14 | 180 |
|  |  | SB | 11 |  | 14 | B |  | B | 11 | B |  |  | SB | 15 | 217 |  |  | 15 | 217 |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  |  <br> Terrace Mall Access | NB |  | 1 | 1 |  | A | A | 1 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 8 |  | 6 | A |  | A | 7 | A |  |  | WB |  | 18 |  |  |  | 18 |
|  |  | SB | 1 | 0 |  | A | A |  | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 35th Avenue | NB | 1 | 1 | 1 | A | A | A | 1 | A | 2 | A | NB |  | 32 |  | 32 |  | 32 |
|  |  | WB | 11 | 12 | 7 | B | B | A | 8 | A |  |  | WB | 1 | 66 | 1 | 66 | 1 | 66 |
|  |  | SB | 2 | 1 | 1 | A | A | A | 1 | A |  |  | SB |  | 54 |  | 54 |  | 54 |
|  |  | EB | 9 | 9 | 6 | A | A | A | 7 | A |  |  | EB |  | 14 |  | 14 |  | 14 |

* Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
approximately 5,475 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
Source: WSB \& Associates
K:0101844-05TTrafficic[LOS Results.XIS]2030AM_LRT(34h)_GROWTH

| Intersection |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queuing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 들 | Location |  |  |  |  | Left Turn | Through |  |  |  | Right Turn |  |
| $\bigcirc$ |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave Queue | Max <br> Queue | Ave Queue | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Ave Queue | Max Queue |
|  | Halifax Avenue \& 36th Avenue | NB | 14 | 0 | 10 | B | A | B | 13 | B | 1 | A |  | NB |  | 13 |  | 13 |  | 13 |
|  |  | WB | 7 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 14 | 0 | 8 | B | A | A | 9 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 7 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  | Grimes Avenue \& 36th Avenue | NB | 9 | 8 | 7 | A | A | A | 8 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 3 | 0 | 1 | A | A | A | 0 | A |  |  | WB |  | 7 |  | 7 |  | 7 |
|  |  | SB | 8 | 12 | 6 | A | B | A | 8 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 4 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |
|  |  <br> 36th Avenue | NB | 16 | 11 | 8 | B | B | A | 14 | B | 10 | B | NB | 20 | 285 | 20 | 285 | 20 | 285 |
|  |  | WB | 16 | 9 | 5 | B | A | A | 9 | A |  |  | WB | 10 | 104 | 11 | 104 | 10 | 104 |
|  |  | SB | 12 | 9 | 6 | B | A | A | 9 | A |  |  | SB | 4 | 78 | 4 | 78 | 4 | 78 |
|  |  | EB | 12 | 9 | 7 | B | A | A | 9 | A |  |  | EB | 14 | 115 | 14 | 115 | 14 | 115 |
|  | W. Broadway Frontage Rd \& 36th Avenue | NB | 10 | 12 | 8 | B | B | A | 9 | A | 3 | A | NB |  | 38 |  | 38 |  | 38 |
|  |  | WB | 3 | 1 | 1 | A | A | A | 2 | A |  |  | WB |  | 90 |  | 90 |  | 90 |
|  |  | SB | 8 | 0 | 4 | A | A | A | 6 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 2 | 1 | 4 | A | A | A | 1 | A |  |  | EB |  | 19 |  | 19 |  | 19 |
|  |  <br> 36th Avenue | NB | 37 | 13 | 6 | D | B | A | 17 | B | 22 | C | NB | 33 | 232 | 27 | 228 |  | 30 |
|  |  | WB | 52 | 36 | 10 | D | D | B | 29 | C |  |  | WB | 15 | 105 | 18 | 104 | 14 | 105 |
|  |  | SB | 45 | 23 | 6 | D | C | A | 22 | C |  |  | SB | 14 | 122 | 33 | 172 |  |  |
|  |  | EB | 44 | 27 | 6 | D | C | A | 29 | C |  |  | EB | 53 | 250 | 54 | 252 | 44 | 253 |
|  |  <br> Abbott Avenue | NB | 40 | 5 |  | D | A |  | 8 | A | 18 | B | NB | 18 | 121 | 18 | 122 |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  |  | WB |  |  |  |  |  |  |
|  |  | SB |  | 21 | 5 |  | C | A | 19 | B |  |  | SB |  |  | 30 | 216 | 29 | 216 |
|  |  | EB | 38 |  | 9 | D |  | A | 34 | C |  |  | EB | 45 | 205 |  |  | 42 | 205 |
|  | Halifax Avenue \& 34th Avenue | NB | 0 | 0 | 0 | A | A | A | 0 | A | 1 | A | NB |  |  |  |  |  |  |
|  |  | WB | 6 | 0 | 5 | A | A | A | 6 | A |  |  | WB |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 6 | 7 | 5 | A | A | A | 6 | A |  |  | EB |  |  |  |  |  |  |
| ت |  <br> 34th Avenue | NB | 28 |  | 11 | C |  | B | 26 | C | 15 | B | NB | 11 | 102 |  |  | 2 | 70 |
|  |  | WB | 20 |  | 15 | C |  | B | 15 | B |  |  | WB | 28 | 290 |  |  | 28 | 290 |
|  |  | SB | 10 |  | 11 | B |  | B | 10 | B |  |  | SB | 15 | 216 |  |  | 15 | 216 |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  |  <br> Terrace Mall Access | NB |  | 1 | 1 |  | A | A | 1 | A | 1 | A | NB |  |  |  | 8 |  | 8 |
|  |  | WB | 9 |  | 7 | A |  | A | 7 | A |  |  | WB |  | 42 |  |  |  | 42 |
|  |  | SB | 3 | 0 |  | A | A |  | 0 | A |  |  | SB |  | 36 |  | 36 |  |  |
|  |  | EB |  |  |  |  |  |  |  |  |  |  | EB |  |  |  |  |  |  |
|  | France Avenue \& 35th Avenue | NB | 2 | 1 | 1 | A | A | A | 1 | A | 3 | A | NB |  | 32 |  | 32 |  | 32 |
|  |  | WB | 11 | 9 | 8 | B | A | A | 10 | A |  |  | WB | 1 | 65 | 1 | 65 | 1 | 65 |
|  |  | SB | 3 | 1 | 1 | A | A | A | 1 | A |  |  | SB |  | 53 |  | 53 |  | 53 |
|  |  | EB | 11 | 12 | 6 | B | B | A | 9 | A |  |  | EB | 1 | 42 | 1 | 42 | 1 | 42 |

* Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
approximately 5,475 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
Source: WSB \& Associates
k:011484-05TTraffic [LOS Results.x|s|2030PM_LRT(344h)_GROWTH


## Appendix H

## Weekend Traffic Analysis

(Supporting Information)

# All Detector Report Mn/DOT Twin Cities Freeway Detectors 




## ATR locations on the TH 100 \& 36th Ave Ramps <br> All ramp volumes combined

| MAX Hourly Volume |  |  | 2102 Weekday |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4:30 PM | TO | 5:30 PM |  |  |  |
|  |  | MAX Hourly Volume |  | 1644 | f weekday |
| 3:30 PM | TO | 4:30 PM |  | Sat |  |
|  |  |  |  | 78\% |  |
|  |  |  | MAX Hourly Volume |  | 1607 |
| Noon | TO | 1:00 PM |  |  | Sun |
|  |  |  |  |  | 76\% |


| Weekdays: | Saturdays: | Sundays: |
| ---: | ---: | ---: |
| $4 / 19 / 2011$ | $4 / 23 / 2011$ | $4 / 24 / 2011$ |
| $4 / 20 / 2011$ | $4 / 30 / 2011$ | $5 / 1 / 2011$ |
| $4 / 21 / 2011$ | $5 / 7 / 2011$ | $5 / 8 / 2011$ |
| $4 / 26 / 2011$ | $5 / 14 / 2011$ | $5 / 15 / 2011$ |
| $4 / 27 / 2011$ |  |  |
| $4 / 28 / 2011$ |  |  |
| $5 / 3 / 2011$ |  |  |
| $5 / 4 / 2011$ |  |  |
| $5 / 5 / 2011$ |  |  |
| $5 / 10 / 2011$ |  |  |
| $5 / 11 / 2011$ |  |  |
| $5 / 12 / 2011$ |  |  |

Bottineau - Robbinsdale
Weekend Traffic Ratio to be used for Rainbow (Terrace Mall) Modeling Approximately 80\% of the weekday PM peak hour traffic is present during the highest hourly volume on both Saturday and Sunday 5/18/2011

|  |  |  | Totals (Hourly volumes) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | ALL | ALL | ALL |
|  |  |  | RAMPS | RAMPS | RAMPS |
|  |  |  | Weekday | Sat | Sun |
| 12:00 AM | TO | 1:00 AM | 196 | 343 | 395 |
| 12:15 AM | TO | 1:15 AM | 161 | 307 | 354 |
| 12:30 AM | TO | 1:30 AM | 139 | 278 | 313 |
| 12:45 AM | TO | 1:45 AM | 126 | 256 | 269 |
| 1:00 AM | TO | 2:00 AM | 119 | 235 | 247 |
| 1:15 AM | TO | 2:15 AM | 116 | 213 | 229 |
| 1:30 AM | TO | 2:30 AM | 111 | 199 | 223 |
| 1:45 AM | TO | 2:45 AM | 103 | 194 | 214 |
| 2:00 AM | TO | 3:00 AM | 98 | 182 | 201 |
| 2:15 AM | TO | 3:15 AM | 89 | 170 | 181 |
| 2:30 AM | TO | 3:30 AM | 79 | 156 | 152 |
| 2:45 AM | TO | 3:45 AM | 76 | 133 | 132 |
| 3:00 AM | TO | 4:00 AM | 75 | 122 | 118 |
| 3:15 AM | TO | 4:15 AM | 84 | 119 | 107 |
| 3:30 AM | TO | 4:30 AM | 100 | 114 | 100 |
| 3:45 AM | TO | 4:45 AM | 129 | 114 | 90 |
| 4:00 AM | TO | 5:00 AM | 163 | 112 | 85 |
| 4:15 AM | TO | 5:15 AM | 194 | 117 | 86 |
| 4:30 AM | TO | 5:30 AM | 261 | 128 | 88 |
| 4:45 AM | TO | 5:45 AM | 374 | 155 | 109 |
| 5:00 AM | TO | 6:00 AM | 498 | 195 | 137 |
| 5:15 AM | TO | 6:15 AM | 627 | 223 | 160 |
| 5:30 AM | TO | 6:30 AM | 793 | 270 | 207 |
| 5:45 AM | TO | 6:45 AM | 1017 | 330 | 255 |
| 6:00 AM | TO | 7:00 AM | 1236 | 387 | 298 |
| 6:15 AM | TO | 7:15 AM | 1434 | 436 | 333 |
| 6:30 AM | TO | 7:30 AM | 1592 | 479 | 349 |
| 6:45 AM | TO | 7:45 AM | 1651 | 531 | 377 |
| 7:00 AM | TO | 8:00 AM | 1737 | 591 | 401 |
| 7:15 AM | TO | 8:15 AM | 1774 | 633 | 443 |
| 7:30 AM | TO | 8:30 AM | 1762 | 705 | 501 |
| 7:45 AM | TO | 8:45 AM | 1696 | 784 | 550 |
| 8:00 AM | TO | 9:00 AM | 1572 | 855 | 631 |
| 8:15 AM | TO | 9:15 AM | 1471 | 951 | 730 |
| 8:30 AM | TO | 9:30 AM | 1383 | 1033 | 832 |
| 8:45 AM | TO | 9:45 AM | 1313 | 1085 | 944 |
| 9:00 AM | TO | 10:00 AM | 1249 | 1183 | 1035 |
| 9:15 AM | TO | 10:15 AM | 1200 | 1248 | 1089 |
| 9:30 AM | TO | 10:30 AM | 1156 | 1321 | 1139 |
| 9:45 AM | TO | 10:45 AM | 1138 | 1372 | 1169 |
| 10:00 AM | TO | 11:00 AM | 1140 | 1373 | 1212 |
| 10:15 AM | TO | 11:15 AM | 1158 | 1409 | 1249 |
| 10:30 AM | TO | 11:30 AM | 1184 | 1446 | 1264 |
| 10:45 AM | TO | 11:45 AM | 1213 | 1489 | 1297 |
| 11:00 AM | TO | Noon | 1238 | 1518 | 1324 |


|  |  | Totals (Hourly volumes) |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | ALL | ALL | ALL |
|  |  | RAMPS | RAMPS | RAMPS |
|  |  | Weekday | Sat | Sun |
| 11:15 AM | TO 12:15 PM | 1258 | 1581 | 1399 |
| 11:30 AM | TO 12:30 PM | 1278 | 1594 | 1511 |
| 11:45 AM | TO 12:45 PM | 1295 | 1596 | 1580 |
| Noon | TO 1:00 PM | 1299 | 1574 | 1607 |
| 12:15 PM | TO 1:15 PM | 1317 | 1557 | 1597 |
| 12:30 PM | TO 1:30 PM | 1314 | 1549 | 1549 |
| 12:45 PM | TO 1:45 PM | 1333 | 1550 | 1503 |
| 1:00 PM | TO 2:00 PM | 1360 | 1600 | 1478 |
| 1:15 PM | TO 2:15 PM | 1403 | 1591 | 1470 |
| 1:30 PM | TO 2:30 PM | 1485 | 1597 | 1434 |
| 1:45 PM | TO 2:45 PM | 1596 | 1609 | 1422 |
| 2:00 PM | TO 3:00 PM | 1711 | 1623 | 1419 |
| 2:15 PM | TO 3:15 PM | 1823 | 1625 | 1411 |
| 2:30 PM | TO 3:30 PM | 1901 | 1620 | 1420 |
| 2:45 PM | TO 3:45 PM | 1909 | 1634 | 1427 |
| 3:00 PM | TO 4:00 PM | 1932 | 1628 | 1408 |
| 3:15 PM | TO 4:15 PM | 1924 | 1643 | 1414 |
| 3:30 PM | TO 4:30 PM | 1931 | 1644 | 1432 |
| 3:45 PM | TO 4:45 PM | 2000 | 1618 | 1438 |
| 4:00 PM | TO 5:00 PM | 2033 | 1596 | 1457 |
| 4:15 PM | TO 5:15 PM | 2091 | 1585 | 1432 |
| 4:30 PM | TO 5:30 PM | 2102 | 1544 | 1413 |
| 4:45 PM | TO 5:45 PM | 2068 | 1512 | 1423 |
| 5:00 PM | TO 6:00 PM | 2042 | 1485 | 1415 |
| 5:15 PM | TO 6:15 PM | 1983 | 1444 | 1388 |
| 5:30 PM | TO 6:30 PM | 1954 | 1446 | 1339 |
| 5:45 PM | TO 6:45 PM | 1887 | 1413 | 1253 |
| 6:00 PM | TO 7:00 PM | 1797 | 1345 | 1178 |
| 6:15 PM | TO 7:15 PM | 1669 | 1278 | 1135 |
| 6:30 PM | TO 7:30 PM | 1516 | 1208 | 1112 |
| 6:45 PM | TO 7:45 PM | 1372 | 1161 | 1086 |
| 7:00 PM | TO 8:00 PM | 1248 | 1118 | 1047 |
| 7:15 PM | TO 8:15 PM | 1186 | 1090 | 1045 |
| 7:30 PM | TO 8:30 PM | 1144 | 1030 | 1035 |
| 7:45 PM | TO 8:45 PM | 1130 | 973 | 1001 |
| 8:00 PM | TO 9:00 PM | 1110 | 944 | 978 |
| 8:15 PM | TO 9:15 PM | 1088 | 900 | 899 |
| 8:30 PM | TO 9:30 PM | 1055 | 901 | 796 |
| 8:45 PM | TO 9:45 PM | 994 | 886 | 728 |
| 9:00 PM | TO 10:00 PM | 923 | 872 | 661 |
| 9:15 PM | TO 10:15 PM | 852 | 867 | 612 |
| 9:30 PM | TO 10:30 PM | 781 | 833 | 590 |
| 9:45 PM | TO 10:45 PM | 732 | 833 | 569 |
| 10:00 PM | TO 11:00 PM | 682 | 781 | 542 |
| 10:15 PM | TO 11:15 PM | 626 | 730 | 495 |
| 10:30 PM | TO 11:30 PM | 559 | 689 | 431 |
| 10:45 PM | TO 11:45 PM | 486 | 612 | 368 |
| 11:00 PM | TO Midnight | 417 | 579 | 304 |

# TRIP generation An ITE Informational Report 

## 8th Edition • Volume 3 of 3

## Supermarket (850)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area<br>On a: Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 40
Average 1000 Sq. Feet GFA: 59
Directional Distribution: 51\% entering, 49\% exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 10.50 | $5.15-20.29$ | 4.97 |

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area
On a: Saturday,
Peak Hour of Generator

Number of Studies: 32
Average 1000 Sq. Feet GFA: 67
Directional Distribution: $51 \%$ entering, $49 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

|  | Average Rate | Range of Rates |
| :---: | :---: | :---: |
| 10.85 | $5.78-22.60$ | Standard Deviation |



Institute of Transportation Engineers
To: Joe Gladke, PE Copy: N/A

| From: | Tony Heppelmann, PE |
| :--- | :--- |
|  | Chad Ellos, PE |
|  | Dean Chamberlain, EIT |

Date: June 2011 File: WSB No. 01484-05
Subject: Operations Analysis of LRT and BRT at the $42^{\text {nd }}$ Avenue Crossing Bottineau Transitway - Robbinsdale

## Introduction

The purpose of this study is to document traffic and transit (light-rail transit [LRT] and bus rapid transit [BRT]) operations at the $42^{\text {nd }}$ Avenue (CSAH 9) railroad crossing in the City of Robbinsdale. It is assumed that either LRT or BRT will operate within the railroad right-of-way and include a Park \& Ride facility south of $42^{\text {nd }}$ Avenue. At the crossing, a flashing signal system and gate arms will provide transit preemption according to the assumptions in Technical Memorandum 1.

Within the study area are seven key intersections on $42^{\text {nd }}$ Avenue that were analyzed, which include:

- TH 100 Northbound Ramps
- Regent Avenue
- Quail Avenue
- Railroad Avenue
- Hubbard Avenue
- West Broadway Avenue (CSAH 8)
- Bottineau Boulevard (CSAH 81)

The study area, key intersections, and transit/railroad crossing location being analyzed are shown on Figure 1.


It was assumed that freight trains would use the crossing during non-peak times and therefore, freight trains were excluded from this analysis. Three scenarios were analyzed as part of this study. All the scenarios assume the existing (2010) roadway network. These scenarios include:

- Existing (2010 traffic volumes)
- LRT - (2030 forecast traffic volumes)
- BRT - (2030 forecast traffic volumes)

For the LRT and BRT scenarios, the crossing would consist of a two-way transitway and separate freight line as displayed in Appendix A.

## Existing Conditions

## Roadway System

The major roadways within the study area are $42^{\text {nd }}$ Avenue, West Broadway Avenue, and CSAH 81. The functional classification of all the roadways being analyzed is provided in Table 1.

Table 1: Roadway Functional Classification

| Roadway | Functional <br> Classification |
| :--- | :--- |
| 42nd Avenue | A Minor Augmentor |
| CSAH 81 | A Minor Augmentor |
| CSAH 8 (West Broadway Avenue) | Major Collector |
| Regent Avenue | Local |
| Quail Avenue | Local |
| Railroad Avenue | Local |
| Hubbard Avenue | Local |

SOURCE: Metropolitan Council
k:101484-05\AdminlDocs \TTables.x|s]Fun - Juris Class Tables
Adjacent to $42^{\text {nd }}$ Avenue, single-family and multi-family residential land uses are mainly prevalent west of the railroad/transit crossing. East of the crossing has mainly commercial land uses. Figure 2 displays the 2005 land-use and the proposed 2030 land-use based on the City of Robbinsdale 2030 Comprehensive Plan.

Figure 2: 2005 and Proposed 2030 Land-use


## Existing Traffic

Average daily traffic counts in 2009 revealed that 42nd Avenue carries 11,600 vehicles per day (vpd) from TH 100 to West Broadway Avenue and 10,900 vpd from West Broadway Avenue to CSAH 81. CSAH 81 carries 15,700 vpd north of 42 nd Avenue and 16,600 vpd south of 42 nd Avenue1.

Turning movement data used to determine peak hour conditions was collected in early November of 2010. For reference, these counts are provided in Appendix B. This data was then analyzed to determine the AM and PM peak hours. Figure 3 shows the existing (2010) AM and PM peak hour turning movements and lane configurations at the key intersections.

[^8]
## Modeling Methodology

VISSIM computer software was used to model the Existing, LRT, and BRT scenarios. VISSIM is a microscopic, time step and behavior based simulation model developed to model urban traffic and public transit operations. It is approved by $\mathrm{Mn} / \mathrm{DOT}$ as a method to analyze complex transportation systems involving traffic and transit operations under constraints such as lane configuration, traffic composition, traffic signals, transit stops, etc., thus making it a useful tool for the evaluation of various alternatives based on transportation engineering and planning measures of effectiveness.

## Level of Service Definition

The traffic operations analysis results are presented in the form of a letter grade from A to F, called level of service (LOS). The letter grade provides a qualitative assessment of the intersection operations based on the amount of delay per vehicle. The LOS system is set up similar to a report card with " $A$ " representing high quality operations and " $F$ " representing poor operations. At LOS A, motorists experience very little delay or interference. On a roadway or intersection with LOS F conditions, motorists would experience severe congestion and extreme delay, i.e., gridlock. The LOS analysis criteria for signalized and unsignalized intersections are shown in Figure 4. Although LOS A conditions represents the best possible level of traffic flow, the cost to construct intersections to such a high standard exceeds the benefit to the user. Within an urbanized area, it is generally regarded that LOS D or better provides for acceptable operations.

Figure 4: Delay Based LOS Thresholds


## Existing Operations

Existing bus data was collected and integrated into the traffic model. This included bus frequency, bus stops, and three seconds of stopped time before proceeding across the railroad tracks. Refer to Appendix C for bus data within the study area.

A summary of the existing level of service conditions is provided below. Appendix $\mathbf{D}$ provides more detail as to the individual movements at each intersection and the queue lengths recorded.

Table 2 summarizes the LOS conditions at the key intersections based on the existing lane geometry, Hennepin County signal timing, and 2010 traffic volumes as shown in Figure 3. All intersections are currently operating at LOS C or better during the am and pm peak hours. A few movements were operating at LOS D, refer to Appendix $D$. Adequate storage for queuing vehicles was provided at all approaches.

Table 2: Intersection LOS - Existing (No Build)

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) |
| TH 100 NB Off-Ramp \& 42nd Ave | B | 14 | C | 21 |
| Regent Ave \& 42nd Ave | A | 1 | A | 1 |
| Quail Ave \& 42nd Ave | A | 1 | A | 1 |
| Railroad Ave \& 42nd Ave | A | 1 | A | 1 |
| Hubbard Ave \& 42nd Ave | A | 1 | A | 3 |
| CSAH 8 \& 42nd Ave | B | 11 | C | 24 |
| CSAH 81 \& 42nd Ave | C | 24 | C | 30 |

K: $01484-05 \backslash$ Admin\DocsIMemos_Reports - 42 nd $\$ [LOS Summary Tables.xls]No Build
Source: WSB \& Associates

## Traffic Forecasts

The project is located in a historically built-up area in the City of Robbinsdale, which is an inner suburb of Minneapolis. Based the City's comprehensive plan and other known development plans, no increase in traffic within the study area over the next 20 years was assumed. Historical traffic volumes for the study area, provided in Appendix E, generally supports this assumption by showing that traffic volumes have decreased or remained relatively unchanged.

Year 2030 traffic forecasts for the LRT and BRT scenarios do include increased traffic due to the proposed Park \& Ride station within the study area. The trip generation and distribution methodology used to route this additional traffic is described below.

## Park \& Ride Distribution

For the LRT scenario, a 250 vehicle Park \& Ride lot was assumed. The BRT Park \& Ride lot was assumed to hold 200 vehicles. Both scenarios assumed the lot would be filled during the AM
peak hour and emptied during the PM peak hour. According to Metro Transit, the market area related to this Park \& Ride is approximately 7.5 square miles as shown by the dashed red line on Figure 5. This figure also shows the assumed distribution percentages (by roadway) of vehicles traveling to and from the Park \& Ride lot.

Figure 5: Park \& Ride Distribution


## Operations Analysis

## General LRT Operating Assumptions

The LRT is assumed to operate at a frequency of 7.5 minutes, in each direction, during the peak hours with a station dwell time of approximately twenty seconds. Random arrival times at the Robbinsdale Transit Center Station are based on a mean value of 450 seconds ( 7.5 minutes) with a standard deviation of 90 seconds (representing $\pm 20 \%$ ). Since a station is present within the study area, LRT speeds will vary. The desired LRT speed north and south of the station is 55 mph . Each LRT train consists of three 94 foot cars with an acceleration rate of 3.0 mph per second and a deceleration rate of 1.5 mph per second. LRT is given preemption at the crossing.

## General BRT Operating Assumptions

The BRT is assumed to operate at a frequency of 3.75 minutes, in each direction, during the peak hours with a station dwell time of approximately twenty-five seconds. Random arrival times at the Robbinsdale Transit Center Station are based on a mean value of 225 seconds (3.75
minutes) with a standard deviation of 45 seconds (representing $\pm 20 \%$ ). Since a station is present within the study area, BRT speeds will vary. The desired BRT speed north and south of the station is 55 mph . Each BRT vehicle consists of a 60 -foot bus with an acceleration rate of 1.5 mph per second and a deceleration rate of 1.5 mph per second. BRT is given preemption at the crossing.

## Crossing Signal Assumptions

Currently, an overhead flashing signal system alerts drivers that a train is approaching the crossing. For the proposed LRT and BRT scenarios, gate arms would be added to this system. The amount of time assumed from when the signal starts flashing to when the gates are back up is approximately 25 seconds plus the transit vehicle crossing time, as itemized below ${ }^{2}$.

- Flashing warning prior to gates descending: 3 to 5 seconds
- Gates descending: 10 to 12 seconds
- Time after gates are down and before transit vehicle arrives at crossing: 5 seconds
- Transit vehicle crossing the roadway ${ }^{3}$
o LRT = 3-6 seconds
o BRT: 2-3 seconds
- Gates ascending: 5 seconds

This delay ( 27 to 31 seconds) will be experienced by drivers each time a transit vehicle crosses $42^{\text {nd }}$ Avenue.

## Traffic LOS Conditions

Analysis data from the models was collected for both the traffic and transit (LRT or BRT) traveling through the study area. A summary of the levels of service conditions and queue lengths for each scenario is provided below. Appendix $D$ provides more detail as to the individual movements at each intersection and the queue lengths recorded through the study area.

## LRT Scenario

Table 3 summarizes the LOS conditions for the forecasted traffic volumes as shown in Figure 6. All intersections operate at LOS C or better during the AM and PM peak hours. These conditions were similar to the Existing conditions. A few movements in the peak hours are operating at LOS D and E as shown in Appendix D. The westbound left-turn movement at the West Broadway Avenue (CSAH 8) intersection and the southbound left-turn movement at the CSAH 81 intersection were operating just into LOS E during the PM peak hour.

[^9]

Eastbound queue lengths at West Broadway Avenue intersection and Hubbard Avenue intersection were also reviewed to determine if vehicles were backing up onto the railroad tracks. The distance between Hubbard Avenue and the railroad tracks is approximately 350 feet. The maximum queue recorded at Hubbard Avenue during the peak hours was 208 feet (PM peak hour).

Table 3: Intersection LOS - LRT Scenario

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) |
| TH 100 NB Off-Ramp \& 42nd Ave | B | 15 | C | 24 |
| Regent Ave \& 42nd Ave | A | 2 | A | 2 |
| Quail Ave \& 42nd Ave | A | 1 | A | 1 |
| Railroad Ave \& 42nd Ave | A | 2 | A | 2 |
| Hubbard Ave \& 42nd Ave | A | 2 | A | 6 |
| CSAH 8 \& 42nd Ave | B | 14 | C | 25 |
| CSAH 81 \& 42nd Ave | C | 25 | C | 30 |

K:\01484-05\AdminlDocs\Memos_Reports - 42nd \[LOS Summary Tables.xls]LRT
Source: WSB \& Associates

## BRT Scenario

Table 4 summarizes the LOS conditions for the forecasted traffic volumes as shown in Figure 7. All intersections operate at LOS C or better during the AM and PM peak hours. These conditions were similar to the Existing conditions. A few movements in the peak hours are operating at LOS D and E as shown in Appendix D. As with the LRT scenario, the westbound left-turn movement at the West Broadway Avenue (CSAH 8) intersection and the southbound left-turn movement at the CSAH 81 intersection were operating at LOS E during the PM peak hour. The maximum queue recorded at Hubbard Avenue during the peak hours was 215 feet (PM peak hour).

Table 4: Intersection LOS - BRT Scenario

| Intersection | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | LOS | Control <br> Delay <br> (sec/veh) | LOS | Control <br> Delay <br> (sec/veh) |
| TH 100 NB Off-Ramp \& 42nd Ave | B | 15 | C | 24 |
| Regent Ave \& 42nd Ave | A | 2 | A | 2 |
| Quail Ave \& 42nd Ave | A | 1 | A | 1 |
| Railroad Ave \& 42nd Ave | A | 1 | A | 2 |
| Hubbard Ave \& 42nd Ave | A | 1 | A | 6 |
| CSAH 8 \& 42nd Ave | B | 13 | C | 25 |
| CSAH 81 \& 42nd Ave | C | 25 | C | 30 |



## Additional Analyses

## Sensitivity Analysis

Since the traffic forecasts were the same as the existing counts, a sensitivity analysis was also performed to determine if an increase in forecasted traffic would result in acceptable conditions. Even though traffic volumes in the study area are not anticipated to increase in the future, traffic on TH 100 is forecasted to increase. Congestion on TH 100 could lead to an increase in vehicles using CSAH 81 and $42^{\text {nd }}$ Avenue during peak traffic hours. Revised 2030 forecasts for this analysis were developed assuming an annual increase of one percent per year.

For the sensitivity analysis, signal timing at the West Broadway Avenue intersection was revised. The existing 45 second cycle length was revised to 90 seconds. The CSAH 81 intersection also has a cycle length of 90 seconds thus preserving signal coordination.

The results of the sensitivity analysis were similar to the original analysis, producing acceptable LOS conditions for both the LRT and BRT scenarios. With the increase in traffic, eastbound queues at the West Broadway Avenue and Hubbard Avenue intersections also increased.

During the PM peak hour, the LRT maximum queue increased to 328 feet while the BRT maximum queue increased to 348 feet. With this growth scenario, the BRT maximum queue could extend back to the tracks causing vehicles to be in harms way if a transit vehicle was approaching. Tables of these results are provided in Appendix F.

## Conclusions

- Future traffic levels on $42^{\text {nd }}$ Avenue are not expected to increase unless a Park \& Ride is built in the study area. A Park \& Ride will generate additional peak hour trips and only slightly increase traffic. Background traffic along $42^{\text {nd }}$ Avenue is not expected to increase based on historical counts.
- Both the LRT and BRT scenarios produce acceptable LOS conditions and queue lengths at all the area intersections. This assumes no traffic growth in the corridor.
- The sensitivity analysis, which included revising the 2030 forecasts to include an annual traffic increase of one percent, produced acceptable LOS conditions but increased the maximum queue lengths causing the BRT scenario to possibly strand vehicles on the tracks during the PM peak hour. The LRT scenario maximum queue length was only slightly less that of the BRT scenario.
- Interconnecting the railroad signal with the traffic signal at West Broadway Avenue may be needed if traffic volumes along CSAH 42 increase. By interconnecting the signals, a clearance phase could be introduced to allow eastbound vehicles to clear the tracks prior to a transit vehicle crossing $42^{\text {nd }}$ Avenue.


## Appendix A

## Crossing Detail Figure



## Appendix B

## Turning Movement Counts

(Taken by WSB or supplied by Hennepin County)

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave \& NB TH 100 Ramp
7:00-8:30 and 4:15-5:45
Robbinsdale, MN sunny

File Name : 42nd ave \& nb 100 ramp
Site Code : 00014841
Start Date : 11/3/2010
Page No : 1

Groups Printed- Unshifted

|  | NB TH 100 Ramp From North |  |  |  |  | 42nd Ave From East |  |  |  |  | NB TH 100 Ramp From South |  |  |  |  | 42nd Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 0 | 0 | 0 | 1 | 1 | 5 | 110 | 0 | 0 | 115 | 16 | 0 | 34 | 4 | 54 | 2 | 68 | 28 | 0 | 98 | 268 |
| 07:15 AM | 0 | 0 | 0 | 1 | 1 | 3 | 135 | 0 | 0 | 138 | 15 | 1 | 41 | 0 | 57 | 2 | 102 | 38 | 0 | 142 | 338 |
| 07:30 AM | 0 | 0 | 0 | 1 | 1 | 11 | 135 | 0 | 0 | 146 | 20 | 0 | 37 | 0 | 57 | 0 | 118 | 47 | 0 | 165 | 369 |
| 07:45 AM | 0 | 0 | 0 | 1 | 1 | 9 | 145 | 0 | 0 | 154 | 28 | 1 | 56 | 0 | 85 | 0 | 134 | 38 | 0 | 172 | 412 |
| Total | 0 | 0 | 0 | 4 | 4 | 28 | 525 | 0 | 0 | 553 | 79 | 2 | 168 | 4 | 253 | 4 | 422 | 151 | 0 | 577 | 1387 |
| 08:00 AM | 0 | 0 | 0 | 1 | 1 | 7 | 105 | 0 | 0 | 112 | 31 | 1 | 44 | 0 | 76 | 0 | 109 | 57 | 0 | 166 | 355 |
| 08:15 AM | 0 | 0 | 0 | 1 | 1 | 9 | 123 | 0 | 0 | 132 | 29 | 1 | 45 | 1 | 76 | 0 | 88 | 55 | 0 | 143 | 352 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 0 | 0 | 0 | 2 | 2 | 16 | 228 | 0 | 0 | 244 | 60 | 2 | 89 | 1 | 152 | 0 | 197 | 112 | 0 | 309 | 707 |

*** BREAK ***

| 04:15 PM | 0 | 0 | 0 | 1 | 1 | 6 | 143 | 0 | 0 | 149 | 82 | 5 | 155 | 2 | 244 | 0 | 131 | 54 | 0 | 185 | 579 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 0 | 0 | 0 | 3 | 3 | 11 | 144 | 0 | 0 | 155 | 55 | 2 | 161 | 0 | 218 | 0 | 126 | 44 | 0 | 170 | 546 |
| 04:45 PM | 0 | 0 | 0 | 4 | 4 | 6 | 143 | 0 | 0 | 149 | 71 | 3 | 149 | 12 | 235 | 0 | 153 | 70 | 0 | 223 | 611 |
| Total | 0 | 0 | 0 | 8 | 8 | 23 | 430 | 0 | 0 | 453 | 208 | 10 | 465 | 14 | 697 | 0 | 410 | 168 | 0 | 578 | 1736 |
| 05:00 PM | 0 | 0 | 0 | 1 | 1 | 11 | 152 | 0 | 0 | 163 | 84 | 3 | 163 | 2 | 252 | 0 | 152 | 59 | 0 | 211 | 627 |
| 05:15 PM | 0 | 0 | 0 | 4 | 4 | 12 | 136 | 0 | 0 | 148 | 94 | 5 | 170 | 3 | 272 | 0 | 147 | 48 | 0 | 195 | 619 |
| 05:30 PM | 0 | 0 | 0 | 2 | 2 | 8 | 131 | 0 | 0 | 139 | 59 | 1 | 133 | 2 | 195 | 0 | 142 | 79 | 0 | 221 | 557 |
| Grand Total | 0 | 0 | 0 | 21 | 21 | 98 | 1602 | 0 | 0 | 1700 | 584 | 23 | 1188 | 26 | 1821 | 4 | 1470 | 617 | 0 | 2091 | 5633 |
| Apprch \% | 0 | 0 | 0 | 100 |  | 5.8 | 94.2 | 0 | 0 |  | 32.1 | 1.3 | 65.2 | 1.4 |  | 0.2 | 70.3 | 29.5 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0.4 | 0.4 | 1.7 | 28.4 | 0 | 0 | 30.2 | 10.4 | 0.4 | 21.1 | 0.5 | 32.3 | 0.1 | 26.1 | 11 | 0 | 37.1 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave N \& Regent Ave
7:00-8:30 4:15-5:45
Robbinsdale, MN
cloudy, 30's

File Name : 42nd ave \& regent ave
Site Code : 00000002
Start Date : 10/27/2010
Page No : 1

Groups Printed- Unshifted

|  | Regent Ave N From North |  |  |  |  | 42nd Ave N From East |  |  |  |  | Regent Ave N From South |  |  |  |  | 42nd Ave N From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 1 | 0 | 0 | 0 | 1 | 0 | 89 | 11 | 0 | 100 | 3 | 0 | 21 | 0 | 24 | 12 | 68 | 1 | 0 | 81 | 206 |
| 07:15 AM | 4 | 0 | 0 | 0 | 4 | 1 | 128 | 5 | 0 | 134 | 4 | 0 | 21 | 0 | 25 | 9 | 117 | 1 | 0 | 127 | 290 |
| 07:30 AM | 4 | 0 | 1 | 0 | 5 | 2 | 118 | 12 | 0 | 132 | 10 | 0 | 19 | 0 | 29 | 18 | 107 | 3 | 0 | 128 | 294 |
| 07:45 AM | 1 | 0 | 1 | 0 | 2 | 1 | 159 | 19 | 0 | 179 | 8 | 0 | 22 | 0 | 30 | 36 | 134 | 3 | 0 | 173 | 384 |
| Total | 10 | 0 | 2 | 0 | 12 | 4 | 494 | 47 | 0 | 545 | 25 | 0 | 83 | 0 | 108 | 75 | 426 | 8 | 0 | 509 | 1174 |
| 08:00 AM | 2 | 0 | 2 | 0 | 4 | 0 | 120 | 8 | 0 | 128 | 7 | 0 | 33 | 0 | 40 | 15 | 127 | 0 | 0 | 142 | 314 |
| 08:15 AM | 3 | 2 | 1 | 0 | 6 | 1 | 111 | 2 | 0 | 114 | 3 | 1 | 24 | 0 | 28 | 4 | 115 | 2 | 0 | 121 | 269 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 5 | 2 | 3 | 0 | 10 | 1 | 231 | 10 | 0 | 242 | 10 | 1 | 57 | 0 | 68 | 19 | 242 | 2 | 0 | 263 | 583 |

*** BREAK ***

| 04:15 PM | 2 | 1 | 0 | 0 | 3 | 6 | 115 | 5 | 0 | 126 | 6 | 0 | 17 | 0 | 23 | 23 | 175 | 2 | 0 | 200 | 352 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 4 | 0 | 1 | 0 | 5 | 2 | 144 | 7 | 0 | 153 | 8 | 0 | 25 | 0 | 33 | 26 | 168 | 2 | 0 | 196 | 387 |
| 04:45 PM | 2 | 1 | 2 | 0 | 5 | 3 | 129 | 7 | 0 | 139 | 6 | 0 | 12 | 0 | 18 | 29 | 199 | 3 | 0 | 231 | 393 |
| Total | 8 | 2 | 3 | 0 | 13 | 11 | 388 | 19 | 0 | 418 | 20 | 0 | 54 | 0 | 74 | 78 | 542 | 7 | 0 | 627 | 1132 |
| 05:00 PM | 4 | 0 | 0 | 0 | 4 | 0 | 144 | 3 | 0 | 147 | 6 | 0 | 16 | 0 | 22 | 12 | 197 | 4 | 0 | 213 | 386 |
| 05:15 PM | 1 | 0 | 3 | 0 | 4 | 3 | 134 | 13 | 0 | 150 | 10 | 0 | 13 | 0 | 23 | 22 | 173 | 2 | 0 | 197 | 374 |
| 05:30 PM | 1 | 0 | 0 | 0 | 1 | 0 | 132 | 7 | 0 | 139 | 4 | 0 | 12 | 0 | 16 | 28 | 177 | 1 | 0 | 206 | 362 |
| Grand Total | 29 | 4 | 11 | 0 | 44 | 19 | 1523 | 99 | 0 | 1641 | 75 | 1 | 235 | 0 | 311 | 234 | 1757 | 24 | 0 | 2015 | 4011 |
| Apprch \% | 65.9 | 9.1 | 25 | 0 |  | 1.2 | 92.8 | 6 | 0 |  | 24.1 | 0.3 | 75.6 | 0 |  | 11.6 | 87.2 | 1.2 | 0 |  |  |
| Total \% | 0.7 | 0.1 | 0.3 | 0 | 1.1 | 0.5 | 38 | 2.5 | 0 | 40.9 | 1.9 | 0 | 5.9 | 0 | 7.8 | 5.8 | 43.8 | 0.6 | 0 | 50.2 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave N \& Quail Ave N
7:00-8:30 AM Turning Movement
Robbinsdale, MN
cloudy, 30's

File Name : 42nd ave \& quail ave
Site Code : 00000003
Start Date : 10/28/2010
Page No : 1

Groups Printed- Unshifted

|  | From North |  |  |  |  | 42nd Ave N From East |  |  |  |  | Quail Ave From South |  |  |  |  | 42nd Ave N From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 110 | 1 | 0 | 111 | 1 | 0 | 5 | 0 | 6 | 1 | 80 | 0 | 0 | 81 | 198 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 108 | 4 | 0 | 112 | 1 | 0 | 1 | 0 | 2 | 2 | 113 | 0 | 0 | 115 | 229 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 134 | 3 | 0 | 137 | 0 | 0 | 5 | 0 | 5 | 1 | 112 | 0 | 0 | 113 | 255 |
| 07:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 155 | 6 | 0 | 161 | 3 | 0 | 5 | 0 | 8 | 0 | 126 | 0 | 0 | 126 | 295 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 507 | 14 | 0 | 521 | 5 | 0 | 16 | 0 | 21 | 4 | 431 | 0 | 0 | 435 | 977 |
| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 93 | 2 | 0 | 95 | 4 | 0 | 0 | 0 | 4 | 0 | 132 | 0 | 0 | 132 | 231 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 105 | 2 | 0 | 107 | 0 | 0 | 2 | 0 | 2 | 0 | 109 | 0 | 0 | 109 | 218 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 198 | 4 | 0 | 202 | 4 | 0 | 2 | 0 | 6 | 0 | 241 | 0 | 0 | 241 | 449 |

*** BREAK ***

| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 128 | 5 | 0 | 133 | 3 | 0 | 3 | 0 | 6 | 3 | 165 | 0 | 0 | 168 | 307 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 132 | 1 | 0 | 133 | 2 | 0 | 4 | 0 | 6 | 2 | 191 | 0 | 0 | 193 | 332 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 136 | 2 | 0 | 138 | 2 | 0 | 3 | 0 | 5 | 4 | 196 | 0 | 0 | 200 | 343 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 396 | 8 | 0 | 404 | 7 | 0 | 10 | 0 | 17 | 9 | 552 | 0 | 0 | 561 | 982 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 131 | 3 | 0 | 134 | 5 | 0 | 3 | 0 | 8 | 2 | 195 | 0 | 0 | 197 | 339 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 156 | 2 | 0 | 158 | 2 | 0 | 4 | 0 | 6 | 1 | 210 | 0 | 0 | 211 | 375 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 137 | 3 | 0 | 140 | 3 | 0 | 2 | 0 | 5 | 2 | 210 | 0 | 0 | 212 | 357 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 1525 | 34 | 0 | 1559 | 26 | 0 | 37 | 0 | 63 | 18 | 1839 | 0 | 0 | 1857 | 3479 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 97.8 | 2.2 | 0 |  | 41.3 | 0 | 58.7 | 0 |  | 1 | 99 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0 | 43.8 | 1 | 0 | 44.8 | 0.7 | 0 | 1.1 | 0 | 1.8 | 0.5 | 52.9 | 0 | 0 | 53.4 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave \& Railroad Ave
7:00-8:30 and 4:15-5:45
Robbinsdale, MN
sunny

File Name : 42nd ave \& railroad ave
Site Code : 00000004
Start Date : 11/4/2010
Page No : 1

Groups Printed- Unshifted

|  | Railroad Ave From North |  |  |  |  | 42nd Ave From East |  |  |  |  | Railroad Ave From South |  |  |  |  | 42nd Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 3 | 0 | 0 | 0 | 3 | 1 | 111 | 1 | 0 | 113 | 2 | 0 | 1 | 0 | 3 | 0 | 82 | 1 | 0 | 83 | 202 |
| 07:15 AM | 0 | 0 | 2 | 0 | 2 | 1 | 123 | 0 | 0 | 124 | 4 | 0 | 4 | 0 | 8 | 2 | 120 | 0 | 0 | 122 | 256 |
| 07:30 AM | 1 | 0 | 2 | 0 | 3 | 3 | 131 | 0 | 0 | 134 | 4 | 0 | 3 | 0 | 7 | 0 | 120 | 0 | 0 | 120 | 264 |
| 07:45 AM | 0 | 0 | 1 | 0 | 1 | 3 | 142 | 2 | 0 | 147 | 3 | 0 | 2 | 0 | 5 | 5 | 125 | 3 | 0 | 133 | 286 |
| Total | 4 | 0 | 5 | 0 | 9 | 8 | 507 | 3 | 0 | 518 | 13 | 0 | 10 | 0 | 23 | 7 | 447 | 4 | 0 | 458 | 1008 |
| 08:00 AM | 1 | 0 | 0 | 0 | 1 | 0 | 79 | 0 | 0 | 79 | 1 | 0 | 1 | 0 | 2 | 3 | 119 | 1 | 0 | 123 | 205 |
| 08:15 AM | 2 | 0 | 0 | 0 | 2 | 0 | 83 | 1 | 0 | 84 | 0 | 0 | 2 | 0 | 2 | 3 | 114 | 2 | 0 | 119 | 207 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 3 | 0 | 0 | 0 | 3 | 0 | 162 | 1 | 0 | 163 | 1 | 0 | 3 | 0 | 4 | 6 | 233 | 3 | 0 | 242 | 412 |

*** BREAK ***

| 04:15 PM | 0 | 0 | 4 | 0 | 4 | 4 | 146 | 2 | 0 | 152 | 2 | 1 | 3 | 0 | 6 | 9 | 179 | 3 | 0 | 191 | 353 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 1 | 0 | 4 | 0 | 5 | 2 | 152 | 0 | 0 | 154 | 5 | 0 | 1 | 0 | 6 | 6 | 178 | 1 | 0 | 185 | 350 |
| 04:45 PM | 3 | 0 | 3 | 0 | 6 | 2 | 109 | 3 | 0 | 114 | 4 | 1 | 1 | 0 | 6 | 8 | 184 | 1 | 0 | 193 | 319 |
| Total | 4 | 0 | 11 | 0 | 15 | 8 | 407 | 5 | 0 | 420 | 11 | 2 | 5 | 0 | 18 | 23 | 541 | 5 | 0 | 569 | 1022 |
| 05:00 PM | 0 | 2 | 3 | 0 | 5 | 3 | 135 | 5 | 0 | 143 | 1 | 0 | 3 | 0 | 4 | 5 | 159 | 2 | 0 | 166 | 318 |
| 05:15 PM | 2 | 0 | 3 | 0 | 5 | 1 | 133 | 1 | 0 | 135 | 4 | 0 | 3 | 0 | 7 | 7 | 172 | 1 | 0 | 180 | 327 |
| 05:30 PM | 2 | 0 | 2 | 0 | 4 | 1 | 125 | 2 | 0 | 128 | 1 | 0 | 1 | 0 | 2 | 12 | 166 | 2 | 0 | 180 | 314 |
| Grand Total | 15 | 2 | 24 | 0 | 41 | 21 | 1469 | 17 | 0 | 1507 | 31 | 2 | 25 | 0 | 58 | 60 | 1718 | 17 | 0 | 1795 | 3401 |
| Apprch \% | 36.6 | 4.9 | 58.5 | 0 |  | 1.4 | 97.5 | 1.1 | 0 |  | 53.4 | 3.4 | 43.1 | 0 |  | 3.3 | 95.7 | 0.9 | 0 |  |  |
| Total \% | 0.4 | 0.1 | 0.7 | 0 | 1.2 | 0.6 | 43.2 | 0.5 | 0 | 44.3 | 0.9 | 0.1 | 0.7 | 0 | 1.7 | 1.8 | 50.5 | 0.5 | 0 | 52.8 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave \& Hubbard Ave/driveway 7:00-8:30 AM Turning Movement
Robbinsdale, MN sunny

File Name : 42nd ave \& hubbard ave
Site Code : 00000007
Start Date : 11/4/2010
Page No : 1

Groups Printed- Unshifted

|  | driveway From North |  |  |  |  | 42nd Ave From East |  |  |  |  | Hubbard Ave From South |  |  |  |  | 42nd Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 111 | 1 | 0 | 113 | 3 | 0 | 3 | 0 | 6 | 10 | 73 | 0 | 0 | 83 | 202 |
| 07:15 AM | 1 | 0 | 0 | 0 | 1 | 0 | 121 | 0 | 0 | 121 | 4 | 0 | 0 | 0 | 4 | 9 | 115 | 0 | 0 | 124 | 250 |
| 07:30 AM | 2 | 0 | 0 | 0 | 2 | 1 | 133 | 1 | 0 | 135 | 6 | 0 | 2 | 0 | 8 | 10 | 113 | 0 | 0 | 123 | 268 |
| 07:45 AM | 2 | 0 | 0 | 0 | 2 | 1 | 140 | 1 | 0 | 142 | 8 | 0 | 0 | 0 | 8 | 12 | 116 | 1 | 0 | 129 | 281 |
| Total | 5 | 0 | 0 | 0 | 5 | 3 | 505 | 3 | 0 | 511 | 21 | 0 | 5 | 0 | 26 | 41 | 417 | 1 | 0 | 459 | 1001 |
| 08:00 AM | 2 | 0 | 0 | 0 | 2 | 6 | 69 | 1 | 0 | 76 | 5 | 0 | 2 | 0 | 7 | 9 | 112 | 0 | 0 | 121 | 206 |
| 08:15 AM | 3 | 0 | 0 | 0 | 3 | 1 | 81 | 1 | 0 | 83 | 9 | 0 | 1 | 0 | 10 | 7 | 103 | 1 | 0 | 111 | 207 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 5 | 0 | 0 | 0 | 5 | 7 | 150 | 2 | 0 | 159 | 14 | 0 | 3 | 0 | 17 | 16 | 215 | 1 | 0 | 232 | 413 |

*** BREAK ***

| 04:15 PM | 2 | 0 | 0 | 0 | 2 | 0 | 137 | 1 | 0 | 138 | 11 | 0 | 0 | 0 | 11 | 20 | 165 | 0 | 0 | 185 | 336 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 6 | 0 | 0 | 0 | 6 | 0 | 143 | 1 | 0 | 144 | 11 | 0 | 2 | 0 | 13 | 7 | 176 | 0 | 0 | 183 | 346 |
| 04:45 PM | 1 | 0 | 0 | 0 | 1 | 2 | 109 | 1 | 0 | 112 | 19 | 1 | 4 | 0 | 24 | 9 | 184 | 0 | 0 | 193 | 330 |
| Total | 9 | 0 | 0 | 0 | 9 | 2 | 389 | 3 | 0 | 394 | 41 | 1 | 6 | 0 | 48 | 36 | 525 | 0 | 0 | 561 | 1012 |
| 05:00 PM | 2 | 0 | 0 | 0 | 2 | 2 | 143 | 0 | 0 | 145 | 11 | 0 | 1 | 1 | 13 | 15 | 146 | 0 | 0 | 161 | 321 |
| 05:15 PM | 6 | 0 | 0 | 0 | 6 | 3 | 131 | 0 | 0 | 134 | 11 | 0 | 0 | 0 | 11 | 10 | 163 | 1 | 0 | 174 | 325 |
| 05:30 PM | 4 | 0 | 0 | 0 | 4 | 1 | 125 | 1 | 0 | 127 | 15 | 0 | 1 | 0 | 16 | 14 | 163 | 2 | 0 | 179 | 326 |
| Grand Total | 31 | 0 | 0 | 0 | 31 | 18 | 1443 | 9 | 0 | 1470 | 113 | 1 | 16 | 1 | 131 | 132 | 1629 | 5 | 0 | 1766 | 3398 |
| Apprch \% | 100 | 0 | 0 | 0 |  | 1.2 | 98.2 | 0.6 | 0 |  | 86.3 | 0.8 | 12.2 | 0.8 |  | 7.5 | 92.2 | 0.3 | 0 |  |  |
| Total \% | 0.9 | 0 | 0 | 0 | 0.9 | 0.5 | 42.5 | 0.3 | 0 | 43.3 | 3.3 | 0 | 0.5 | 0 | 3.9 | 3.9 | 47.9 | 0.1 | 0 | 52 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave \& Co Rd 8/Broadway Ave
7:00-8:30 and 4:15-5:45
Robbinsdale, MN
sunny 50's

File Name : 42nd ave \& co rd 8
Site Code : 00014845
Start Date : 11/4/2010
Page No : 1

Groups Printed- Unshifted

|  | Co Rd 8/Broadway From North |  |  |  |  | 42nd Ave From East |  |  |  |  | Co Rd 8/Broadway From South |  |  |  |  | 42nd Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 9 | 15 | 15 | 0 | 39 | 8 | 85 | 9 | 0 | 102 | 0 | 16 | 14 | 0 | 30 | 18 | 51 | 4 | 1 | 74 | 245 |
| 07:15 AM | 8 | 20 | 19 | 1 | 48 | 5 | 84 | 13 | 0 | 102 | 10 | 13 | 26 | 2 | 51 | 24 | 88 | 3 | 1 | 116 | 317 |
| 07:30 AM | 15 | 22 | 21 | 1 | 59 | 9 | 103 | 8 | 0 | 120 | 10 | 18 | 19 | 0 | 47 | 22 | 82 | 15 | 2 | 121 | 347 |
| 07:45 AM | 11 | 22 | 19 | 1 | 53 | 7 | 112 | 9 | 1 | 129 | 3 | 18 | 18 | 1 | 40 | 20 | 93 | 13 | 0 | 126 | 348 |
| Total | 43 | 79 | 74 | 3 | 199 | 29 | 384 | 39 | 1 | 453 | 23 | 65 | 77 | 3 | 168 | 84 | 314 | 35 | 4 | 437 | 1257 |
| 08:00 AM | 7 | 17 | 17 | 0 | 41 | 12 | 64 | 9 | 2 | 87 | 8 | 6 | 10 | 1 | 25 | 15 | 84 | 10 | 0 | 109 | 262 |
| 08:15 AM | 9 | 17 | 10 | 1 | 37 | 11 | 48 | 7 | 0 | 66 | 4 | 6 | 20 | 1 | 31 | 23 | 92 | 7 | 1 | 123 | 257 |
| *** BREAK *** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 16 | 34 | 27 | 1 | 78 | 23 | 112 | 16 | 2 | 153 | 12 | 12 | 30 | 2 | 56 | 38 | 176 | 17 | 1 | 232 | 519 |

*** BREAK ***

| 04:15 PM | 11 | 52 | 25 | 1 | 89 | 37 | 89 | 22 | 4 | 152 | 13 | 41 | 33 | 1 | 88 | 28 | 130 | 17 | 1 | 176 | 505 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 6 | 27 | 28 | 1 | 62 | 18 | 105 | 18 | 1 | 142 | 14 | 43 | 28 | 0 | 85 | 46 | 112 | 22 | 2 | 182 | 471 |
| 04:45 PM | 7 | 28 | 17 | 0 | 52 | 12 | 75 | 15 | 2 | 104 | 17 | 28 | 25 | 2 | 72 | 37 | 154 | 17 | 1 | 209 | 437 |
| Total | 24 | 107 | 70 | 2 | 203 | 67 | 269 | 55 | 7 | 398 | 44 | 112 | 86 | 3 | 245 | 111 | 396 | 56 | 4 | 567 | 1413 |
| 05:00 PM | 8 | 30 | 21 | 0 | 59 | 25 | 99 | 15 | 2 | 141 | 15 | 39 | 35 | 0 | 89 | 29 | 109 | 20 | 2 | 160 | 449 |
| 05:15 PM | 9 | 27 | 25 | 2 | 63 | 27 | 87 | 19 | 2 | 135 | 17 | 41 | 34 | 4 | 96 | 32 | 138 | 15 | 1 | 186 | 480 |
| 05:30 PM | 9 | 36 | 24 | 1 | 70 | 14 | 87 | 11 | 1 | 113 | 14 | 32 | 25 | 1 | 72 | 24 | 120 | 15 | 0 | 159 | 414 |
| Grand Total | 109 | 313 | 241 | 9 | 672 | 185 | 1038 | 155 | 15 | 1393 | 125 | 301 | 287 | 13 | 726 | 318 | 1253 | 158 | 12 | 1741 | 4532 |
| Apprch \% | 16.2 | 46.6 | 35.9 | 1.3 |  | 13.3 | 74.5 | 11.1 | 1.1 |  | 17.2 | 41.5 | 39.5 | 1.8 |  | 18.3 | 72 | 9.1 | 0.7 |  |  |
| Total \% | 2.4 | 6.9 | 5.3 | 0.2 | 14.8 | 4.1 | 22.9 | 3.4 | 0.3 | 30.7 | 2.8 | 6.6 | 6.3 | 0.3 | 16 | 7 | 27.6 | 3.5 | 0.3 | 38.4 |  |

# WSB \& Associates 

701 Xenia Ave S
Minneapolis, MN

42nd Ave \& Co Rd 81/Bottaneau Blvd
7:00-8::30 and 4:15-5:45
Robbinsdale, MN sunny

File Name: 42nd ave \& co rd 81
Site Code : 00000006
Start Date : 11/3/2010
Page No : 1

Groups Printed- Unshifted

|  | Co Rd 81/Bottaneau From North |  |  |  |  | 42nd Ave From East |  |  |  |  | Co Rd 81/Bottaneau From South |  |  |  |  | 42nd Ave From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 07:00 AM | 9 | 136 | 10 | 0 | 155 | 8 | 61 | 13 | 0 | 82 | 5 | 57 | 15 | 0 | 77 | 32 | 28 | 11 | 0 | 71 | 385 |
| 07:15 AM | 16 | 144 | 6 | 0 | 166 | 12 | 61 | 12 | 0 | 85 | 1 | 67 | 26 | 0 | 94 | 55 | 56 | 21 | 0 | 132 | 477 |
| 07:30 AM | 23 | 169 | 6 | 0 | 198 | 19 | 104 | 14 | 0 | 137 | 0 | 111 | 22 | 0 | 133 | 70 | 48 | 11 | 0 | 129 | 597 |
| 07:45 AM | 19 | 168 | 12 | 0 | 199 | 9 | 80 | 17 | 0 | 106 | 1 | 84 | 22 | 0 | 107 | 55 | 45 | 6 | 0 | 106 | 518 |
| Total | 67 | 617 | 34 | 0 | 718 | 48 | 306 | 56 | 0 | 410 | 7 | 319 | 85 | 0 | 411 | 212 | 177 | 49 | 0 | 438 | 1977 |
| 08:00 AM | 11 | 149 | 16 | 0 | 176 | 11 | 41 | 12 | 0 | 64 | 5 | 69 | 25 | 0 | 99 | 54 | 52 | 15 | 0 | 121 | 460 |
| 08:15 AM | 8 | 139 | 3 | 0 | 150 | 16 | 73 | 8 | 0 | 97 | 5 | 54 | 35 | 0 | 94 | 36 | 53 | 10 | 0 | 99 | 440 |
| *** BREAK ** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 19 | 288 | 19 | 0 | 326 | 27 | 114 | 20 | 0 | 161 | 10 | 123 | 60 | 0 | 193 | 90 | 105 | 25 | 0 | 220 | 900 |

*** BREAK ***

| 04:15 PM | 20 | 113 | 22 | 0 | 155 | 17 | 66 | 5 | 0 | 88 | 15 | 172 | 54 | 0 | 241 | 35 | 110 | 21 | 0 | 166 | 650 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:30 PM | 18 | 107 | 35 | 0 | 160 | 17 | 70 | 11 | 0 | 98 | 8 | 194 | 58 | 0 | 260 | 40 | 108 | 31 | 0 | 179 | 697 |
| 04:45 PM | 19 | 84 | 22 | 0 | 125 | 21 | 60 | 7 | 0 | 88 | 12 | 199 | 54 | 0 | 265 | 34 | 118 | 19 | 0 | 171 | 649 |
| Total | 57 | 304 | 79 | 0 | 440 | 55 | 196 | 23 | 0 | 274 | 35 | 565 | 166 | 0 | 766 | 109 | 336 | 71 | 0 | 516 | 1996 |
| 05:00 PM | 22 | 119 | 29 | 0 | 170 | 23 | 61 | 10 | 0 | 94 | 21 | 211 | 57 | 0 | 289 | 56 | 142 | 17 | 0 | 215 | 768 |
| 05:15 PM | 13 | 107 | 32 | 0 | 152 | 26 | 75 | 17 | 0 | 118 | 10 | 203 | 62 | 0 | 275 | 57 | 107 | 21 | 0 | 185 | 730 |
| 05:30 PM | 14 | 109 | 20 | 0 | 143 | 17 | 74 | 13 | 0 | 104 | 6 | 138 | 48 | 0 | 192 | 29 | 120 | 20 | 0 | 169 | 608 |
| Grand Total | 192 | 1544 | 213 | 0 | 1949 | 196 | 826 | 139 | 0 | 1161 | 89 | 1559 | 478 | 0 | 2126 | 553 | 987 | 203 | 0 | 1743 | 6979 |
| Apprch \% | 9.9 | 79.2 | 10.9 | 0 |  | 16.9 | 71.1 | 12 | 0 |  | 4.2 | 73.3 | 22.5 | 0 |  | 31.7 | 56.6 | 11.6 | 0 |  |  |
| Total \% | 2.8 | 22.1 | 3.1 | 0 | 27.9 | 2.8 | 11.8 | 2 | 0 | 16.6 | 1.3 | 22.3 | 6.8 | 0 | 30.5 | 7.9 | 14.1 | 2.9 | 0 | 25 |  |

## Appendix C

Existing Bus Data


## Appendix D

## Detailed LOS Conditions and Queue Lengths

| Intersection |  | Appr | Forecast Volumes |  |  |  | Modeled Volumes |  |  |  | Error Volumes |  |  | $\begin{gathered} \text { Volume } \\ \text { Error } \\ \text { by Approach } \end{gathered}$ |  | Appr | Total Delay by Movement （Sec／Veh） |  |  | Level of Service by Movement |  |  | LOS by Approach （Sec／Veh） |  | LOS by Intersection （Sec／Veh） |  | Appr | Average \＆Maximum Traffic Queueing（feet） |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { 흘 } \\ & \text { B } \end{aligned}$ | Location |  |  |  |  |  | Left Turn | Through |  | Right Turn |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | L | T | R | Total |  |  |  |  | L | T | R | Total | L |  | T | R | Total | \％ | L | T | R | L | T | R |  | Delay | LOS | Delay | LOS | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\left\lvert\, \begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}\right.$ | Ave Queue | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ | $\begin{gathered} \text { Ave } \\ \text { Queue } \end{gathered}$ | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ |
|  | NB TH 100 Ramps \＆ 42nd Avenue | NB | 178 | 3 | 94 | 275 | 177 | 2 | 95 | 274 |  |  |  | －1 | －1 | 1 | －1 | 0\％ | NB | 31 | 33 | 10 | C | C | B | 24 | C | 14 | B | NB | 21 | 99 | 16 | 99 | 16 | 99 |
| ＊ |  | WB | 0 | 529 | 30 | 559 | 0 | 520 | 27 | 547 | 0 | －9 | －3 | －12 | －2\％ | WB | 0 | 11 | 5 | A | B | A | 11 | B | WB |  |  |  |  | 16 | 157 | 15 | 158 |
| $\dot{\omega}$ |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A | SB |  |  |  |  |  |  |  |  |
|  |  | EB | 180 | 476 | 0 | 656 | 173 | 483 | 0 | 656 | －7 | 7 | 0 | 0 | 0\％ | EB | 38 | 5 | 0 | D | A | A | 14 | B | EB | 22 | 107 |  |  | 18 | 107 |  |  |
|  | Regent Avenue \＆ | NB | 95 | 0 | 29 | 124 | 95 | 0 | 29 | 124 | 0 | 0 | 0 | 0 | 0\％ | NB | 12 | 0 | 9 | B | A | A | 11 | B |  |  | NB | 5 | 85 | 5 | 85 | 5 | 85 |
| 晋 | 42nd Avenue | WB | 44 | 453 | 4 | 501 | 42 | 442 | 4 | 488 | －2 | －11 | 0 | －13 | －3\％ | WB | 3 | 0 | 1 | A | A | A | 0 | A | 1 | A | WB |  | 38 |  | 38 |  | 38 |
| $5$ |  | SB | 4 | 0 | 11 | 15 | 4 | 0 | 10 | 14 | 0 | 0 | －1 | －1 | －7\％ | SB | 7 | 0 | 5 | A | A | A | 6 | A |  |  | SB |  | 27 |  | 27 |  | 27 |
|  |  | EB | 7 | 485 | 78 | 570 | 5 | 496 | 77 | 578 | －2 | 11 | －1 | 8 | 1\％ | EB | 2 | 0 | 1 | A | A | A | 0 | A |  |  | EB |  | 54 |  | 54 |  | 54 |
| $\Xi$ | Quail Avenue \＆ | NB | 11 | 0 | 8 | 19 | 11 | 0 | 7 | 18 | 0 | 0 | －1 | －1 | －5\％ | NB | 9 | 0 | 5 | A | A | A | 7 | A |  |  | NB |  | 33 |  |  |  | 33 |
| 豆 | 42nd Avenue | WB | 15 | 490 | 0 | 505 | 17 | 477 | 0 | 494 | 2 | －13 | 0 | －11 | －2\％ | WB | 2 | 0 | 0 | A | A | A | 0 | A | 0 | A | WB |  |  |  |  |  |  |
| $\left\|\begin{array}{c} \stackrel{\rightharpoonup}{\vec{a}} \\ 5 \end{array}\right\|$ |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 486 | 32 | 518 | 0 | 498 | 31 | 529 | 0 | 12 | －1 | 11 | 2\％ | EB | 0 | 0 | 1 | A | A | A | 0 | A |  |  | EB |  |  |  | 18 |  | 18 |
|  | Railroad Avenue \＆ | NB | 27 | 0 | 12 | 39 | 26 | 0 | 12 | 38 | －1 | 0 | 0 | －1 | －3\％ | NB | 9 | 0 | 5 | A | A | A | 8 | A |  |  | NB | 1 | 46 | 1 | 46 | 1 | 46 |
| 霛 | 42nd Avenue | WB | 2 | 465 | 7 | 474 | 2 | 456 | 7 | 465 | 0 | －9 | 0 | －9 | －2\％ | WB | 1 | 0 | 1 | A | A | A | 0 | A | 0 | A | WB |  |  |  |  |  |  |
| $\stackrel{\rightharpoonup}{5}$ |  | SB | 5 | 0 | 13 | 18 | 5 | 0 | 13 | 18 | 0 | 0 | 0 | 0 | 0\％ | SB | 11 | 0 | 5 | B | A | A | 7 | A |  |  | SB | 1 | 30 | 1 | 30 | 1 | 30 |
|  |  | EB | 4 | 480 | 10 | 494 | 5 | 492 | 9 | 506 | 1 | 12 | －1 | 12 | 2\％ | EB | 2 | 0 | 1 | A | A | A | 0 | A |  |  | EB |  | 21 |  | 21 |  | 21 |
|  | Hubbard Avenue \＆ | NB | 0 | 0 | 23 | 23 | 0 | 0 | 22 | 22 | 0 | 0 | －1 | －1 | －4\％ | NB | 0 | 0 | 5 | A | A | A | 5 | A |  |  | NB |  |  |  |  |  | 32 |
| 会 | 42nd Avenue | WB | 0 | 474 | 0 | 474 | 0 | 465 | 0 | 465 | 0 | －9 | 0 | －9 | －2\％ | WB | 0 | 0 | 0 | A | A | A | 0 | A | 1 | A | WB |  |  |  |  |  |  |
| $\stackrel{a}{5}$ |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 456 | 41 | 497 | 0 | 470 | 38 | 508 | 0 | 14 | －3 | 11 | 2\％ | EB | 0 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  | 24 |  | 24 |
|  |  | NB | 70 | 55 | 31 | 156 | 68 | 59 | 27 | 154 | －2 | 4 | －4 | －2 | －1\％ | NB | 15 | 13 | 7 | B | B | A | 13 | B |  |  | NB | 8 | 111 | 8 | 111 | 8 | 111 |
| 令 | 42nd Avenue | WB | 39 | 363 | 33 | 435 | 41 | 357 | 33 | 431 | 2 | －6 | 0 | －4 | －1\％ | WB | 17 | 11 | 7 | B | B | A | 11 | B | 11 | B | WB | 13 | 138 | 13 | 138 | 13 | 138 |
| 品 |  | SB | 76 | 81 | 41 | 198 | 76 | 81 | 40 | 197 | 0 | 0 | －1 | －1 | －1\％ | SB | 14 | 14 | 8 | B | B | A | 13 | B |  |  | SB | 10 | 120 | 10 | 120 | 10 | 120 |
|  |  | EB | 41 | 357 | 81 | 479 | 46 | 368 | 81 | 495 | 5 | 11 | 0 | 16 | 3\％ | EB | 16 | 9 | 6 | B | A | A | 9 | A |  |  | EB | 14 | 142 | 14 | 142 | 14 | 142 |
|  | CSAH 81 \＆ | NB | 95 | 331 | 7 | 433 | 88 | 334 | 9 | 431 | －7 | 3 | 2 | －2 | 0\％ | NB | 47 | 16 | 5 | D | B | A | 22 | C |  |  | NB | 29 | 133 | 30 | 133 | 27 | 133 |
| 怣 | 42nd Avenue | WB | 55 | 271 | 51 | 377 | 58 | 270 | 50 | 378 | 3 | －1 | －1 | 1 | 0\％ | WB | 52 | 33 | 23 | D | C | C | 35 | C | 24 | C | WB | 37 | 166 | 37 | 166 | 36 | 166 |
| $\begin{aligned} & 50 \\ & i n \end{aligned}$ |  | SB | 40 | 630 | 69 | 739 | 41 | 636 | 69 | 746 | 1 | 6 | 0 | 7 | 1\％ | SB | 51 | 19 | 9 | D | B | A | 20 | B |  |  | SB | 44 | 259 | 44 | 259 | 43 | 259 |
|  |  | EB | 53 | 189 | 222 | 464 | 50 | 194 | 228 | 472 | －3 | 5 | 6 | 8 | 2\％ | EB | 49 | 32 | 11 | D | C | B | 24 | C |  |  | EB | 26 | 151 | 28 | 151 | 25 | 151 |



| 달 | Intersection | Appr | Forecast Volumes |  |  |  | Modeled Volumes |  |  |  | Error Volumes |  |  | $\begin{gathered} \text { Volume } \\ \text { Error } \\ \text { by Approach } \end{gathered}$ |  | Appr | Total Delay by Movement (Sec/Veh) |  |  | Level of Service byMovement |  |  | LOS by Approach (Sec/Veh) |  | $\begin{gathered} \text { LOS by } \\ \text { Intersection } \\ (\text { Sec/Veh) } \end{gathered}$ |  | App | Average \& Maximum Traffic Queueing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Location |  |  |  |  |  | Left Turn | Through |  | Right Turn |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | L | T | R | Total |  |  |  |  | L | T | R | Total | L |  | T | R | Total | \% | L | T | R | L | T | R |  | Delay | Los | Delay | Los | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{gathered} \text { Max } \\ \text { Queue } \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c} \hline \text { Max } \\ \text { Queue } \end{array}$ |
|  | NB TH 100 Ramps \& 42nd Avenue | NB | 200 | 5 | 115 | 320 | 201 | 5 | 115 | 321 |  |  |  | 1 | 0 | 0 | 1 | 0\% | NB | 31 | 43 | 12 | c | D | в | 24 | C | 15 | в | NB | 23 | 108 | 19 | 108 | 19 | 108 |
|  |  | WB | 0 | 590 | 35 | 625 | 0 | 588 | 33 | 621 | 0 | -2 | -2 | -4 | -1\% | wB | 0 | 13 | 6 | A | B | A | 13 | в | wB |  |  |  |  | 21 | 205 | 20 | 205 |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | SB | 0 | 0 | 0 | A | A | A | 0 | A | SB |  |  |  |  |  |  |  |  |
|  |  | EB | 200 | 640 | 0 | 840 | 194 | 658 | 0 | 852 | -6 | 18 | 0 | 12 | 1\% | EB | 36 | 6 | 0 | D | A | A | 13 | в | EB | 25 | 131 |  |  | 21 | 131 |  |  |
|  <br> 42nd Avenue |  | NB | 105 | 0 | 35 | 140 | 105 | 0 | 36 | 141 | 0 | 0 | 1 | 1 | 1\% | NB | 15 | 0 | 11 | c | A | в | 14 | в | 2 | A | NB | 8 | 108 | 8 | 108 | 8 | 108 |
|  |  | WB | 50 | 505 | 5 | 560 | 45 | 500 | 4 | 549 | -5 | -5 | -1 | -11 | -2\% | WB | 4 | 1 | 2 | A | A | A | 1 | A |  |  | wB | 1 | 91 | 1 | 91 | 1 | 91 |
|  |  | SB | 5 | 0 | 15 | 20 | 4 | 0 | 15 | 19 | -1 | 0 | 0 | -1 | -5\% | SB | 9 | 0 | 6 | A | A | A | 7 | A |  |  | SB |  | 33 |  | 33 |  | 33 |
|  |  | EB | 10 | 655 | 90 | 755 | 9 | 679 | 85 | 773 | -1 | 24 | -5 | 18 | 2\% | EB | 2 | 1 | 2 | A | A | A | 1 | A |  |  | EB | 1 | 102 | 1 | 102 | 1 | 102 |
|  |  <br> 42nd Avenue | NB | 15 | 0 | 10 | 25 | 14 | 0 | 10 | 24 | -1 | 0 | 0 | -1 | -4\% | NB | 10 | 0 | 6 | в | A | A | 8 | A | 0 | A | NB |  | 36 |  |  |  | 36 |
|  |  | WB | 20 | 545 | 0 | 565 | 22 | 534 | 0 | 556 | 2 | -11 | 0 | -9 | -2\% | wB | 2 | 0 | 0 | A | A | A | 0 | A |  |  | wB |  | 16 |  | 16 |  |  |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 655 | 40 | 695 | 0 | 683 | 37 | 720 | 0 | 28 | -3 | 25 | 4\% | EB | 0 | 0 | 1 | A | A | A | 0 | A |  |  | EB |  |  |  | 22 |  | 22 |
|  |  | NB | 30 | 0 | 15 | 45 | 28 | 0 | 15 | 43 | -2 | 0 | 0 | -2 | -4\% | NB | 11 | 0 | 5 | в | A | A | 9 | A | 2 | A | NB | 1 | 55 | 1 | 55 | 1 | 55 |
|  |  | WB | 5 | 520 | 10 | 535 | 6 | 514 | 9 | 529 | 1 | -6 | -1 | -6 | -1\% | wB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | wB |  | 8 |  | 8 |  | 8 |
|  |  | SB | 10 | 0 | 15 | 25 | 9 | 0 | 14 | 23 | -1 | 0 | -1 | -2 | -8\% | SB | 14 | 0 | 6 | в | A | A | 9 | A |  |  | SB |  | 30 |  | 30 |  | 30 |
|  |  | EB | 5 | 645 | 15 | 665 | 6 | 672 | 15 | 693 | 1 | 27 | 0 | 28 | 4\% | EB | 2 | 2 | 2 | A | A | A | 2 | A |  |  | EB | 1 | 82 | 1 | 82 | 1 | 82 |
| Hubbard Avenue \& |  | NB | 0 | 0 | 30 | 30 | 0 | 0 | 29 | 29 | 0 | 0 | -1 | -1 | -3\% | NB | 0 | 0 | 6 | A | A | A | 6 | A | 2 | A | NB |  |  |  |  | 1 | 49 |
|  |  | WB | 0 | 535 | 0 | 535 | 0 | 527 | 0 | 527 | 0 | -8 | 0 | -8 | -1\% | wB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | wB |  |  |  | 31 |  |  |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 505 | 165 | 670 | 0 | 520 | 176 | 696 | 0 | 15 | 11 | 26 | 4\% | EB | 0 | 3 | 2 | A | A | A | 3 | A |  |  | EB |  |  | 1 | 112 | 1 | 112 |
| CSAH 8 \& 42nd Avenue |  | NB | 80 | 65 | 35 | 180 | 75 | 67 | 34 | 176 | -5 | 2 | -1 | -4 | -2\% | NB | 15 | 13 | 6 | в | B | A | 13 | B | 14 | B | NB | 8 | 87 | 8 | 87 | 8 | 87 |
|  |  | WB | 105 | 405 | 40 | 550 | 102 | 405 | 41 | 548 | -3 | 0 | 1 | -2 | 0\% | WB | 26 | 16 | 13 | C | B | в | 18 | B |  |  | wB | 29 | 224 | 29 | 224 | 29 | 224 |
|  |  | SB | 85 | 130 | 50 | 265 | 86 | 128 | 48 | 262 | 1 | -2 | -2 | -3 | -1\% | SB | 14 | 13 | 8 | в | B | A | 12 | B |  |  | SB | 11 | 116 | 11 | 116 | 11 | 116 |
|  |  | EB | 50 | 395 | 90 | 535 | 54 | 407 | 89 | 550 | 4 | 12 | -1 | 15 | 3\% | EB | 21 | 11 | 7 | c | B | A | 11 | B |  |  | EB | 22 | 161 | 22 | 161 | 22 | 161 |
| CSAH 81 \&42 nd Avenue |  | NB | 105 | 370 | 10 | 485 | 96 | 377 | 11 | 484 | -9 | 7 | 1 | -1 | 0\% | NB | 46 | 19 | 6 | D | B | A | 24 | c | 25 | c | NB | 34 | 152 | 34 | 152 | 32 | 152 |
|  |  | WB | 65 | 325 | 60 | 450 | 64 | 336 | 59 | 459 | -1 | 11 | -1 | 9 | 2\% | wB | 50 | 32 | 23 | D | C | C | 33 | C |  |  | wB | 42 | 192 | 43 | 192 | 41 | 192 |
|  |  | SB | 45 | 700 | 120 | 865 | 45 | 701 | 117 | 863 | 0 | 1 | -3 | -2 | 0\% | SB | 52 | 22 | 11 | D | C | B | 22 | C |  |  | SB | 54 | 278 | 55 | 278 | 53 | 278 |
|  |  | EB | 60 | 210 | 245 | 515 | 56 | 221 | 251 | 528 | -4 | 11 | 6 | 13 | 3\% | EB | 49 | 30 | 12 | D | C | B | 23 | C |  |  | EB | 30 | 161 | 31 | 161 | 29 | 161 |
|  | RR Xing | NB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | NB | 0 | 0 | 0 | A | A | A | 0 | A | 2 | A | NB |  |  |  |  |  |  |
|  | 42nd Avenue | WB | 0 | 535 | 0 | 535 | 0 | 529 | 0 | 529 | 0 | -6 | 0 | -6 | -1\% | wB | 0 | 3 | 0 | A | A | A | 3 | A |  |  | WB |  |  | 4 | 98 |  |  |
|  | b | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | ев | 0 | 670 | 0 | 670 | 0 | 696 | 0 | 696 | 0 | 26 | 0 | 26 | 4\% | eb | 0 | 1 | 0 | A | A | A | 1 | A |  |  | ев |  |  | 7 | 98 |  |  |





## Appendix E

Historical Traffic Counts


## Appendix F

## Sensitivity Analysis Results

(Revised Forecasts: Annual growth rate of one percent)

## *GROWTH SCENARIO* 2030 AM LRT



## *GROWTH SCENARIO* 2030 PM LRT



## ＊GROWTH SCENARIO＊ 2030 AM BRT

|  | Intersection | Appr | Forecast Volumes |  |  |  | Modeled Volumes |  |  |  | Error Volumes |  |  | VolumeErrorby Approach |  | Appr | Total Delay by Movement （Sec／Veh） |  |  | Level of Service byMovement |  |  | LOS by Approach （Sec／Veh） |  | $\begin{array}{\|c\|} \text { LOS by } \\ \text { Intersection } \\ \text { (Sec/Veh) } \end{array}$ |  | Appr | Average \＆Maximum Traffic Queueing（feet） |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 흘 | Location |  |  |  |  |  | Left Turn | Through |  | Right Turn |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | L | T | R | Total |  |  |  |  | L | T | R | Total | L |  | T | R | Total | \％ | L | T | R | L | T | R |  | Delay | Los | Delay | Los | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \\ \hline \end{array}$ | $\begin{array}{\|c\|c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | $\begin{array}{\|c\|} \hline \begin{array}{l} \text { Max } \\ \text { Queue } \end{array} \end{array}$ |
| NB TH 100 Ramps \＆ 42nd Avenue |  | NB | 220 | 5 | 119 | 344 | 220 | 5 | 119 | 344 |  |  |  | 0 | 0 | 0 | 0 | 0\％ | NB | 30 | 37 | 11 | c | D | в | 24 | c | 15 | в | NB | 23 | 124 | 20 | 125 | 19 | 125 |
|  |  | WB | 0 | 650 | 40 | 690 | 0 | 653 | 36 | 689 | 0 | 3 | －4 | －1 | 0\％ | wB | 0 | 14 | 7 | A | B | A | 14 | в | WB |  |  |  |  | 25 | 195 | 24 | 195 |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A | SB |  |  |  |  |  |  |  |  |
|  |  | EB | 220 | 683 | 0 | 903 | 214 | 700 | 0 | 914 | －6 | 17 | 0 | 11 | 1\％ | EB | 38 | 6 | 0 | D | A | A | 13 | B | EB | 28 | 141 |  |  | 24 | 142 |  |  |
| $\qquad$ |  | NB | 120 | 0 | 40 | 160 | 121 | 0 | 40 | 161 | 1 | 0 | 0 | 1 | 1\％ | NB | 18 | 0 | 14 | c | A | в | 17 | C | 3 | A | NB | 12 | 137 | 12 | 137 | 12 | 137 |
|  |  | wB | 55 | 555 | 5 | 615 | 49 | 554 | 5 | 608 | －6 | －1 | 0 | －7 | －1\％ | WB | 6 | 1 | 1 | A | A | A | 1 | A |  |  | WB | 1 | 109 | 1 | 109 | 1 | 109 |
|  |  | SB | 5 | 0 | 15 | 20 | 4 | 0 | 15 | 19 | －1 | 0 | 0 | －1 | －5\％ | SB | 12 | 0 | 7 | в | A | A | 8 | A |  |  | SB | 1 | 38 | 1 | 38 | 1 | 38 |
|  |  | EB | 10 | 692 | 100 | 802 | 9 | 715 | 95 | 819 | －1 | 23 | －5 | 17 | 2\％ | EB | 2 | 1 | 2 | A | A | A | 1 | A |  |  | EB | 1 | 131 | 1 | 131 | 1 | 131 |
| Quail Avenue \＆ <br> 42nd Avenue |  | NB | 15 | 0 | 10 | 25 | 13 | 0 | 10 | 23 | －2 | 0 | 0 | －2 | －8\％ | NB | 10 | 0 | 6 | в | A | A | 8 | A | 1 | A | NB |  | 39 |  |  |  | 39 |
|  |  | WB | 20 | 600 | 0 | 620 | 23 | 594 | 0 | 617 | 3 | －6 | 0 | －3 | 0\％ | wB | 3 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  | 29 |  | 29 |  |  |
|  |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 697 | 40 | 737 | 0 | 723 | 37 | 760 | 0 | 26 | －3 | 23 | 3\％ | EB | 0 | 1 | 1 | A | A | A | 1 | A |  |  | EB |  |  |  | 28 |  | 28 |
|  | 42nd Avenue | NB | 35 | 0 | 15 | 50 | 34 | 0 | 14 | 48 | －1 | 0 | －1 | －2 | －4\％ | NB | 10 | 0 | 6 | в | A | A | 9 | A | 2 | A | NB |  | 46 |  | 46 |  | 46 |
| ， |  | WB | 5 | 565 | 10 | 580 | 6 | 564 | 9 | 579 | 1 | －1 | －1 | －1 | 0\％ | wB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | wB |  | 10 |  | 10 |  | 10 |
| 旁 |  | SB | 10 | 0 | 20 | 30 | 9 | 0 | 18 | 27 | －1 | 0 | －2 | －3 | －10\％ | SB | 12 | 0 | 5 | B | A | A | 7 | A |  |  | SB |  | 38 |  | 38 |  | 38 |
|  |  | EB | 5 | 687 | 15 | 707 | 6 | 712 | 14 | 732 | 1 | 25 | －1 | 25 | 4\％ | EB | 4 | 2 | 3 | A | A | A | 2 | A |  |  | EB | 1 | 54 | 1 | 54 | 1 | 54 |
|  | Hubbard Avenue \＆42nd Avenue | NB | 0 | 0 | 30 | 30 | 0 | 0 | 29 | 29 | 0 | 0 | －1 | －1 | －3\％ | NB | 0 | 0 | 6 | A | A | A | 6 | A | 2 | A | NB |  |  |  |  | 1 | 48 |
| 䂞 |  | WB | 0 | 580 | 0 | 580 | 0 | 576 | 0 | 576 | 0 | －4 | 0 | －4 | －1\％ | wB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | WB |  |  |  |  |  |  |
| $\begin{gathered} \frac{80}{3} \\ \stackrel{y}{2} \end{gathered}$ |  | SB | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\％ | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |
|  |  | EB | 0 | 565 | 147 | 712 | 0 | 580 | 158 | 738 | 0 | 15 | 11 | 26 | 4\％ | EB | 0 | 3 | 2 | A | A | A | 3 | A |  |  | ев |  |  | 1 | 95 | 1 | 95 |
|  | 42nd Avenue | NB | 90 | 70 | 40 | 200 | 85 | 73 | 39 | 197 | －5 | 3 | －1 | －3 | －2\％ | NB | 16 | 12 | 6 | B | B | A | 13 | B | 14 | ${ }^{\text {B }}$ | NB | 9 | 90 | 9 | 90 | 9 | 90 |
| 边 |  | WB | 98 | 435 | 45 | 578 | 96 | 441 | 45 | 582 | －2 | 6 | 0 | 4 | 1\％ | WB | 31 | 17 | 14 | c | B | B | 19 | B |  |  | WB | 34 | 233 | 34 | 233 | 34 | 233 |
| io |  | SB | 95 | 132 | 55 | 282 | 97 | 131 | 52 | 280 | 2 | －1 | －3 | －2 | －1\％ | SB | 14 | 14 | 9 | B | B | A | 13 | B |  |  | SB | 13 | 134 | 13 | 134 | 13 | 134 |
|  |  | EB | 55 | 440 | 100 | 595 | 59 | 452 | 98 | 609 | 4 | 12 | －2 | 14 | 2\％ | EB | 21 | 11 | 7 | C | B | A | 11 | B |  |  | EB | 23 | 159 | 23 | 159 | 23 | 159 |
|  | CSAH 81 \＆ | NB | 115 | 405 | 10 | 530 | 107 | 410 | 11 | 528 | －8 | 5 | 1 | －2 | 0\％ | NB | 46 | 19 | 6 | D | в | A | 24 | c | 27 | C | NB | 37 | 155 | 38 | 155 | 36 | 155 |
|  |  | WB | 70 | 350 | 65 | 485 | 68 | 365 | 64 | 497 | －2 | 15 | －1 | 12 | 2\％ | WB | 51 | 30 | 24 | D | C | C | 32 | C |  |  | WB | 45 | 198 | 46 | 198 | 44 | 198 |
| $\stackrel{\text { in }}{ }$ |  | SB | 50 | 770 | 113 | 933 | 52 | 768 | 113 | 933 | 2 | －2 | 0 | 0 | 0\％ | SB | 52 | 26 | 14 | D | C | B | 26 | C |  |  | SB | 71 | 429 | 72 | 429 | 70 | 429 |
|  |  | EB | 65 | 235 | 275 | 575 | 63 | 250 | 278 | 591 | －2 | 15 | 3 | 16 | 3\％ | EB | 50 | 31 | 14 | D | c | B | 25 | c |  |  | EB | 35 | 210 | 36 | 210 | 34 | 210 |
|  | RR Xing <br> 42nd Avenue |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3 | A | NB |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB |  |  |  | 6 | 111 |  |  |
| $\left\|\begin{array}{l} \dot{u} \mid \\ \stackrel{\rightharpoonup}{n} \end{array}\right\|$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | EB |  |  |  | 9 | 96 |  |  |

## *GROWTH SCENARIO* 2030 PM BRT



Memorandum

To: Ed Hunter, Project Manager Interchange Project office

From: JoNette Kuhnau, P.E., PTOE

Thru: Debra Brisk, P.E.

Date: August 22, 2011

## Subject: The Interchange <br> 2030 Build Analysis Update - Traffic Technical Memorandum \#3

The purpose of this memorandum is to update the traffic analysis completed for the 2030 Build scenario of The Interchange project. This memorandum documents the Phase 1 build-out for The Interchange site as documented in the Environmental Assessment, and is an update to Traffic Technical Memorandum \#2 dated March 7, 2011.

## Traffic Forecasts

The same assumptions relative to background traffic, opening and design years, pedestrian volumes, and transit operations, as previously documented in Traffic Technical Memorandum \#1 and \#2, have been retained in this analysis. The previous 2030 analysis evaluated scenarios with either Southwest LRT or Bottineau LRT grade separated, or both grade separated. This analysis assumes the same geometrics as Option 1-BT, with Bottineau LRT operating at-grade through the TH $55 / 7^{\text {th }}$ Street $/ 6^{\text {th }}$ Avenue intersection, Southwest LRT grade separated at the TH $55 / 7^{\text {th }}$ Street $/ 6^{\text {th }}$ Avenue intersection, and both LRT lines grade separated at the HERC driveway.

The only differences between the updated 2030 Build analysis and the previous 2030 Option 1-BT analysis are as follows:

- The maximum number of structured parking spaces on The Interchange site is assumed to be 425 (increased from 350).
- The lower level of the parking ramp (up to 244 spaces) is restricted to only enter/exit at the $5^{\text {th }}$ Street $\mathrm{N} / 5^{\text {th }}$ Avenue N intersection.
- The upper level of the parking ramp (up to 108 spaces) is restricted to only enter/exit onto $6^{\text {th }}$ Avenue N .
- Parking is assumed to be open for public use. Previously all parking was assumed to be leased to known tenants near The Interchange site. While it has not been determined at this time how the parking will be used, public parking is a possibility that is being considered, therefore this analysis documents the expected worst case of traffic impacts if the public parking is allowed.

The modified assumptions regarding the parking on The Interchange site result in changes to the forecast volumes entering and exiting the site during peak hours, as summarized in Table $\mathbf{1}$ below. The trips produced by the 425 parking spaces on The Interchange site were estimated assuming that the all spaces are fully occupied and the percentage of spaces entering/exiting the site during peak hours was based on Chapter 18 of the Institute of Transportation Engineers (ITE) Transportation Planning Handbook $3^{\text {rd }}$ Edition. The public parking and increased number of spaces results in greater trip generation during the peak hours, in both directions (entering and exiting).

Table 1. 2030 Traffic Analysis - Parking Assumptions

|  |  | 3/2011 Analysis | 8/2011 Analysis |
| :--- | :--- | :---: | :---: |
| Parking Spaces | 350 | 425 |  |
| Use | Office <br> (No Public <br> Parking) | Open to Public <br> Use |  |
|  | Upper Level - <br> access 5th St only <br> Lower Level - <br> access 6th Ave <br> only |  |  |
|  | In | 140 | 180 |
|  | Out | 20 | 70 |
| PM Traffic | In | 20 | 60 |
|  | Out | 120 | 190 |

## Infrastructure and Operations

The access points to the parking on The Interchange site are still proposed to be located at the south leg of the $5^{\text {th }}$ Street $N / 5^{\text {th }}$ Avenue $N$ intersection and at a new driveway located on $6^{\text {th }}$ Avenue N between the HERC entrance and $5^{\text {th }}$ Street $N$. The parking spaces by themselves do not generate new trips (i.e., land uses generate trips, not parking), but will redistribute existing vehicle and pedestrian trips into The Interchange modeling area. The same assumptions regarding the reconfiguration of the $5^{\text {th }}$ Street $\mathrm{N} / 6^{\text {th }}$ Avenue N intersection were also used.

The level of service analysis included all ten intersections from the previous 2030 Build analysis of Option 1-BT, although the only significant changes in volume were at the $6{ }^{\text {th }}$ Avenue N/HERC driveway, $6^{\text {th }}$ Avenue $N /$ Interchange driveway, $6{ }^{\text {th }}$ Avenue $N / 5^{\text {th }}$ Street $N$, and $5^{\text {th }}$ Street $N / 5^{\text {th }}$ Avenue $N$ intersections.

## Modeling Results

The VISSIM modeling results reported in this memorandum are based on the average of five one-hour runs for each peak hour in each scenario. Within the peak hour, the 15-minute forecast flows were input into the model to accurately reflect the variations in traffic during the overall peak hour. Based on the counts, 7:30-8:30 AM was modeled as the AM peak hour and 4:45-5:45 PM was modeled as the PM peak hour.

## 2030 Build - Option 1-BT + Parking

In this option, Southwest LRT was assumed to cross $7^{\text {th }}$ Street $N$ above grade, and Bottineau LRT was assumed to be at-grade along TH 55 through the TH 55/7 ${ }^{\text {th }}$ Street $N / 6^{\text {th }}$ Avenue $N$ intersection. Within the TH 55/7 ${ }^{\text {th }}$ Street $N$ intersection, Bottineau LRT was assumed to transition from a center running alignment to a side-running alignment (south side of $6^{\text {th }}$ Avenue N ). The TH 55/7 ${ }^{\text {th }}$ Street N and TH 55/Border Avenue/Oak Lake Avenue N signals are assumed to utilize TSP operation (no gates or preemption).

The AM and PM peak hour results for this option are shown in Table 2 and Table 3, respectively, on the following pages. The peak hour queuing results for the primary intersections around The Interchange site are shown in Table 4 and Table5.

The LOS operations shown for the parking exit and $5{ }^{\text {th }}$ Avenue N approach at the $5^{\text {th }}$ Street N intersection in the AM and PM peak hours is primarily due to the low volumes on those approaches. The signal is assumed to operate with a 110-second cycle, therefore most vehicles on the minor approaches will arrive during the red phase and the average delay will be greater than 55 seconds. However, the relatively short $95^{\text {th }}$ percentile queues on both the parking access and $5^{\text {th }}$ Avenue N approaches show that the LOS E operations do not represent an operational deficiency.

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Table 2. 2030 Build Option 1-BT + Parking: AM Peak Hour Level of Service Results

| Intersection | Control | Approach | Operations by Movement |  |  |  |  |  | Overall Intersection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left |  | Through |  | Right |  | Delay (sec/veh) | LOS |
|  |  |  | Delay (sec/veh) | LOS | Delay (sec/veh) | LOS | Delay (sec/veh) | LOS |  |  |
| $\begin{aligned} & \text { TH } 55 / \\ & \text { 7th Street N/ } \\ & \text { 6th Avenue N } \end{aligned}$ | Signal | NB | 63.5 | E | 68.2 | E | 73.5 | E | 56.1 | E |
|  |  | EB | 0.0 | A | 38.5 | D | 16.0 | B |  |  |
|  |  | SB | 92.5 | F | 89.1 | F | 94.9 | F |  |  |
|  |  | WB | 372.2 | F | 24.0 | C | 8.1 | A |  |  |
| 6th Avenue N/ Bradford Avenue/ HERC Entrance | Signal | NB | 75.1 | E | 0.0 | A | 64.7 | E | 9.7 | A |
|  |  | EB | 8.2 | A | 4.2 | A | 9.7 | A |  |  |
|  |  | SB | 65.7 | E | 0.0 | A | 68.3 | E |  |  |
|  |  | WB | 44.4 | D | 15.5 | B | 6.8 | A |  |  |
| 6th Avenue N/ 5th Street N | Signal | NB | 46.3 | D | 42.8 | D | 46.0 | D | 15.2 | B |
|  |  | EB | 5.7 | A | 5.7 | A | 1.2 | A |  |  |
|  |  | SB | 47.0 | D | 49.6 | D | 54.5 | D |  |  |
|  |  | WB | 8.6 | A | 10.8 | B | 9.6 | A |  |  |
| 5th Street N/ 5th Avenue N | Signal | NB | 8.0 | A | 6.8 | A | 5.7 | A | 11.6 | B |
|  |  | EB | 65.6 | E | 0.0 | A | 62.6 | E |  |  |
|  |  | SB | 4.6 | A | 3.6 | A | 3.1 | A |  |  |
|  |  | WB | 64.6 | E | 0.0 | A | 61.9 | E |  |  |
| 6th Avenue N/ Interchange Driveway | Driveway Stop Control | NB | 3.4 | A | 0.0 | A | 0.7 | A | 0.8 | A |
|  |  | EB | 0.0 | A | 1.6 | A | 1.4 | A |  |  |
|  |  | SB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | WB | 1.5 | A | 2.5 | A | 0.0 | A |  |  |
| TH 55/ <br> Border Avenue/ Oak Lake Avenue | Signal | NB | 57.5 | E | 53.5 | D | 18.2 | B | 25.4 | C |
|  |  | EB | 70.1 | E | 19.1 | B | 14.8 | B |  |  |
|  |  | SB | 84.4 | F | 71.5 | E | 38.5 | D |  |  |
|  |  | WB | 76.6 | E | 5.4 | A | 0.8 | A |  |  |
| 5th Street N/ 3rd Avenue N | Signal | NB | 51.1 | D | 17.5 | B | 0.0 | A | 22.3 | C |
|  |  | EB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | SB | 0.0 | A | 28.4 | C | 28.1 | C |  |  |
|  |  | WB | 27.1 | C | 25.3 | C | 9.3 | A |  |  |
| 7th Street N/ 5th Avenue N | Signal | NB | 22.7 | C | 9.9 | A | 0.0 | A | 8.6 | A |
|  |  | EB | 51.8 | D | 0.0 | A | 29.9 | C |  |  |
|  |  | SB | 0.0 | A | 4.5 | A | 8.7 | A |  |  |
|  |  | WB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
| Royalston Ave N/ 5th Avenue N | Signal | NB | 0.0 | A | 36.9 | D | 16.2 | B | 27.6 | C |
|  |  | EB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | SB | 36.0 | D | 34.1 | C | 0.0 | 0 |  |  |
|  |  | WB | 27.7 | C | 0.0 | A | 14.4 | B |  |  |
| 7th Street N/ Oak Lake Avenue N | Signal | NB | 49.3 | D | 47.2 | D | 0.0 | A | 12.5 | B |
|  |  | EB | 63.8 | E | 67.4 | E | 17.7 | B |  |  |
|  |  | SB | 58.3 | E | 28.2 | C | 25.6 | C |  |  |
|  |  | WB | 0.0 | A | 11.9 | B | 10.9 | B |  |  |

## Ihe Interchange

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Table 3. 2030 Build Option 1-BT+ Parking: PM Peak Hour Level of Service Results

| Intersection | Control | Approach | Operations by Movement |  |  |  |  |  | Overall Intersection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left |  | Through |  | Right |  | Delay (sec/veh) | LOS |
|  |  |  | Delay (sec/veh) | LOS | Delay (sec/veh) | LOS | Delay (sec/veh) | LOS |  |  |
| TH 55/ <br> 7th Street N/ <br> 6th Avenue N | Signal | NB | 46.3 | D | 49.3 | D | 54.2 | D | 46.0 | D |
|  |  | EB | 0.0 | A | 41.9 | D | 10.9 | B |  |  |
|  |  | SB | 56.2 | E | 56.5 | E | 52.9 | D |  |  |
|  |  | WB | 166.8 | F | 42.5 | D | 20.2 | C |  |  |
| 6th Avenue N/ Bradford Avenue/ HERC Entrance | Signal | NB | 49.4 | D | 0.0 | A | 50.3 | D | 8.5 | A |
|  |  | EB | 7.5 | A | 6.4 | A | 8.0 | A |  |  |
|  |  | SB | 53.9 | D | 0.0 | A | 12.4 | B |  |  |
|  |  | WB | 9.3 | A | 7.4 | A | 6.8 | A |  |  |
| 6th Avenue N/ 5th Street N | Signal | NB | 50.8 | D | 48.6 | D | 51.6 | D | 28.7 | C |
|  |  | EB | 25.9 | C | 23.3 | C | 3.0 | A |  |  |
|  |  | SB | 48.0 | D | 44.1 | D | 51.8 | D |  |  |
|  |  | WB | 17.7 | B | 19.3 | B | 15.5 | B |  |  |
| 5th Street N/ 5th Avenue N | Signal | NB | 15.8 | B | 17.1 | B | 12.7 | B | 24.7 | C |
|  |  | EB | 60.2 | E | 0.0 | A | 55.2 | E |  |  |
|  |  | SB | 13.1 | B | 10.9 | B | 6.6 | A |  |  |
|  |  | WB | 49.6 | D | 0.0 | A | 50.6 | D |  |  |
| 6th Avenue N/ Interchange Driveway | Driveway Stop Control | NB | 3.1 | A | 0.0 | A | 1.9 | A | 0.4 | A |
|  |  | EB | 0.0 | A | 2.9 | A | 1.1 | A |  |  |
|  |  | SB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | WB | 1.2 | A | 0.3 | A | 0.0 | A |  |  |
| TH 55/ <br> Border Avenue/ <br> Oak Lake Avenue | Signal | NB | 49.3 | D | 44.2 | D | 23.2 | C | 19.6 | B |
|  |  | EB | 64.8 | E | 8.1 | A | 3.2 | A |  |  |
|  |  | SB | 54.3 | D | 58.2 | E | 35.6 | D |  |  |
|  |  | WB | 58.9 | E | 11.9 | B | 4.3 | A |  |  |
| 5th Street N/ 3rd Avenue N | Signal | NB | 65.2 | E | 20.0 | B | 0.0 | A | 28.0 | C |
|  |  | EB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | SB | 0.0 | A | 30.7 | C | 26.4 | C |  |  |
|  |  | WB | 41.9 | D | 37.4 | D | 19.1 | B |  |  |
| 7th Street N/ 5th Avenue N | Signal | NB | 19.7 | B | 13.6 | B | 0.0 | A | 16.3 | B |
|  |  | EB | 39.0 | D | 0.0 | A | 25.5 | C |  |  |
|  |  | SB | 0.0 | A | 13.0 | B | 14.4 | B |  |  |
|  |  | WB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
| Royalston Ave N/ 5th Avenue N | Signal | NB | 0.0 | A | 45.4 | D | 32.7 | C | 37.2 | D |
|  |  | EB | 0.0 | A | 0.0 | A | 0.0 | A |  |  |
|  |  | SB | 41.4 | D | 39.9 | D | 0.0 | A |  |  |
|  |  | WB | 30.1 | C | 0.0 | A | 13.4 | B |  |  |
| 7th Street N/ Oak Lake Avenue N | Signal | NB | 72.0 | E | 57.0 | E | 0.0 | A | 21.7 | C |
|  |  | EB | 12.1 | B | 6.8 | A | 2.5 | A |  |  |
|  |  | SB | 43.4 | D | 40.4 | D | 44.0 | D |  |  |
|  |  | WB | 0.0 | A | 10.4 | B | 11.6 | B |  |  |

Table 4. 2030 Build Option 1-BT+ Parking: AM Peak Hour $95^{\text {th }}$ Percentile Queue Results

| Intersection | Control | Approach | 95th Percentile Queue |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | 95th \% <br> Queue | Available Storage |
| 6th Avenue N/ Bradford Avenue/ HERC Entrance | Signal | NB | 59 | 280 |
|  |  | EB | 83 | 310 |
|  |  | SB | 179 | 390 |
|  |  | WB | 58 | 240 |
| 6th Avenue N/ <br> 5th Street N | Signal | NB | 83 | 150 |
|  |  | EB | 106 | 550 |
|  |  | SB | 65 | 350 |
|  |  | WB | 202 | 330 |
| 5th Street N/ 5th Avenue N | Signal | NB | 12 | 740 |
|  |  | EB | 5 | 50 |
|  |  | SB | 25 | 330 |
|  |  | WB | 8 | 410 |
| 6th Avenue N/ Interchange Driveway | Driveway Stop Control | NB | 55 | 60 |
|  |  | EB | 45 | 240 |
|  |  | SB | - | - |
|  |  | WB | 34 | 150 |

Table 5. 2030 Build Option 1-BT+ Parking: PM Peak Hour $95^{\text {th }}$ Percentile Queue Results

| Intersection | Control | Approach | 95th Percentile Queue |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | 95th \% Queue | Available Storage |
| 6th Avenue N/ Bradford Avenue/ HERC Entrance | Signal | NB | 79 | 280 |
|  |  | EB | 68 | 310 |
|  |  | SB | 42 | 390 |
|  |  | WB | 106 | 240 |
| 6th Avenue N/ <br> 5th Street N | Signal | NB | 210 | 150 |
|  |  | EB | 123 | 550 |
|  |  | SB | 185 | 350 |
|  |  | WB | 339 | 330 |
| 5th Street N/ 5th Avenue N | Signal | NB | 25 | 740 |
|  |  | EB | 16 | 50 |
|  |  | SB | 51 | 330 |
|  |  | WB | 27 | 410 |
| 6th Avenue N/ Interchange Driveway | Driveway Stop Control | NB | 86 | 60 |
|  |  | EB | 46 | 240 |
|  |  | SB | - | - |
|  |  | WB | 11 | 150 |

In the AM peak hour, none of the $95^{\text {th }}$ percentile queues are expected to exceed the available storage. In the PM peak hour, the $95^{\text {th }}$ percentile queue on the northbound approach of the $6^{\text {th }}$ Avenue $\mathrm{N} / 5^{\text {th }}$ Street

N is expected to exceed the distance between the signal and The Interchange driveway by approximately 3 vehicles ( 60 feet). The $95^{\text {th }}$ percentile queue represents the longest queue that would ever be expected to occur and in the PM peak hour simulation, the northbound queue was not observed to impact or delay vehicles exiting or entering The Interchange site. This is confirmed by the expected LOS A operations for all movements at the driveway. The $95^{\text {th }}$ percentile queue reported for The Interchange exit onto $6^{\text {th }}$ Avenue was 86 feet, or approximately four vehicles. If the northbound queue at the $6^{\text {th }}$ Avenue $N / 5^{\text {th }}$ Street $N$ intersection became an operational of safety issue during the PM peak period, The Interchange access could be limited to right-in/right-out using signed restrictions for peak periods only or the driveway could be modified to prevent left-turn movements.

## Summary and Conclusions

The 2030 peak hour operations of The Interchange, including updated parking assumptions, were modeled in VISSIM to determine the expected traffic impact. For this analysis, only the scenario with Bottineau LRT at-grade at $7^{\text {th }}$ Street $N$, Southwest LRT grade separated at $7^{\text {th }}$ Street $N$, and both LRT lines grade separated at the HERC driveway was analyzed. Based on the 2030 peak hour traffic and LRT modeling results of Options 1-BT+Parking, the following conclusions are made:

- The intersections adjacent to The Interchange are expected to have acceptable operations with up to 425 parking spaces created on the site, whether the parking is leased or open to the public, which is the worst case scenario in terms of traffic.
- Queuing at the intersections adjacent to The Interchange during peak hours is not expected to impact access in and out of The Interchange site. If the northbound queue on $6^{\text {th }}$ Avenue N at the $5^{\text {th }}$ Street N intersection becomes an issue at The Interchange access during the PM peak hour, the driveway could be limited to right-in/right-out using signed restrictions for the PM peak period or the driveway could be modified to restrict left-turn movements.


## Memorandum

To: Brent Rusco, P.E.<br>Hennepin County Engineering and Transit Planning<br>From: JoNette Kuhnau, P.E., PTOE<br>Sarah Ott, EIT<br>Kimley-Horn and Associates, Inc.

Date: September 19, 2011
Subject: Bottineau Segment D2
Traffic Analysis Memorandum
This memorandum presents the results of the scoping level traffic operations analysis for the D2 alignment of the Bottineau Transitway within the City of Minneapolis. This document is intended for review and consideration by the technical staff of the Minneapolis Focused Issue Group (FIG), and is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor.

The following sections focus on the traffic operational aspects of the D2 alignment, including a summary of the existing conditions, the alternative D2 alignments, forecast 2030 traffic volumes, and the 2030 operational modeling results for light rail (LRT) transit. Additional considerations including noise analysis, parking impacts, pedestrian/bicycle circulation, property access, roadway network impacts, and emergency vehicle access, are important issues to be considered for the D2 alignment, but are not part of the analysis documented in this memorandum.

## EXISTING CONDITIONS

The D2 segment for this analysis extends from Trunk Highway (TH) 55 to Lowry Avenue in Minneapolis along Penn Avenue (County State Aid Highway (CSAH) 2)/ Oliver Avenue and W Broadway Avenue (CSAH 81). Figure 1 shows the existing daily and PM peak hour traffic volumes for these arterials. The focus of the operations analysis was the PM peak hour because it has the highest volumes and would therefore represent a worst case condition. The existing daily and peak hour volumes were collected by the City of Minneapolis in 2009.

The D2 LRT alignment may impact traffic operations at signalized intersections because of the potential for additional delay. Traffic signals are currently operating at the following intersections:

- W Broadway Avenue (CSAH 81) at $29^{\text {th }}$ Avenue
- W Broadway Avenue (CSAH 81) at $26^{\text {th }}$ Avenue
- W Broadway Avenue (CSAH 81) at Penn Avenue (CSAH 2)/ McNair Avenue
- Penn Avenue (CSAH 2) at Golden Valley Road
- Penn Avenue (CSAH 2) at $16^{\text {th }}$ Avenue
- Penn Avenue (CSAH 2) at $14^{\text {th }}$ Avenue
- Penn Avenue (CSAH 2) at Plymouth Avenue
- Penn Avenue (CSAH 2) at $12^{\text {th }}$ Avenue
- Penn Avenue (CSAH 2) at Oak Park Avenue
- TH 55 at Penn Avenue (CSAH 2)

Existing peak hour parking restrictions are located on W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2). W Broadway Avenue (CSAH 81) is signed as NO PARKING 7 AM-9 AM MON-FRI on the west side of the roadway and NO PARKING 4 PM-6 PM MON-FRI on the east side of the roadway for the length of the study corridor. Penn Avenue (CSAH 2) is signed NO PARKING 4 PM-6 PM MON-FRI on both sides of the street north of W Broadway Avenue (CSAH 81) and the east side of the street south of W Broadway Avenue (CSAH 81). Parking is also currently restricted on Penn Avenue (CSAH 2) at bus stops, which are generally located at the near side of intersections.

## Existing Traffic Volumes

The existing (2009-2010) daily and peak hour traffic volumes on Broadway Avenue (CSAH 81), Penn Avenue (CSAH 2), and Oliver Avenue are shown in Table 1. In general, the daily directional volumes are approximately equal on W Broadway Avenue (CSAH 81), but the daily volumes on Penn Avenue (CSAH 2) and TH 55 are heavier in the northbound and eastbound directions. These are also the dominant traffic flows during the PM peak hour. On W Broadway Avenue (CSAH 81), the primary traffic flow is westbound.

BottineauTransitway
DRAFT ENVIRONMENTAL IMPACT STATEMENT
Table 1. Existing Daily and Peak Hour Traffic Volumes

| Roadway Segment | Time Period | Broadway Avenue <br> (CSAH 81) |  |  | Penn Avenue (CSAH 2) |  |  | Oliver Avenue |  |  | TH 55 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | EB | WB | Total | SB | NB | Total | SB | NB | Total | EB | WB |
| Lowry Avenue (CSAH 153) to Penn Avenue (CSAH 2) | Daily | 13,100 | 6,400 | 6,700 |  |  |  |  |  |  |  |  |  |
|  | PM Peak Hour | 1,171 | 470 | 701 |  |  |  |  |  |  |  |  |  |
| Penn Avenue (CSAH 2) to Fremont Avenue | Daily | 15,300 | 7,900 | 7,400 |  |  |  |  |  |  |  |  |  |
|  | PM Peak Hour | 1,161 | 522 | 639 |  |  |  |  |  |  |  |  |  |
| W Broadway Ave (CSAH 81) to Golden Valley Rd | Daily |  |  |  | 11,300 | 5,300 | 6,000 | 340 | 150 | 190 |  |  |  |
|  | PM Peak Hour |  |  |  | 936 | 403 | 533 | 33 | 15 | 19 |  |  |  |
| Golden Valley Rd to Plymouth Ave | Daily |  |  |  | 12,500 | 5,900 | 6,600 | 360 | 360 | 0 |  |  |  |
|  | PM Peak Hour |  |  |  | 1,069 | 419 | 650 | 27 | 27 | 0 |  |  |  |
| Plymouth Ave to TH 55 | Daily |  |  |  | 10,700 | 4,900 | 5,800 | 280 | 280 | 0 |  |  |  |
|  | PM Peak Hour |  |  |  | 888 | 385 | 503 | 29 | 29 | 0 |  |  |  |
| West City Limits to Penn Avenue (CSAH 2) | Daily |  |  |  |  |  |  |  |  |  | 22,700 | 14,100 | 8,600 |
|  | PM Peak Hour |  |  |  |  |  |  |  |  |  | 2,725 | 1,534 | 1,191 |
| Penn Avenue (CSAH 2) to Bryant Avenue | Daily |  |  |  |  |  |  |  |  |  | 25,700 | 15,100 | $\begin{gathered} 10,60 \\ 0 \end{gathered}$ |
|  | PM Peak Hour |  |  |  |  |  |  |  |  |  | 3,036 | 1,658 | 1,378 |

DRAFT ENVIRONMENTAL IMPACT STATEMENT

## Operations Analysis

All operational analysis for the D2 segment was completed in Synchro/SimTraffic because it provides the ability to efficiently analyze a number of different geometric and operational parameters. In addition, transit is assumed to operate in a priority scheme under all the future operations scenarios, therefore Synchro/ SimTraffic can be used to analyze these type of operations because the LRT will not preempt the traffic signals and will generally operate concurrently with the through vehicle phases or as its own signal phase that will be called only when a transit vehicle is detected. In a priority scheme, it is important to note that the early green or green extension time is typically considered to be about 10 percent of the cycle, and therefore the transit vehicle still may have to stop at the signal.

The existing/ proposed signals at Penn Avenue (CSAH 2)/ $16^{\text {th }}$ Avenue and Penn Avenue (CSAH 2)/ Oak Park Avenue and the existing signals at Penn Avenue (CSAH 2)/ $14^{\text {th }}$ Avenue and Penn Avenue (CSAH 2)/ $12^{\text {th }}$ Avenue were excluded from the existing and future operations analysis. These cross streets generally have very low traffic volumes (less than 60 approach vehicles per direction in the peak hour, except westbound $14^{\text {th }}$ Avenue, which had 90 vehicles in the peak hour) and therefore are not expected to be significantly impacted by the D2 alternatives.

Existing Operations
The existing conditions PM peak hour analysis shows that W Broadway Avenue (CSAH 81) has adequate capacity to accommodate peak hour volumes and intersection levels of service (LOS) are all LOS C or better at the $29^{\text {th }}$ Avenue, $26^{\text {th }}$ Avenue, and Penn Avenue (CSAH 2) intersections. Table 2 shows the results from the existing conditions analysis.

DRAFT ENVIRONMENTAL IMPACT STATEMENT

Table 2. Existing Delay and LOS Results

| Intersection | Intersection |  | Comments |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/ veh) | LOS |  |
| W Broadway Ave <br> (CSAH 81) \& 29th <br> Ave | 4.0 | A | All movements operate at LOS B or <br> better |
| W Broadway Ave <br> (CSAH 81) \& 26th <br> Ave | 9.3 | A | All movements operate at LOS C or <br> better |
| W Broadway Ave <br> (CSAH 81) \& Penn <br> Ave (CSAH 2) - | 19.6 | B | All movements operate at LOS C or <br> better |
| McNair Avenue <br> approach assumed <br> closed |  |  |  |
| Penn Ave (CSAH 2) <br> \& Golden Valley Rd | 17.3 | B | All movements operate at LOS C or <br> better |
| Penn Ave (CSAH 2) <br> \& Plymouth Ave | 22.1 | C | All movements operate at LOS D or <br> better |
| TH 55 \& Penn Ave <br> (CSAH 2) - with <br> N/ S split phasing | 80.2 | F | All movements operate at LOS E or <br> F except eastbound through |
| TH 55 \& Penn Ave <br> (CSAH 2) - without <br> N/ S split phasing | 34.4 | C | Southbound left-turns operate at <br> LOS F, northbound and eastbound <br> left-turns operate at LOS E |

The intersection of W Broadway Avenue (CSAH 81) at Penn Avenue (CSAH 2)/ McNair Avenue was modeled without McNair Avenue because the traffic volumes on McNair Avenue are very low, and the approach requires an additional signal phase that would increase delay for the other movements. The McNair Avenue approach is closed at the W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) intersection for alternatives D2-A and D2-C, and could be closed for D2-B; therefore, disregarding McNair Avenue in the existing model allows for a fair comparison between the existing and 2030 D2 alternative models.

On Penn Avenue (CSAH 2) at Golden Valley Road, Plymouth Avenue, and TH 55, a short 50 -foot left-turn lane was incorporated into the model to represent the ability for through vehicles to bypass a waiting left-turn vehicle. Although there are no designated left-turn lanes, based on field observations this geometry more accurately represents actual operations.

DRAFT ENVIRONMENTAL IMPACT STATEMENT

The signal at TH 55/ Penn Avenue (CSAH 2) currently operates with split phasing because of the wide median and geometrics that cause the paths of the northbound and southbound left-turn movements to overlap. The split phasing could be eliminated if geometric changes were made to allow the left-turn movements to operate concurrently. Split phasing reduces the available green time for the Penn Avenue (CSAH 2) movements, and therefore increases the delay significantly. Field observations indicated that the existing PM peak hour queues on Penn Avenue (CSAH 2) frequently extend more than one block and vehicles wait more than one signal cycle to clear the intersection. The intersection was modeled with and without the split phasing for the existing conditions to provide a more accurate comparison with the 2030 D2 alternative operational results.

Oliver Avenue is a local street and is stop-controlled at major intersections. There are currently no direct vehicle connections from Oliver Avenue to Broadway Avenue (CSAH 81) or to TH 55. In addition, Oliver Avenue has the following unique operational and traffic flow characteristics:

- One-way southbound traffic flow from Golden Valley Road to Plymouth Avenue
- Roadway is discontinuous between Plymouth Avenue and $12^{1 ⁄ 2}$ Avenue (i.e., no through traffic in this block)
- One-way southbound traffic flow from $12^{½}$ Avenue to TH 55 North Frontage Road

As a result of the roadway being severed and also the one-way traffic flow, existing traffic volumes on Oliver Avenue are very low ( $<400$ vehicles per day). Therefore, Oliver Avenue was not analyzed in the existing conditions.

## FUTURE CONDITIONS

Along the W Broadway Avenue (CSAH 81) segment of the D2 alignment, access to several cross streets will be modified at the following unsignalized intersections:

- $30^{\text {th }}$ Avenue (right-in/right-out access on east side only)
- $27^{\text {th }}$ Avenue (right-in/right-out on west side only; eliminate connection on east side)
- Queen Avenue (dead end on east side only)
- $24^{\text {th }}$ Avenue (dead end on west side only)

The D2 segment alternative includes three alignment sub-options along Penn Avenue (CSAH 2) and Oliver Avenue: D2-A, D2-B, and D2-C. All three alternatives operate in the median of TH 55 , turn north on either Penn Avenue (CSAH 2) or Oliver Avenue, and then turn northwest to operate in a center median on W Broadway Avenue (CSAH 81). Each of the alternatives is described in greater detail in the following:

- Alternative D2-A

On Alternative D2-A, the transitway operates on the east side of Penn Avenue (CSAH 2) and then crosses diagonally through the W Broadway Avenue (CSAH 81)/Penn Avenue (CSAH 2) intersection. Due to the limited right-of-way, all southbound vehicular traffic continues to operate on Penn Avenue (CSAH 2), but all northbound vehicular traffic is moved to Oliver Avenue. In this alternative, northbound traffic rejoins Penn Avenue (CSAH 2) at 23rd Avenue, just south of W Broadway Avenue (CSAH 81). All existing on-street parking on Penn Avenue (CSAH 2) would be eliminated in this alternative. One side parking was assumed to remain on Oliver Avenue based on a roadway width of 28 to 29 feet, which is wide enough for one driving lane and one parking lane.

Under alignment D2-A, traffic signals are assumed to be added at the following intersections:

- TH 55 and Oliver Avenue
- Oliver Avenue and Oak Park Avenue
- Oliver Avenue and Plymouth Avenue
- Oliver Avenue and $16^{\text {th }}$ Avenue
- Oliver Avenue and Golden Valley Road

On Penn Avenue (CSAH 2), the existing signals at $12^{\text {th }}$ Avenue and $14^{\text {th }}$ Avenue are assumed to be removed. All other existing signals were assumed to continue to operate under this alternative.

The D2-A alternative creates a one-way pair between Penn Avenue (CSAH 2) and Oliver Avenue and would likely necessitate changes in jurisdiction and functional class. Oliver Avenue would need to be reopened between Plymouth Avenue and $121 / 2$ Avenue. Several cross streets would be severed at the east side of Penn Avenue (CSAH 2) due to the need to prevent crossings of the LRT alignment at the following unsignalized intersections:

- 21 st Avenue
- $17^{\text {th }}$ Avenue
- $15^{\text {th }}$ Avenue
- $14^{\text {th }}$ Avenue
- $12^{\text {th }}$ Avenue
- $8^{\text {th }}$ Avenue

At the dead end sections, additional parking is proposed to be provided to replace the loss of the on-street parking.

## - Alternative D2-B

Alternative D2-B turns north on Oliver Avenue from TH 55 to W Broadway Avenue (CSAH 81), where it crosses the W Broadway Avenue (CSAH 81)/ Oliver Avenue intersection diagonally at a new signalized intersection. The D2-B alternative removes all vehicular traffic from Oliver Avenue to accommodate the transitway, and the vehicular traffic was assumed to relocate to Penn Avenue (CSAH 2). The existing volumes on Oliver Avenue are extremely low (less than 500 vehicles per day), therefore this does not result in significant changes in traffic patterns. Penn Avenue (CSAH 2) would remain open to two-way traffic, with signals at the existing locations except for $12^{\text {th }}$ Avenue and $14^{\text {th }}$ Avenue, which were assumed to be removed. All existing on-street parking on Oliver Avenue would be eliminated in this alternative.

Due to the need to control crossings of the transitway, traffic signals were assumed to be added at the following intersections:

- TH 55 and Oliver Avenue
- Oliver Avenue and Oak Park Avenue
- Oliver Avenue and Plymouth Avenue
- Oliver Avenue and $16^{\text {th }}$ Avenue
- Oliver Avenue and Golden Valley Road
- W Broadway Avenue (CSAH 81) and Oliver Avenue

The signalized intersection at W Broadway (CSAH 81)/ Oliver Avenue would include a diagonal crossing of the transitway through the intersection, which would require its own signal phase.
Several cross streets would be severed on both sides of Oliver Avenue due to the need to prevent crossings of the LRT alignment at the following unsignalized intersections:

```
- 23rd Avenue (cul-de-sac at west side only)
- 21st Avenue
- 18th Avenue (east side only)
- 17th Avenue
- 15th Avenue
- 14th Avenue
- 12th Avenue
- 8th Avenue
```

At the dead end sections, additional parking is proposed to be provided to replace the loss of the on-street parking.

- Alternative D2-C

Alternative D2-C assumes the transitway is center-running along Penn Avenue (CSAH 2), similar to alternative D2-A, except that two-way traffic is maintained on Penn Avenue (CSAH 2). This would require that all properties on one side of Penn Avenue (CSAH 2) would need to be acquired in order to provide adequate right-of-way. All existing on-street parking on Penn Avenue (CSAH 2) would be eliminated in this alternative.

Similar to alternative D2-A, the existing signals at $14^{\text {th }}$ Avenue and $12^{\text {th }}$ Avenue are assumed to be removed. However, no additional traffic signals would be needed on Oliver Avenue.
Several cross streets would become right-in/right-out on both sides of Penn Avenue (CSAH 2) due to the need to prevent crossings of the LRT alignment at the following unsignalized intersections:

- McNair Avenue (west side only)
- 23rd Avenue
- $21^{\text {st }}$ Avenue

```
- 17th Avenue
- 15th Avenue (east side only)
- 14th Avenue
- 12th Avenue
- 8th Avenue
```


## 2030 Forecast Traffic Volumes

The existing and daily peak hour turning movement counts were used to forecast 2030 traffic volumes for the study area. The McNair Avenue approach was removed from the W Broadway Avenue (CSAH 81)/Penn Avenue (CSAH 2) intersection for all D2 alternatives because the additional phase would cause more delay to the intersection, which already experiences capacity and level of service (LOS) challenges. Left-turn movements were also assumed to be banned when an exclusive left-turn lane could not be provided along the transitway alignment (see the Future Operations section of this memorandum). Left-turn movements along the alignment were assumed to be prohibited at the following intersections:

- W Broadway Avenue (CSAH 81) and 29th Avenue (less than 50 total vehicles in the peak hour)
- W Broadway Avenue (CSAH 81) and $26^{\text {th }}$ Avenue (less than 75 total vehicles in the peak hour)
- W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2)
- Eastbound left-turn movements for all D2 alternatives (less than 25 vehicles in the peak hour)
- Westbound left-turn movements for alternative D2-B only (less than 100 vehicles in the peak hour)
Left-turn movements at signalized intersections on Penn Avenue are not assumed to be restricted at any locations under any of the alternatives, based on the provision of left-turn lanes.

The forecast 2030 volumes assume a one percent per year background growth rate, consistent with the Bottineau Transitway Alternatives Analysis "Traffic Operations Analysis Report" dated January 2010. These 2030 volumes were then reduced based on assumed traffic diversion due to the capacity constrictions on W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2), as well as the left-turn restrictions. The Metropolitan Council travel demand model was used to determine the magnitude of the expected diversion in 2030, which is forecast to be approximately 20 percent of through traffic along W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2). The travel demand model shows that approximately 500 tol,000 vehicles per day would be expected to divert from W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) to sections of TH 100, I-94, Lowry Avenue, and Penn Avenue (CSAH 2) north of W Broadway

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Avenue (CSAH 81). Figure 2 shows the reassignment of traffic volumes due to the Bottineau Transitway.

Table 3 shows the 2030 daily and PM peak hour traffic volumes for the Penn Avenue (CSAH 2) and Oliver Avenue corridors under each D2 alternative. The 2030 forecast volumes and lane geometry for alternatives D2-A, -B, and -C are shown in Figure 3, Figure 4, and Figure 5, respectively.

Table 3. 2030 Daily and PM Peak Hour Traffic Volumes

| Roadway Segment | Time Period | Alternative D2-A |  |  |  |  |  | Alternative D2-B |  |  |  |  | Alternative D2-C |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Penn Ave (CSAH 2) |  |  | Oliver Ave |  |  | Penn Ave (CSAH 2) |  |  | Oliver Ave |  | Penn Ave (CSAH 2) |  |  | Oliver Ave |  |  |
|  |  | $\begin{aligned} & \overline{\boxed{6}} \\ & \stackrel{y}{\circ} \end{aligned}$ | \% | ¢ | $\stackrel{\overline{6}}{\circ}$ | $\stackrel{\sim}{z}$ | @ |  | \% | @ | 或 | $\because$ | ¢00 | \% | ต | ¢ | $\underline{\sim}$ | ๓ |
| W Broadway <br> Ave (CSAH <br> 81) to <br> Golden <br> Valley Rd | Daily | 5,410 | 5,410 | 0 | 6,090 | 0 | 6,090 | 11,500 | 5,410 | 6,090 | 00 | 0 | 11,160 | 5,260 | 5,900 | 430 | 190 | 240 |
|  | PM Peak | 410 | 410 | 0 | 540 | 0 | 540 | 950 | 410 | 540 | 00 | 0 | 930 | 400 | 530 | 40 | 20 | 20 |
| Golden <br> Valley Rd to <br> Plymouth <br> Ave | Daily | 6,140 | 6,140 | 0 | 6,500 | 0 | 6,500 | 12640 | 6,140 | 6,500 | 0 | 0 | 12,280 | 5,780 | 6,500 | 450 | 450 | 0 |
|  | $\begin{gathered} \hline \text { PM } \\ \text { Peak } \end{gathered}$ | 440 | 440 | 0 | 640 | 0 | 640 | 1080 | 440 | 640 | 0 | 0 | 1050 | 410 | 640 | 30 | 30 | 0 |
| Plymouth <br> Ave to TH 55 | Daily | 5,150 | 5,150 | 0 | 5,670 | 0 | 5,670 | 10820 | 5,150 | 5,670 | 0 | 0 | 10,540 | 4,870 | 5,670 | 350 | 350 | 0 |
|  | PM <br> Peak | 410 | 410 | 0 | 500 | 0 | 500 | 910 | 410 | 500 | 00 | 0 | 880 | 380 | 500 | 40 | 40 | 0 |

## Future Operations

The D2 alternatives were modeled with LRT, which is assumed to operate under a transit priority scheme in these corridors, based on the operating speeds of the transit and the adjacent roadway. For this analysis, LRT was assumed to operate three-car trains at 7.5 minute headways.

The transitway was modeled in Synchro either as a concurrent phase with the through movements or as an exclusive phase with overlaps, depending on the alignment of the transitway through the intersection. The following intersections are assumed to operate as a concurrent phase with the through movements:

- W Broadway Avenue (CSAH 81) and 29th Avenue
- W Broadway Avenue (CSAH 81) and $26^{\text {th }}$ Avenue
- W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) (D2-B only)
- Penn Avenue (CSAH 2) and Golden Valley Road (D2-A and D2-C)
- Penn Avenue (CSAH 2) and 16th Avenue (D2-A and D2-C) - not modeled
- Penn Avenue (CSAH 2) and Plymouth Avenue (D2-A and D2-C)
- Penn Avenue (CSAH 2) and Oak Park Avenue (D2-A and D2-C) - not modeled

The following intersections have transit modeled as an exclusive phase, with overlaps as appropriate:

- W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) (D2-A and D2-C)
- W Broadway Avenue (CSAH 81) and Oliver Avenue (D2-B only)
- Oliver Avenue and Golden Valley Road (D2-B only)
- Oliver Avenue and $16^{\text {th }}$ Avenue (D2-B only) - not modeled
- Oliver Avenue and Plymouth Avenue (D2-B only)
- Oliver Avenue and Oak Park Avenue (D2-B only) - not modeled
- TH 55 and Penn Avenue (CSAH 2) (D2-A and D2-C)
- TH 55 and Oliver Avenue (D2-B only)

Protected only phasing is needed to control left-turn movements for vehicles traveling parallel to the guideway that then turn left across the guideway. The through movement is generally compatible with the transit movement, but the left-turn vehicles must be protected so as not to turn across the path of an approaching transit vehicle. This is particularly a risk for a transit vehicle approaching from the rear of the passenger vehicle. Left-turn movements from a cross street across the guideway can be either protected or permissive, as the through movements are not compatible with the transit phase.

Left-turn movements in the D2 alignment are assumed to operate with protectedonly phasing for all movements that cross the tracks with a concurrent or an overlap phase. Left-turns on the arterial (W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2)) are therefore assumed to be prohibited where an exclusive left-turn lane is not provided due to the severe operational impacts of operating opposing travel directions with split phasing (e.g., NB and SB Penn Avenue (CSAH 2) operating as separate phases). These assumptions are consistent with current operations on the Hiawatha LRT line in downtown Minneapolis and the proposed operations on the Central Corridor LRT line.

The northbound and southbound approaches at TH 55 and Penn Avenue (CSAH 2) currently operate with split phasing (i.e., northbound and southbound movements do not operate concurrently); however this causes a number of capacity and LOS issues that are observed in the existing conditions. Therefore, the signal phasing for all the D2 alternatives was assumed to consist of permissive left-turning phasing and concurrent through movement phasing for the northbound and southbound approaches.

In the alternative D2-A modeling, parking restrictions were assumed to be implemented on Oliver Avenue near the signalized intersections to allow for through vehicles to maneuver around turning vehicles. Alternative D2-A was modeled with two different signal phasing options (D2-A1 and D2-A2) at the intersections of TH 55/Penn Avenue (CSAH 2) and TH 55/Oliver Avenue. D2-A1 assumes that one controller would be used to operate both intersections and D2A2 assumes two separate controllers would be used, which allows independent operations/ phasing at each intersection, although coordination was still assumed. Due to the high volume of northbound vehicles on Penn Avenue that are diverted to northbound Oliver Avenue, with two controllers this movement must weave across three lanes of eastbound TH 55 traffic to turn left at Oliver Avenue. The interaction of accelerating northbound vehicles with high-speed eastbound TH 55 vehicles could cause safety concerns. With one controller in option D2-A1, the northbound traffic will not interact with eastbound TH 55 traffic, which eliminates the weaving issue. However, one controller operation limits the available green time for eastbound/ westbound through vehicles on TH 55.

In alternative D2-B, the intersections of Penn Avenue (CSAH 2) at Golden Valley Road and Penn Avenue (CSAH 2) at Plymouth Avenue will not be affected by the transit route; therefore, the geometries modeled were the same as existing.

The operational results for the D2-A, D2-B, and D2-C alignments under 7.5minute (LRT) headways are shown in Table 4, Table 5, and Table 6, respectively.

Table 4. Intersection Delay and LOS Results: Alignment D2-A (LRT - 7.5-Minute Headways)

| Intersection | D2-A1: Single Controller at TH 55/Penn Avenue (CSAH <br> 2)/ Oliver Avenue |  |  | D2-A2: Two Signal Controllers at TH 55/Penn Avenue (CSAH 2) and TH 55/Oliver Avenue |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { Delay } \\ (\mathrm{sec} / \mathrm{veh}) \end{gathered}$ | LOS | Comments | $\begin{gathered} \text { Delay } \\ (\mathrm{sec} / \mathrm{veh}) \end{gathered}$ | LOS | Comments |
| W Broadway Ave (CSAH 81) \& 29th Ave ${ }^{1}$ | 7.4 | A | All movements operate at LOS B or better | - | - | Same as D2-A1 |
| W Broadway Ave (CSAH 81) \& $26^{\text {th }}$ Ave ${ }^{2}$ | 19.2 | B | All movements operate at LOS D or better | - | - | Same as D2-A1 |
| W Broadway Ave (CSAH 81) \& Penn Ave (CSAH 2) ${ }^{3}$ | 112.1 | F | Northbound approach and westbound left-turn operate at LOS F | - | - | Same as D2-A1 |
| Penn Ave (CSAH 2) \& Golden Valley Rd | 12.3 | B | All movements operate at LOS B or better | - | - | Same as D2-A1 |
| Oliver Ave \& Golden Valley Rd | 17.9 | B | All movements operate at LOS C or better | - | - | Same as D2-A1 |
| Penn Ave (CSAH 2) \& Plymouth Ave | 21.1 | C | All movements operate at LOS C or better | - | - | Same as D2-A1 |
| Oliver Ave \& Plymouth Ave | 26.1 | C | All movements operate at LOS D or better | - | - | Same as D2-A1 |
| TH 55 \& Penn Ave (CSAH 2) | 62.9 | E | Eastbound and southbound approaches operate at LOS F | 72.6 | E | Eastbound through and southbound approach operates at LOS F |
| TH 55 \& Oliver Ave | 32.0 | C | All movements operate at LOS D or better | 20.2 | C | All movements operate at LOS D or better |

${ }^{1}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 60 vehicles in the PM peak hour and more than 500 vehicles daily.
${ }^{2}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 80 vehicles in the PM peak hour and more than 800 vehicles daily.
${ }^{3}$ Assumes eastbound left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 30 vehicles in the PM peak hour and more than 250 vehicles daily.

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Table 5. Intersection Delay and LOS Results: Alignment D2-B

| Intersection | 7.5-Minute Headways (LRT) |  |  |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (sec/ veh <br> ( | LOS | Comments |
| W Broadway Ave (CSAH <br> 81) \& 29th Ave 1 | 6.9 | A | All movements operate at <br> LOS B or better |
| W Broadway Ave (CSAH <br> 81) \& 26th Ave 2 | 17.8 | B | All movements operate at <br> LOS D or better |
| W Broadway Ave (CSAH <br> 81) \& Penn Ave (CSAH <br> 2) 3 | 36.8 | D | Northbound left-turn and <br> through and southbound left- <br> turn movements operate at <br> LOS E |
| Penn Ave (CSAH 2) and <br> Golden Valley Rd | 23.1 | C | All movements operate at <br> LOS C or better |
| Penn Ave (CSAH 2) and <br> Plymouth Ave | 38.5 | D | Eastbound left-turn and <br> through movements and <br> southbound left-turn <br> movement operate at LOS E |
| TH 55 \& Penn Ave <br> (CSAH 2) | 59.8 | E | Southbound approach <br> operates at LOS F; eastbound <br> left-turn and northbound left- <br> turn and through operate at <br> LOS E |
| TH 55 \& Oliver Ave | 7.0 | A | All movements operate at <br> LOS A |

${ }^{1}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 60 vehicles in the PM peak hour and more than 500 vehicles daily.
2 Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 80 vehicles in the PM peak hour and more than 800 vehicles daily.
${ }^{3}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 150 vehicles in the PM peak hour and more than 1,100 vehicles daily.

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Table 6. Intersection Delay and LOS Results: Alignment D2-C

| Intersection | 7.5-Minute Headways (LRT) |  |  |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/ veh) | LOS | Comments |
| W Broadway Ave (CSAH <br> 81) \& 29th Ave 1 | 7.2 | A | All movements operate at LOS <br> B or better |
| W Broadway Ave (CSAH <br> 81) \& 26th Ave 2 | 18.7 | B | All movements operate at LOS <br> D or better |
| W Broadway Ave (CSAH <br> 81) \& Penn Ave (CSAH <br> 2) 3 | 56.4 | E | Westbound left-turns operate <br> at LOS F; northbound approach <br> and southbound left-turns <br> operate at LOS E |
|  <br> Golden Valley Rd | 32.3 | C | All movements operate at LOS <br> D or better except northbound <br> left-turns which operate at LOS <br> E |
|  <br> Plymouth Ave | 49.2 | D | All left-turn movements operate <br> at LOS E or LOS F; southbound <br> through and right-turn operate <br> at LOS E |
| TH 55 \& Penn Ave <br> (CSAH 2) | 79.1 | E | Southbound approach and <br> eastbound left-turns operate at <br> LOS F; all other movements <br> operate at LOS D or LOS E |

${ }^{1}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 60 vehicles in the PM peak hour and more than 500 vehicles daily.
${ }^{2}$ Assumes left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 80 vehicles in the PM peak hour and more than 800 vehicles daily.
${ }^{3}$ Assumes eastbound left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 30 vehicles in the PM peak hour and more than 250 vehicles daily.

In alternative D2-A, the through movements along the W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) corridors operate concurrently with the transit phase and generally operate at LOS D or better. However, the high-volume intersections of W Broadway Avenue (CSAH 81)/Penn Avenue (CSAH 2 ) and TH 55/Penn Avenue (CSAH 2 ) are expected to operate near capacity, with several movements at LOS F. Options D2-A1 and D2-A2 had similar intersection delays for the two different signal operations at the intersections of TH 55 at Penn Avenue (CSAH 2) and Oliver Avenue. Option D2-A1 with one controller had slightly

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lower delays, which appeared to be due to less delay for the northbound movements from Penn Avenue (CSAH 2) to Oliver Avenue and also because vehicles on TH 55 only have to stop at one intersection, instead of two.

Alternative D2-A as currently proposed is expected to result in the W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) and TH 55/Penn Avenue (CSAH 2) intersections operating at LOS E or LOS F in the 2030 PM peak hour. This is due to limited capacity at the intersection due to the LRT guideway requiring the removal of through lanes and/ or taking a portion of the signal cycle for the LRT movement diagonally through the intersection. At W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2), the mitigation measures would require either banning the northbound left-turn movement or providing a 200 -foot northbound left-turn lane to improve from LOS F to LOS D/E (the assumed threshold for acceptable peak hour operations). Restricting the northbound left-turn movements would be expected to result in diversion to other intersections, such as Oliver Avenue/ Golden Valley Road, Theodore Wirth Parkway/ Golden Valley Road and/ or Penn Avenue (CSAH 2)/ Lowry Avenue. Additional northbound leftturn storage prevents the left-turn traffic from spilling into the through lane and also provides additional capacity to allow better use of the northbound green time at the signal, however it appears that additional right-of-way would be required to extend the northbound left-turn lane. There is sufficient storage for the westbound left-turn, therefore restricting this movement is not recommended. The westbound left-turn movement was modeled with protected only phasing, due to the crossing of the LRT guideway, which contributes to the high delay for that movement.

At TH 55 and Penn Avenue (CSAH 2), providing a 200 foot southbound left-turn lane improves the capacity of the intersection by removing the left-turning vehicles from the through lane. Left-turn storage is available through parking restrictions and lane reassignment by removing the exclusive southbound rightturn lane and creating a shared through/right-turn lane. This mitigation is in addition to geometric improvements to allow the northbound/ southbound leftturn movements to operate concurrently. Providing southbound left-turn storage is expected to improve the intersection to LOS D operations during the PM peak. Table 7 shows the intersection delay and LOS for Alternative D2-A with mitigation measures implemented.

Table 7. Intersection Delay and LOS Results: Alignment D2-A Mitigation

| Intersection | Mitigation <br> Measure | $7.5-$ Minute Headways (LRT) |  |  |
| :--- | :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS | Comments |  |
| W Broadway <br> Ave (CSAH 81) <br> \& Penn Ave <br> (CSAH 2) 1 | 200 foot <br> northbound left- <br> turn storage | 54.8 | D | Northbound and <br> westbound left- <br> turns operate at <br> LOS F |
| W Broadway <br> Ave (CSAH 81) <br> \& Penn Ave <br> (CSAH 2) 1 | Banned <br> northbound left- <br> turns | 60.7 | E | Westbound left- <br> turns operate at <br> LOS F |
| TH 55 \& Penn <br> Ave (CSAH 2) | One controller, <br> 200 foot <br> southbound <br> left-turn storage | 49.6 | D | Southbound left- <br> turns operate at <br> LOS F |

${ }^{1}$ Assumes eastbound left-turn movements on Broadway Avenue (CSAH 81) are banned, impacting approximately 30 vehicles in the PM peak hour and more than 250 vehicles daily.

Alternative D2-B operates similar to D2-A with acceptable operations for W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2), with the critical intersections at W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) and TH 55/Penn Avenue (CSAH 2). The impact of the guideway through the W Broadway Avenue (CSAH 81)/Penn Avenue (CSAH 2) intersection has significant capacity impacts, which are somewhat mitigated by eliminating the left-turn movements. However, this traffic would get pushed elsewhere in the network - either to neighborhood streets or onto other arterials such as Emerson Avenue/ Fremont Avenue (one-way pair) and Lowry Avenue.

Alternative D2-B is expected to result in LOS E operations at the TH 55/Penn Avenue (CSAH 2) intersection in the 2030 PM peak hour. Similar to Alternative D2-A, a 200-foot southbound left-turn lane with protected/ permissive southbound left-turn phasing would be expected to improve the overall intersection from LOS E to LOS D. The left-turn storage provides additional capacity to the intersection and allows the southbound through movement to take better advantage of the green time. Similar to D2-A mitigation at the intersection, parking restrictions and lane reassignment would be needed to provide the exclusive left-turn lane with a shared through/right-turn lane. Table 8 shows the intersection delay and LOS for Alternative D2-B with mitigation measures implemented.

Table 8. Intersection Delay and LOS Results: Alignment D2-B Mitigation

| Intersection | Mitigation <br> Measure | $7.5-$ Minute Headways (LRT) |  |  |
| :--- | :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS | Comments |  |
| TH 55 \& Penn <br> Ave (CSAH 2) | One controller, <br> 200 foot <br> southbound <br> left-turn storage | 49.7 | D | Northbound left- <br> turn and through <br> operate at LOS F; <br> Eastbound left- <br> turn operates at <br> LOS F |

Alternative D2-C provides the most capacity on Penn Avenue (CSAH 2) because of the additional right-of-way that provides northbound left-turn storage on Penn Avenue (CSAH 2) at W Broadway Avenue (CSAH 81). In addition, because the transitway does not operate on W Broadway Avenue (CSAH 81) between Oliver Avenue and Penn Avenue (CSAH 2), westbound left-turn storage can be provided on W Broadway Avenue (CSAH 81) at Penn Avenue (CSAH 2).

Alternative D2-C is expected to result in LOS E operations at the W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) in the 2030 PM peak hour. Similar to Alternative D2-A, a recommended mitigation measure would be to provide a 200foot northbound left-turn lane, which would be expected to improve the overall intersection operations from LOS E to LOS D. The left-turn storage provides additional capacity to the intersection and allows the northbound through movement to take better advantage of the green time. The additional northbound left-turn storage would likely require additional right-of-way acquisition. Restricting the northbound left-turn movements would also address the LOS issue, but would be expected to result in diversion to other intersections, such as Oliver Avenue/ Golden Valley Road, Theodore Wirth Parkway/ Golden Valley Road and/ or Penn Avenue (CSAH 2)/ Lowry Avenue. There is sufficient storage for the westbound left-turn, therefore restricting this movement is not recommended. The westbound left-turn movement was modeled with protected only phasing, due to the crossing of the LRT guideway, which contributes to the high delay for that movement. At TH 55 and Penn Avenue (CSAH 2), providing a 200 foot southbound left-turn lane improves the capacity of the intersection, but is not enough to improve the overall intersection to LOS D due to the limited capacity and signal green time available for the left-turn movements. Table 9 shows the intersection delay and LOS for Alternative D2-C with mitigation measures implemented.

Table 9. Intersection Delay and LOS Results: Alignment D2-C Mitigation

| Intersection | Mitigation <br> Measure | 7.5-Minute Headways (LRT) |  |  |
| :--- | :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS | Comments |  |
| W Broadway <br> Ave (CSAH 81) <br> \& Penn Ave <br> (CSAH 2) 1 | 200 foot <br> northbound left- <br> turn storage | 53.0 | D | Westbound left- <br> turns operate at <br> LOS F |
| W Broadway <br> Ave (CSAH 81) <br> \& Penn Ave <br> (CSAH 2) 1 | Banned <br> northbound left- <br> turns | 38.1 | D | Westbound left- <br> turns operate at <br> LOS F |
| TH 55 \& Penn <br> Ave (CSAH 2) | 200-foot <br> southbound <br> left-turn storage | 60.3 | E | All left-turns <br> operate at LOS E <br> or LOS F |

## Pedestrian Crossings

Table 10 shows the current pedestrian crossing volumes of W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2) at signalized intersections in the study area. The table also shows the distance between each signalized intersection under the existing and proposed conditions. Therefore, the furthest distance a pedestrian would have to walk to cross the guideway, assuming no unsignalized crossings of the guideway, is one-half the distance between signalized intersections. Unsignalized crossings of the guideway require additional space in the median to accommodate a refuge space between the road crossing and the guideway crossing and have been assumed to be infeasible on these corridors due to right-of-way constraints. In general, the pedestrian crossing volumes are very low and would not meet the pedestrian warrant for a traffic signal. Increases in pedestrian volumes due to the introduction of LRT would be expected mainly near the station areas (i.e., Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2)).

DRAFT ENVIRONMENTAL IMPACT STATEMENT

Table 10. Pedestrian Volumes Across Transitway and Crossing Distances

| Intersection | Pedestrian <br> Volumes in Peak <br> Hour |  | Existing <br> Distance <br> between <br> Signalized <br> Crossings <br> (miles) | Proposed <br> Distance <br> between <br> Signalized <br> Crossings <br> (miles) |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  | MID | PM |  |  |  |
| W Broadway Ave (CSAH <br> 81) \& 29th Ave | 0 | 0 | 16 |  | 0.29 |
| W Broadway Ave (CSAH <br> 81) \& 26th Ave | 8 | 4 | 20 | 0.29 | 0.20 |
| W Broadway Ave (CSAH <br> 81) \& Penn Ave (CSAH 2) | 10 | 23 | 20 | 0.20 | 0.20 |
|  <br> Golden Valley Rd | 20 | 26 | 41 | 0.33 | 0.33 |
| Penn Ave (CSAH 2) \& 16th <br> Ave | 2 | 10 | 8 | 0.23 | 0.23 |
| Penn Ave (CSAH 2) \& 14 th <br> Ave | 16 | 17 | 13 | 0.12 | - |
|  <br> Plymouth Ave | 43 | 9 | 14 | 0.12 | 0.24 |
| Penn Ave (CSAH 2) \& 12 th <br> Ave | 4 | 1 | 6 | 0.12 | - |
| Penn Ave (CSAH 2) \& Oak <br> Park Ave | 10 | 7 | 11 | 0.12 | 0.24 |
| TH 55 \& Penn Ave (CSAH <br> 2) | 4 | 2 | 8 | 0.22 | 0.11 |

## SUMMARY

The existing PM peak hour modeling showed acceptable operations for all signalized intersections in the study area on W Broadway Avenue (CSAH 81) and Penn Avenue (CSAH 2). Due to the capacity constraints caused by the introduction of a transitway along the corridors, 20 percent of through traffic volumes were assumed to divert to other corridors in 2030. In addition, left-turn movements were assumed to be restricted at all locations along the alignment where left-turn lanes could not be provided. The McNair Avenue approach was also assumed to be removed from the W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) intersection because of the limited capacity available at the signal and the inefficient operation resulting from the inclusion of a fifth leg at the intersection.

In general, the modeling showed that Alternative D2-A resulted in LOS F operations at W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) due to the restricted capacity. The different control options, D2-A1 and D2-A2, showed similar intersection delay results for TH-55 at Penn Avenue (CSAH 2) and Oliver Avenue; however, the single controller for the two intersections operated with slightly less delay for northbound Penn Avenue (CSAH 2) and eliminates the weaving issue between intersections. Alternative D2-C showed the least vehicle delay, but comes at significant right-of-way acquisition costs. The potential mitigation measures to address the operational and delay issues at the W Broadway Avenue (CSAH 81)/ Penn Avenue (CSAH 2) include:

- Construct a 200 -foot northbound left-turn lane. This would require additional right-of-way.
- Restrict all northbound left-turn movements at the intersection. This would result in additional traffic on other roadways such as Golden Valley Road, Theodore Wirth Parkway, or Lowry Avenue.
- While the westbound left-turn movement is expected to operate at LOS F, this is primarily due to the protected only phasing to cross the guideway and limited available green time for this phase. There is adequate storage space for the left-turn movement, therefore restricting the westbound left-turn is not recommended.

The TH 55 and Penn Avenue (CSAH 2) intersection is a second capacity constraint in all alternatives. The introduction of the transit phase does cause some additional delay, primarily to the Penn Avenue (CSAH 2) approaches and the westbound TH 55 approach due to the need for a transit phase (which can also run concurrently with eastbound TH 55). The recommended mitigations to address the 2030 operational issues at the TH 55/Penn Avenue (CSAH 2) intersection include:

- Geometric improvements to allow the northbound/ southbound left-turn movements to operate concurrently (eliminate existing split phasing)
- Reassign traffic lanes to create a 200-foot southbound left-turn lane and a shared through/right-turn lane. This improvement could be done through parking restrictions and does not require additional right-of-way acquisition.







## Memorandum

Date: December 6,2011
To: Brent Rusco, P.E., J oe Gladke, P.E.
Hennepin County Regional Railroad Authority
From: JoNette Kuhnau, P.E., PTOE Kimley-Horn and Associates, Inc.

Subject: Bottineau Transitway Draft Environmental Impact Statement Transit/Traffic Signal Operating Schemes Technical Memorandum

This memorandum presents a discussion of transit signal priority (TSP) and transit preemption operating scenarios at the signalized intersections along the Bottineau Transitway. The document is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor. It is intended for review and consideration by technical staff of Hennepin County, agency staff that operate traffic signals along the transitway alignment, and Metro Transit staff.

The following sections focus on the traffic operations, transit operations, and cost implications of TSP and preemption operating scenarios along the corridor generally, and also identifies potential impacts and implications of the operating scenario in specific segments of the corridor. Additional considerations including transit travel time, transit ridership, and specific right-of-way impacts are discussed in this memorandum, but have not been analyzed for this memorandum.

## DEFINITIONS AND STANDARDS

This section defines the terms and operating scenarios to be discussed in this memorandum, based on the standards and definitions contained in the 2005 Minnesota Manual on Uniform Traffic Control Devices (MnMUTCD).

In terms of traffic signal control, priority and preemption operating schemes are defined as follows:

- Priority Control - The modification of normal operation of a traffic control signal that alters the sequences of phases or changes the timing in response to an external event without leaving the normal mode of operation. This scheme will be referred to as transit signal priority (TSP) or priority in this document, but is also sometimes called "partial priority" in the traffic engineering industry.
For the context of this memorandum, priority operations are assumed to consist of the displaying of early or extended green signal indications at an intersection to assist public transit vehicles in remaining on schedule.
- Preemption Control - The transfer of normal operation of a traffic control signal to a special control mode of operation in response to an external event. This scheme will be referred to as preemption in this document, but is also sometimes called "full priority" in the traffic engineering industry in reference to transit operations (as opposed to emergency vehicle preemption, which is always referred to as a preemption operation).

Preemption control can have several types of operation, even at the same intersection, depending on the type of vehicle making the preemption call:

- The prompt displaying of green signal indications at signalized locations ahead of fire vehicles, law enforcement vehicles, ambulances, and other official emergency vehicles.
- A special sequence of signal phases and timing to provide additional clearance time for vehicles to clear the tracks prior to the arrival of a train (heavy rail or light rail).
- A special sequence of signal phases to display a red indication to prohibit turning movements towards the tracks during the approach or passage of a train (heavy rail or light rail).

The MnMUTCD defines priority at traffic signals that are designed to respond under both preemption and/ or priority control to more than one type or class of vehicle:

1. Railroad preemption, including trains and semiexclusive alignment light rail crossings where the light rail transit movement is not controlled by a traffic control signal or a light rail transit signal.
2. Emergency vehicle preemption, including fire and law enforcement vehicles, ambulances, and other official emergency vehicles.
3. Transit priority, including buses and semiexclusive or mixed-use alignment light rail crossings where the light rail transit movement is controlled by a traffic control signal or a light rail transit signal.

Relative to the transitway alignment, the types of alignments are defined based on the transit vehicle interaction with other traffic as follows:

- Exclusive - A transit right-of-way that is grade-separated or protected by a fence or traffic barrier. Motor vehicles, pedestrians, and bicycles are prohibited within the right-of-way. Subways and aerial structures are included within this group. This type of alignment does not have at-grade grade crossings.
- Semiexclusive: A transit alignment that is in a separate right-of-way or along a street or railroad right-of-way where motor vehicles, pedestrians, and bicycles have limited access and cross at designated locations only.
- Mixed-Use: An alignment where light rail transit operates in mixed traffic with all types of road users. This includes streets, transit malls, and pedestrian malls where the right-of-way is shared.

The Bottineau Transitway would be considered to be a semiexclusive alignment for its entire length, both in the side-running and center-running segments. This means that by MnMUTCD standards, either priority or preemption control can be used at traffic signals.

In combination with traffic signal control, the MnMUTCD also provides for the use of four-quadrant gate systems, automatic gates, flashing-light signals, actuated blank-out and variable message signs, and other active traffic control devices for at-grade transitway crossings. The use of these devices can be based on engineering judgment, but the MnMUTCD provides the following guidance:

- When light rail transit speed is cited in this Part (Section 10D), it refers to the maximum speed at which light rail transit vehicles are permitted to traverse a particular grade crossing. (Section 10D.1)
- Highway-light rail transit grade crossings in semiexclusive alignments should be equipped with automatic gates and flashing-light signals where light rail transit speeds exceed 35 miles per hour (mph).(Section 10D.3)
- Traffic control signals may be used instead of automatic gates at highway-light rail transit grade crossings within highway-highway intersections where light rail transit speeds do not exceed 35 mph . (Section 10D.3)

When automatic gates are used, the following clearance standards apply:

- Minimum Track Clearance Distance - for standard two-quadrant railroad warning devices, the minimum track clearance distance is the length along a highway at one or more railroad tracks, measured either from the highway stop line, warning device, or 12 ft perpendicular to the track centerline, to 6 ft beyond the track(s) measured perpendicular to the far rail, along the centerline or edge line of the highway, as appropriate, to obtain the longer distance. (Section 8A.1)


## OPERATING SCENARIO CONSIDERATIONS

The decision about whether to operate a traffic signal with priority or preemption control is based not only on standards and requirements, but also balancing the needs and priorities of each of the traffic flows (automobile and transit). The following sections discuss the considerations that should influence the decision about whether to operate a signalized intersection under a priority or preemption scenario and the implication of these decisions as applicable to the Bottineau Corridor. This discussion is applicable only to the light rail transit mode, as the MnMUTCD does not discuss dedicated bus rapid transit (BRT) facilities and has no signal operations requirements (automatic gates or preemption) for BRT.

## Signal Preemption

The purpose of traffic signal preemption near railroad or light rail grade crossings is to increase safety at these intersections by clearing vehicles from the path of approaching trains, particularly those traveling at high speeds. The main advantages of signal preemption for transit vehicle operations are more efficient and reliable travel times. Automatic gates are assumed to be used at speeds greater than 35 mph . There are a number of distinct time segments involved in a preemption scenario:

- Preemption call is received through detection in the guideway.
- The signal phase running when the preemption call is received is terminated.
- One or more clearance phases are run to clear any vehicles from the track crossing. The sequence of clearance phases can be as short as 20 seconds or longer than 60 seconds depending on the configuration of the crossing and the preferences of the agency operating the signal.
- The signal runs the phase(s) compatible with the transit crossing. This phase is typically held until the transit vehicle clears the "check out" loop at the far side of the intersection.
- The signal runs the "recovery" phase. There are several ways a signal can recover from a preemption, which can be based on traffic operations and the preferences of the agency operating the traffic signal. The following are typical recovery options for a signal with a siderunning guideway (e.g., Bottineau Transitway along Bottineau Boulevard (CSAH 81) in Brooklyn Park):
- Recover to the phase that the signal would have been in had there not been a preemption call. This mode attempts to recover corridor coordination as soon as possible, but usually has the biggest impact on side street delays and mainline leftturn movements since they are most likely to be skipped either during or recovering from the preemption.
- Recover to the side street through and/ or left-turn phases (i.e., phases that are not compatible with transit and therefore may be skipped during a preemption call).
- Recover to mainline left-turn phases (i.e., phases that are not compatible with transit and therefore may be skipped during a preemption call).

Several important factors must be considered when assessing preemption operations and how signals will recover from a preemption call into normal signal operations:

- Delays to mainline automobile traffic (e.g., minimize mainline through delay at individual intersection)
- Coordination for mainline automobile traffic (e.g., minimize mainline through delay and maximize corridor progression)
- Delays for left-turn and cross-street traffic (e.g., minimize delays for non-compatible phases)
- Consequences of multiple back-to-back preemption calls (e.g., the same phase being omitted in multiple consecutive cycles, which can lead to drivers violating the traffic signal or the automatic gates)
- Capabilities of the traffic signal controller and software at the intersection. There are several type of controllers and software available, but in general Econolite ASC2 and ASC3 controllers (currently used by Hennepin County) have been shown to have greater capabilities to handle both priority and preemption calls compared to Siemens Eagle controllers (currently used by City of Minneapolis).

The diagram in Figure 1 shows an illustration of a "normal" signal cycle and the same cycle with a transit preemption occurring during the mainline through movement phase, with the signal attempting to recover into coordination. Depending on the signal spacing, the frequency of preemption calls, and the length of preemption calls, coordination may or may not be able to be maintained in a corridor. Attempting to maintain coordination, as shown in the example cycle in Figure 1 , will result in higher delays on the cross street because the mainline through movements receive additional green time (since they are compatible with the through movement) and because the signal is forced back to the mainline phases according to the coordination plan.


Figure 1. Premption Signal Cycle Example
The traffic volumes on the cross street should also play a role in determining whether it is more important to maintain mainline coordination or minimize overall intersection delay. Figure 2 shows an alternate operations strategy that recovers into the next phase to be sequenced. This type of recovery typically reduces the delays for the cross street and left-turn movements, but also means the signal will need several cycles to attempt to get back into coordination. Depending on the frequency of preemption calls and the spacing of the signals in the corridor, it may not be possible to maintain coordination at all.


Figure 2. Alternate Preemption Signal Cycle Example
In addition to traffic signal operations, a preemption operating scenario also typically necessitates the use of automatic gates at the intersection to prevent vehicles from crossing the guideway. For a siderunning transit alignment, the gate configuration typically involves a center median on the cross street and one automatic gate for each approaching travel direction (e.g., west of the guideway for eastbound traffic and east of the guideway for westbound traffic). Automatic gates are also typically used for the parallel right-turn movement across the guideway. This is the standard gate installation used for the Hiawatha LRT line where it is side-running along TH 55 in Minneapolis. Automatic gates do have significant cost implications, typically costing approximately $\$ 250,000-\$ 300,000$ per intersection for the gate arms, controllers, blank-out signs, detection, and other necessary equipment.

The installation of automatic gates for a center-running guideway is much more unusual and challenging due to the space needed for the gate foundations and arms, as well as placement of the gate to best protect automobile traffic from conflicts with transit vehicles, as well as other automobile traffic. One known center-running LRT system with gates is located on 36 th Street NE in Calgary, Alberta, Canada. On this system, four-quadrant gates are installed parallel to the guideway to prevent vehicles from crossing the guideway. Gates are also installed in some locations for the parallel leftturn movements across the guideway, where four-quad gates are not used. This configuration is shown in the photo in Figure 3 and is the safest configuration in terms of preventing conflicts between automobile traffic and the transit vehicles. However, an automobile on the cross street that enters the intersection during a preemption, when the automatic gates are being lowered, could potentially be trapped in the intersection and have conflicts with other vehicle traffic.


Figure 3. Center-Running Gated LRT Guideway
$36^{\text {th }}$ Street NE/ $39^{\text {th }}$ Avenue NE intersection. Source: Google Streetview
Another potential option would be to install automatic gates on the cross street approaches (at the stop bars) and across the parallel (mainline) left-turn lanes. The advantage of this approach is that it avoids any cross street traffic from being trapped in the intersection. However, if vehicles violate the traffic signal or drive around the gates, they are not physically prevented from entering the guideway while a transit vehicle is approaching, which is a potential safety concern. In either gate configuration, approximately 10 to 15 feet of additional space is required for the gate installations, either in the guideway or for the mainline left-turn medians. For the Bottineau Transitway, this would potentially have right-of-way or construction implications for CSAH 103 (W Broadway Ave). The other segments with posted roadway speeds greater than 35 mph have assumed transitway operating speeds of 35 mph or less, therefore not triggering the need automatic gates.

For transitway operating speeds greater than 35 mph , a priority scheme may be feasible from a purely technical standpoint, but is not known to be operating on any transit system in the United States. A priority scheme is not recommended for transit operations in any segment with speeds greater than 35 mph for the following reasons:

- Priority operations do not include the track clearance phases that would be triggered by a preemption call. Therefore there may be greater risk that vehicles are queued across the guideway when the train arrives, particularly on a side-running alignment.
- Priority operations ultimately mean that the transit vehicle may be required to stop at signalized intersections. This would in turn require bar signal and transit detection infrastructure located not only at the intersection but also several hundred feet upstream to
allow the transit vehicle adequate time and distance to come to a complete stop prior to the intersection.
- In addition to the delay times transit vehicles would experience at the signalized intersections, which could be 30 seconds or more at any individual intersection, transitway running times will be further impacted by the need to accelerate and decelerate from a minimum speed of 0 mph (stopped condition) to a maximum of $45-55 \mathrm{mph}$ (maximum operating speed).
- Past studies have shown the priority operations compared to preemption operations may have slightly better operations for automobile traffic, but have significantly worse operations for transit traffic. This result is dependent on many factors, including automobile traffic volumes, traffic signal controller, and preemption clearance and recovery phasing. However, it is still important to consider whether the benefit gained by the automobile traffic in a priority scheme is sufficient to justify the impact on transit travel times and delays.
Measures that can mitigate the potential impacts of preemption operations include software/ controller selection and programming, location of transit vehicle detection (i.e., timing of preemption call), preemption clearance phasing and timing, preemption recovery phasing, and vehicle detection.


## Transit Signal Priority

Priority signal operation is generally used in urban and some suburban environments where transit vehicles travel at lower speeds and are controlled at intersections by traffic signals and bar signals. The nature of priority operation is that a transit vehicle may have to slow or stop before proceeding through the intersection. In a signalized corridor, TSP has the benefit of maintaining traffic progression and coordination on the main roadway by providing more green time for the transit vehicle to proceed through an intersection when possible. The agency that operates the signals has the flexibility to designate by corridor or by intersection how much of the total signal cycle time is available to provide the transit advantage. The amount of available time is based on several factors and assumptions:

- Minimum pedestrian crossing times - pedestrian crossing times will never be shortened in a TSP scenario.
- Minimum green and clearance times for the cross street - minimum green times (typically 7 to 20 seconds) and clearance times (yellow + all-red - typically 6 to 10 seconds) will never be shortened in a TSP scenario.
- Assume all signal phases are served - TSP scenarios do not typically involve skipping any phases.

The diagram in Figure 4 shows an illustration of a "normal" signal cycle and the same cycle with a transit vehicle serviced using TSP, assuming that the transit phase is compatible with the mainline through movements, 7 -second minimum green times for the left-turn movements, 15 -second minimum green times for the through movements, and 6 -second clearance times for all phases. The sequence of signal phases following a TSP call are the same as if there is no TSP call - the only differences are in the lengths of time for each phase.


Figure 4. Priority Signal Cycle Examples
The main advantage of TSP is that it provides improved on-time reliability and faster travel times for transit vehicles, while minimizing impacts on automobile traffic. This means that transit vehicles are given additional green time at the signal when possible, but if the additional green time is not sufficient to accommodate the transit vehicle's arrival, the transit vehicle would have to stop and wait for the signal to cycle back to its phase. For both LRT and BRT, the infrastructure necessary to accommodate TSP and the potential need to stop include the following:

1. Advance transit detection, located based on speed and calculated maximum stopping distance of the transit vehicle.
2. Nearside and farside bar signals at each signalized intersection. Advance bar signals may also be needed based on line of sight and necessary stopping distance.
3. Communications infrastructure (e.g., fiber optic line) between traffic signal and transit system.
4. Software and hardware to process the transit detection call input and provide the bar signal output.

Items 1 and 3 are needed for either a priority or preemption operating scenario. However, items 2 and 4 can have costs implications of $\$ 25,000-\$ 40,000$ per intersection.

The primary disadvantage of a priority operating scenario occurs in corridors with high travel speeds for both transit and automobile traffic. A TSP operating scenario means that the transit vehicle may have to stop at signalized intersections, especially where the "extra" green time available for transit is small (due to high traffic volumes and/ or limited intersection capacity). Stopping the transit vehicle is most problematic when the transit mode is LRT and the guideway travel speeds are greater than 35
mph . Under these conditions, the time and distance needed for the light rail vehicle to decelerate and accelerate are significant and have negative impacts on the travel time between stations, but also on the rider experience. Further, operating automatic gates in a priority scenario becomes significantly complicated since sometimes the track clearance and gate sequence would need to be triggered immediately upon detection (if a green signal indication was going to be given) or could be delayed by 60 seconds or more (if the transit vehicle cannot be given a green indication and will have to stop at the intersection). This is a very unusual operating scenario and would require significant communications hardware, software, and programmed logic for the equipment to respond differently under the various potential scenarios. There are no known LRT systems operating in a semi-exclusive alignment at high speeds under a priority scenario.

## BOTTINEAU TRANSITWAY OPERATIONS

A number of operating assumptions have been made as part of the Bottineau Transitway Draft Environmental Impact Statement (DEIS) analysis relative to operating speeds in various segments of the corridor and signal operations at the signalized intersections. The DEIS assumptions and recommended signal operations based on those assumptions are documented in Table 1.

For segments $B$ and $C$, where the proposed transitway operating speeds are greater than 35 mph , operating under a priority scheme, rather than preemption, would change the operations at up to 14 signalized intersections. Considering only intersection delay (i.e., ignoring delay to acceleration/ deceleration), priority operations in these segments would have the potential to increase the running times by several minutes, which in turn has a negative impact on ridership

Table 1. Bottineau Transitway Operating Assumptions

| Segment | Proposed Transitway Operating Speed, mph | Posted Roadway Speed Limit, mph | Proposed Guideway Configuration | Signalized Intersections | Recommended Signal Operation |
| :---: | :---: | :---: | :---: | :---: | :---: |
| A | 35 | 35 | Center Median Running | Arbor Lakes Pkwyl Zachary Ln | Priority |
| A | 35 | 40 | Center Median Running | Brooklyn Blvd/ Northland Dr | Priority |
|  |  |  |  | Brooklyn Blvd/ Boone Ave |  |
| B | 45 | 501 | Center Median Running | W Broadway Ave (CSAH 103)/ 94th Ave | Preemption |
|  |  |  |  | W Broadway Ave (CSAH 103)/ 93rd Ave |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ Setzler Pkwy |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ Maplebrook Pkwy |  |
| A/B | 55 | 55 | Side Running (West) | Bottineau Blvd (CSAH 81)/ 73 ${ }^{\text {rd }}$ Ave | Preemption |
|  |  |  |  | Bottineau Blvd (CSAH 81)/ 71 st Ave |  |
| B | 45 | $45^{1}$ | Center Median Running | W Broadway Ave (CSAH 103)/ 85 ${ }^{\text {th }}$ Ave | Preemption |
|  |  |  |  | W Broadway Ave (CSAH 103)/ College Park Dr |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ Candlewood Dr |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ Brooklyn Blvd |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ $76{ }^{\text {th }}$ Ave |  |
|  |  |  |  | W Broadway Ave (CSAH 103)/ 75 ${ }^{\text {th }}$ Ave |  |
| C | 55 | 45 | Side Running (West) | Bottineau Blvd (CSAH 81)/ 63rd Ave | Preemption |
|  |  |  |  | Bottineau Blvd (CSAH 81)/ Bass Lake Rd |  |

${ }^{1}$ Existing speed limits on CSAH 103 (W Broadway Ave) are 45 mph south of $85^{\text {th }}$ Avenue and 50 mph north of $85^{\text {th }}$ Avenue. Future speed limits will be determined as part of the highway reconstruction project, independent of the Bottineau Transitway DEIS.

Table 1 Continued. Bottineau Transitway Operating Assumptions

| Segment | Proposed <br> Transitway Operating Speed, mph | Posted <br> Roadway Speed Limit, mph | Proposed Guideway Configuration | Signalized Intersections | Recommended Signal Operation |
| :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 35 | 40 | Center Median Running | TH 55/ Thomas Ave | Priority |
| D2 Sub-Alt 2 | 35 | 30 | N/A | France Ave/ Oakdale Ave | Priority |
| D2 - Alt 3 | $\leq 35{ }^{2}$ | 40 | Center Median Running | Bottineau Blvd (CSAH 81)/ 35th Ave | Priority |
| D2 D2 Sub-Alt 1 D2 Sub-Alt 2 D2 Sub-Alt 3 | $\leq 352$ | 40 | D2 and Sub-Alt $1=$ Side Running (West) Sub-Alt 2 and Sub-Alt 3 = Center Median Running | Bottineau Blvd (CSAH 81)/ Abbott Ave | Priority |
| D2 | 30 | 35 | Center Median Running | W Broadway Ave (CSAH 81)/ 29th Ave W Broadway Ave (CSAH 81)/ $26^{\text {th }}$ Ave | Priority |
| $\begin{gathered} \text { D2 } \\ \text { D2 - ALTa } \\ \text { D2 - ALTb } \\ \text { D2 - ALTC } \end{gathered}$ | 30 | 30 | D2 and ALTa $=$ Side <br> Running (East) $\mathrm{ALTb}=\mathrm{N} / \mathrm{A}$ <br> ALTC $=$ Center Median Running | W Broadway Ave (CSAH 81)/ Penn Ave (CSAH 2) Penn Ave (CSAH 2) and Oliver Ave/ Golden Valley Rd Penn Ave (CSAH 2) and Oliver Ave/ 16th Ave Penn Ave (CSAH 2) and Oliver Avenue/ Plymouth Ave Penn Ave (CSAH 2) and Oliver Ave/ Oak Park Ave | Priority |
| $\begin{gathered} \text { D1 } \\ \text { D2-ALTb } \end{gathered}$ | 35 | $\begin{gathered} \text { D1 }=40 \\ \text { D2 ALTb }=30 \end{gathered}$ | $\begin{gathered} \text { D1 = Center Median } \\ \text { Running } \\ \text { D2 ALTb }=\text { Turn } \end{gathered}$ | TH 55/ Penn Ave (CSAH 2) and Oliver Ave | Priority |
| D | 35 | 40 | Center Median Running | TH 55/ Morgan Ave TH 55/ Humboldt Ave TH 55/ Van White Memorial Blvd | Priority |
| D | 30 | 40 | Center Median Running | TH 55/Bryant Ave TH 55/ W Lyndale Ave TH 55/ E Lyndale Ave | Priority |
| D | 30 | 30 | Center Median Running | TH 55/ Border Ave TH 55/ $7^{\text {th St/ }} 6^{\text {th }}$ Ave | Priority |

${ }^{2}$ Operating speeds for the D2 Sub-Alternatives have not been defined in the current operating plan. Based on the adjacent segments and locations
of curves, the operating speed is assumed to be at or less than 35 mph .

## SUMMARY AND RECOMMENDATIONS

This document outlines the standards, advantages, and disadvantages related to preemption and priority scenarios for transit operations. Priority operations provide a transit advantage at signalized intersections, with fewer impacts on automobile traffic. Preemption operations provide the safest operations where transit vehicles are traveling at high speeds, but can also have the greatest impacts on automobile traffic flow. However, these impacts can be reduced by the transit vehicle detection and preemption call timing, traffic signal hardware/ software, preemption phasing, and automobile detection.

Based on safety and operational considerations, preemption operations are recommended on Bottineau Transitway for all signalized intersections in Segments B and C where operating speeds exceed 35 mph . For all other segments, priority operations are recommended.

## OTHER RESOURCES

The following list of resources provides more in-depth technical discussion of priority and preemption operations.

1. Venglar, Jacobson, Sunkari, Engelbrecht, and Urbanik. Guide for Traffic Signal Preemption Near Railroad Grade Crossing. Report FHWA/TX-01/1439-9, September 2000.
2. Bauer and Fuller. An Evaluation of Light Rail Transit Signal Control Options. Completed for City of Phoenix, 2001. Document available at: http:// www.ptvamerica.com/ fileadmin/files_ptvamerica.com/ library/ 2002\%20ITE\%20LRT\% 20Signal\%20Control.pdf
3. Urbanik. Hiawatha LRT Preemption Evaluation. Completed for City of Minneapolis, October 2010.
4. Institute of Transportation Engineers, Preemption of Traffic Signals At or Near Railroad Grade Crossings with Active Control Devices. Committee TENC-99-06, 2003.
5. Korve Engineering, Light Rail Service: Pedestrian and Vehicular Safety. Transportation Research Board Transit Cooperative Research Program Report 69, 2001.

## Memorandum

Date:
To: Brent Rusco, P.E., Joe Gladke, P.E.
Hennepin County Regional Railroad Authority
From:

Subject: Bottineau Transitway Draft Environmental Impact Statement TH 55/7th Street/6th Avenue Intersection - Traffic Analysis

This memorandum presents the results of the traffic operations analysis for the Trunk Highway (TH) 55/7th Street/ 6th Avenue intersection analysis, which is part of the Bottineau Transitway within the City of Minneapolis. This memorandum includes a brief review of the previous modeling work completed at this intersection and the 2030 modeling results with geometric changes at the TH 55/7th Street/ 6th Avenue intersection.

This document is intended for review and consideration by the technical staff of the Minneapolis Focused Issue Group (FIG), and is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor.

## Previous 2030 Operations Modeling

Previous modeling work for the TH 55/ $7^{\text {th }}$ Street/ $6^{\text {th }}$ Avenue intersection is described in detail in the following documents:

- Bottineau Transitway Traffic Study Report, Trunk Highway 55 (Olson Memorial Highway) Corridor, WSB and Associates, November 10, 2009
- The Interchange, Existing and 2030 No Build Analysis Traffic Technical Memorandum \#1, Kimley-Horn and Associates, January 25, 2011
- The Interchange, 2030 Build Analysis Traffic Technical Memorandum \#2, Kimley-Horn and Associates, March 7, 2011
- The Interchange, 2030 Build Analysis Update Traffic Technical Memorandum \#3, KimleyHorn and Associates, August 22, 2011

The previous analysis included a number of intersections along the proposed Bottineau Transitway alignment on TH 55 and around the site of The Interchange, which is approximately located in the southeast quadrant of the TH 55/7th Street/ 6th Avenue intersection. As part of the modeling analysis for The Interchange project, TH 55/7th Street/ 6th Avenue was identified as a critical intersection because it has high traffic volumes, serves as a critical entry point into downtown, and was previously shown to have potential significant traffic operations impacts due to an at-grade LRT alignment.

The operations analysis completed for The Interchange project identified the following key findings:

- Operating both Bottineau LRT and Southwest LRT with at-grade crossings of TH 55/7th Street/ 6th Avenue would be expected to result in LOS E/F operations during both peak hours in 2030. Even with significant capacity improvements, it is unlikely that the intersection would operate at LOS D or better.
- Operating either Bottineau LRT or Southwest LRT with an at-grade crossing of TH 55/7th Street/ 6 th Avenue would be expected to result in LOS E operations in one or both peak hours. Some capacity improvements would likely be needed to improve the overall intersection to LOS D or better.

Based on the previous analysis, the major operational concerns that were identified were:

- Northbound/ southbound split phasing necessitated by the northbound shared left/ through lane. A single northbound left-turn lane would not provide adequate capacity, and geometric changes would be needed to provide two exclusive northbound left-turn lanes. The northbound left-turn is a critical movement in the PM peak.
- Reduction in capacity for the eastbound right-turn movement, compared to existing conditions. This movement currently has two exclusive turn lanes, whereas in the 2030 Build conditions with Bottineau Transitway assumed to be center running on TH 55, the eastbound approach would be limited to through, shared through/right, and right-turn lanes.

These issues are the basis for the analysis of mitigation at the TH 55/7th Street/ 6th Avenue.

## 2030 MITIGATION SCENARIO ANALYSIS

The purpose of the current analysis is to determine what geometric or other mitigations would be necessary to provide LOS D or better operations at the TH 55/7th Street/6th Avenue intersection. The 2030 forecasts and operations modeling for the current modeling effort are based on the following assumptions:

- Background growth rates were previously developed for the study area by WSB and Associates as part of the 2009 Bottineau Transitway Traffic Study Report. The growth rates were based on the Metropolitan Council travel demand model and the anticipated population and employment growth in downtown Minneapolis. The growth rates developed by WSB in the previous study were applied to the 2010 peak hour counts conducted for The Interchange project to produce 2030 peak hour forecasts.
- Development of The Interchange site, including up to 425 on-site parking spaces and grade separation of all LRT at the Hennepin Energy Recovery Center (HERC) entrance.
- Hiawatha LRT operating three-car trains at 7.5-minute headways in each direction, with an end-of-line station at The Interchange.
- Central Corridor LRT operating three-car trains at 7.5-mintue headways in each direction.
- Southwest LRT operating three-car trains at 7.5-minute headways in each direction. Central Corridor LRT would be interlined with Southwest LRT, meaning that Southwest LRT trains would continue onto the Central Corridor line and vise versa (i.e., Southwest and Central

Corridor trains do not reverse direction when they reach the The Interchange). Southwest LRT is assumed to have a grade-separated crossing of 7th Street.

- Bottineau Transitway operating three-car trains at 7.5-minute headways. Assumed to operate on a center running alignment on TH 55, with a diagonal at-grade crossing at the TH 55/7th Street/ 6th Avenue intersection. Previous modeling included both at-grade and gradeseparated operations at TH 55/7th Street/6th Avenue.
- All modeling was completed in VISSIM to simulate the impacts of transit and pedestrians on the study intersections.
- Transit priority operating scheme at the TH 55/7th Street/6th Avenue based on the anticipated transit vehicle operating speeds and the negative traffic operations impacts previously identified under a preemption scheme.
The geometrics that were previously analyzed, including Bottineau Transitway crossing the intersection at-grade, and the 2030 peak hour forecast volumes are shown in Figure 1. As shown in the figure, the TH 55/ $7^{\text {th }}$ Street/ $6^{\text {th }}$ Avenue intersection is skewed, but for the purposes of this discussion the TH 55/ $6^{\text {th }}$ Avenue approaches will be considered to be east/ west and the 7 th Street approaches will be considered to be north/ south. The skewed intersection geometrics are also relevant to the analysis because they increase pedestrian crossing distances (and therefore required signal phase times), increase the vehicle clearance times for the signal, require a longer phase to clear the train from the intersection, and also result in slower turning speeds for some movements, especially larger vehicles. The existing right-turn channelization island in the southeast quadrant of the intersection allows for large vehicles to make the northbound to eastbound right-turn movement. However, the island is a concern for the Bottineau Transitway project because the free right-turn movement is not compatible with a rail crossing at this location. The detailed design of the rail alignment through this area, and the control of the vehicle and pedestrian conflicts at the rail crossing, will be further explored as part of the more advanced design work yet to be completed for the Draft Environmental Impact Statement. As part of the current modeling effort at the intersection, the northbound right-turn movement was assumed to operate as a signalized movement, with no right turn allowed during the red signal phase.

In addition to providing acceptable operations for the overall intersection in 2030, it is desired that the individual movements also have generally have acceptable delays and queues that do not impact upstream intersections. A concept geometric layout was developed to provide two exclusive northbound left-turn lanes, as shown in Figure 2. For this analysis, Southwest LRT was assumed to operate grade-separated at 7th Street and Bottineau LRT was assumed to operate at-grade.
In addition to the need for a second northbound left-turn lane for capacity reasons, a southbound leftturn lane was added to provide better lane alignment for the through movements and to take advantage of the left-turn phasing. The City of Minneapolis striped bike lanes on 7 th Street in late 2011, which are also represented on the layout. To avoid right-of-way impacts, minimum lane widths of 11 feet for through lanes, 10 feet for turn lanes, and 6 feet for bike lanes was used. The existing roadway width on $7^{\text {th }}$ Street is 63 feet north of TH $55 / 6^{\text {th }}$ Avenue and 71 feet south of TH $55 / 6^{\text {th }}$ Avenue. Therefore the roadway section would need to widen by approximately three feet for the length of the southbound left-turn lane, with the north curb line assumed to be held constant, and by about five feet for the length of the second northbound turn lane, with the south curb line assumed to be held constant. Potential right-of-way acquisition for the segment south of TH 55/ 6 th Avenue will be explored further during the DEIS process, but would not be expected to impact any adjacent buildings or other private infrastructure.
The intersection operations and approach delays would also be expected to be improved by adding a second eastbound right-turn only lane on TH 55 at 7 th Street. However, this improvement would be

## BottineauTransitway <br> DRAFT ENVIRONMENTAL IMPACT STATEMENT

dependent on the removal of the segment of Royalston Avenue parallel to TH 55 , which is not part of the Bottineau Transitway project. Since acceptable intersection operations are not dependent on this improvement and it would be an independent project as part of the North Loop Small Area Plan, the addition of the second right-turn lane has not been assumed for this analysis.

The LOS results of the modeling with the improved geometrics and signal operations are shown in Table 2 and the expected queuing is shown in Table 3.
Table 2. 2030 LOS Results - Improved Geometry/ Signal Operations

| Peak <br> Hour | Approach | Operations By Movement |  |  |  |  |  | Overall Intersection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left |  | Through |  | Right |  | Delay (sec/veh) | LOS |
|  |  | Delay (sec/veh) | LOS | $\begin{gathered} \hline \text { Delay } \\ (\mathrm{sec} / \mathrm{veh}) \end{gathered}$ | LOS | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | LOS |  |  |
| AM | NB | 58.2 | E | 50.2 | D | 54.6 | D | 42.9 | D |
|  | EB | 0.0 | A | 28.2 | C | 11.3 | B |  |  |
|  | SB | 65.6 | E | 78.2 | E | 70.3 | E |  |  |
|  | WB | 166.1 | F | 10.9 | B | 4.1 | A |  |  |
| PM | NB | 60.3 | E | 51.3 | D | 51.8 | D | 38.3 | D |
|  | EB | 0.0 | A | 29.1 | C | 6.9 | A |  |  |
|  | SB | 24.2 | C | 43.2 | D | 50.9 | D |  |  |
|  | WB | 111.4 | F | 24.3 | C | 10.9 | B |  |  |

Table 3. 2030 Queuing Results - Improved Geometry/Signal Operations

| Peak <br> Hour | Approach | Movement Queuing |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left |  | Through |  | Right |  |
|  |  | 95th \% <br> Queue (feet) | Available Storage (feet) | 95th \% <br> Queue (feet) | Available Storage (feet) | 95th \% <br> Queue <br> (feet) | Available Storage (feet) |
| AM | NB | 149 | 420 | 149 | 640 | - | - |
|  | EB | - | - | 275 | 330 | 153 | 337 |
|  | SB | 121 | 175 | 573 | 490 | - | - |
|  | WB | 234 | 100 | 38 | 650 | 39 | 100 |
| PM | NB | 302 | 420 | 302 | 640 | - | - |
|  | EB | - | - | 126 | 330 | 28 | 337 |
|  | SB | 42 | 175 | 135 | 490 | - | - |
|  | WB | 195 | 100 | 262 | 650 | 24 | 100 |

The results show that with the proposed geometric changes, the TH $55 / 7^{\text {th }}$ Street/ $6^{\text {th }}$ Avenue intersection would be expected to operate at LOS D or better during peak hours. However, some movements may still be expected to operate at LOS E or LOS F. Movements such as the southbound approach in the AM peak that is projected to operate at LOS E can likely be further improved through the signal timing and phasing. As part of the current analysis, only a cursory evaluation of signal timing splits, sequencing, and offsets was conducted, and operations could likely be improved by
further refining the signal timing parameters. These improvements to the signal operations should be explored further during the preliminary and advanced preliminary design phases.

The westbound left-turn movement would likely continue to operate at LOS F during both peak hours due to very low approach volumes, the long cycle length, and the protected-only phasing. For lowvolume movements, there are typically only one or two vehicles making the movement per signal cycle. This means that most vehicles making this movement will likely arrive on a red signal phase and because of the protected phasing cannot take advantage of the green signal phase for the through movements. With cycle lengths of $120-150$ seconds, a vehicle could wait $60-120$ seconds for the left-turn phase to come up in the next cycle. Given the relatively high delays expected, there are two possible options:

- Provide for the westbound left-turn movement at the intersection due to the lack of readily available alternative routes for a westbound vehicle that is forced to go through at the intersection instead of turning left. Vehicles could use Border Avenue or Oak Lake Avenue-7 th Street to travel towards downtown, but these are relatively circuitous routes and the resulting increase in travel time may actually be greater than the delay that would have been experienced at the TH 55/7 th Street/ $6^{\text {th }}$ Avenue $N$ intersection. The advantage of continuing to allow the left-turn movement is that it provides drivers the option to divert prior to this intersection if there is a convenient alternative route (particularly for commuters or drivers familiar with downtown) or to make the left-turn movement at TH $55 / 7^{\text {th }}$ Street/ 6 th Avenue if there is not another convenient route from their origin to downtown or for drivers unfamiliar with downtown that may not be aware of alternative routes.
- Ban the westbound left-turn movement and either sign an alternative route east of the intersection or use signing prior to the intersection to try to capture drivers destined for downtown. Due to the number of potential origins for the westbound left-turn movement, it likely would not be possible to capture every driver prior to arriving at the intersection, which would result in circuitous rerouting to direct them back towards downtown. However, the primary advantage of this option is that it would eliminate a signal phase at the intersection and therefore improve the overall intersection delay and operations.


## SUMMARY

The following geometric changes are recommended for the TH 55/ $7^{\text {th }}$ Street/ $6^{\text {th }}$ Avenue intersection in order to provide LOS D or better intersection operations with Bottineau LRT assumed to operate atgrade:

- Provide a second northbound left-turn lane, as well as an opposing southbound left-turn lane.
- Operate the northbound and southbound approaches with concurrent (rather than split) phasing.

The addition of the northbound and southbound left-turn lanes will require roadway widening for the length of the turn lanes. Right-of-way impacts due to the widening will need to be explored further in the DEIS process. In addition, the design of the LRT alignment through the right-turn channelization
island at the TH 55/ $7^{\text {th }}$ Street/ $6^{\text {th }}$ Avenue intersection needs to be analyzed further due to potential vehicle and pedestrian conflicts at the LRT crossing.

If the segment of Royalston Avenue parallel to TH 55 is removed, consistent with the North Loop Small Area Plan, the addition of a second eastbound right-turn lane would be expected to further improve intersection operations. However, achieving LOS D or better operations at the TH 55/7 th Street/ $6^{\text {th }}$ Avenue intersection is not expected to be dependent on this improvement.


BottineauTransitway 2030 PEAK HOUR TURNING MOVEMENT VOLUMES


## BottineauTransitway <br> DRAFT ENVIRONMENTAL IMPACT STAIEMENT

## Memorandum

Date:
To: Brent Rusco, P.E., J oe Gladke, P.E. Hennepin County Regional Railroad Authority

From:
Steve Wilson, SRF Consulting Group
Joshua Maus, P.E., SRF Consulting Group
Alex Fox, P.E., SRF Consulting Group

This technical memorandum contains forecasts for both two-lane and four-lane scenarios on CSAH 103 (Broadway Avenue), which were analyzed as part of the CSAH 103 reconstruction project led by Hennepin County Transportation. Independent from the Bottineau Transitway Project, Hennepin County Transportation has determined that a four-lane section will be constructed on CSAH 103. Therefore only the four-lane forecasts have been used for the modeling of the Bottineau Transitway Project.

Subject: Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study Travel Demand Forecasts: Four-Lane and Two-Lane Alternatives

## Introduction

The purpose of this study is to develop 2030 peak hour turning movements for key intersections along CSAH 103 (Broadway Avenue) from 76th Avenue to TH 610 for a four-lane and two-lane roadway alternative. In addition, a comparison of forecast traffic volumes between the two alternatives was conducted for CSAH 103 (Broadway Avenue) and other roadways within the project area.
Currently, CSAH 103 (Broadway Avenue) is a four-lane facility from CSAH 152 (Brooklyn Boulevard) to CSAH 109 (85th Avenue), a two-lane facility between CSAH 109 (85th Avenue) and CSAH 30 (93rd Avenue), and a four-lane facility between CSAH 30 ( 93 rd Avenue) and TH 610. Under the four-lane alternative, the existing two-lane section would be widened to a four-lane facility. Under the two-lane alternative, the four-lane section between Candlewood Drive and CSAH 109 (85th Avenue) would be converted to a two-lane facility and the current two-lane section between CSAH 109 (85th Avenue) and CSAH 30 ( 93 rd Avenue) would remain a two-lane roadway.
Due to the recent conversion of the signalized intersections at TH 169/CSAH 81 (Bottineau Boulevard) and TH 169/CSAH 109 (85th Avenue) into grade-separated interchanges, new traffic data was collected by WSB and Associates, Inc. in September 2011.
Forecasts developed as described in this memorandum were subsequently used in operations analysis and simulation of alternatives.

## Existing Volumes

Existing traffic volumes provided by WSB and Associates, Inc. (September 2011) were utilized to develop future year forecasts. A comparison of this recent data to previously collected data (2005 and 2009) indicates the following:

- Current (2011) daily traffic volumes along the corridor are similar to 2005 data identified in the Brooklyn Park Transportation Plan and the Hennepin County Special Project Analysis Report (SPAR) for CSAH 103 (Broadway Avenue) prepared in May, 2007. However, the recent data does show an increase of approximately 5,000 vehicles on CSAH 103 (Broadway Avenue) north of CSAH 30 (93rd Avenue) when compared to the 2005 data.
- During the a.m. and p.m. peak hours in the peak direction of travel, the current traffic along the corridor is approximately 200-300 vehicles less than counts from 2005 and 2009. However, the current (2011) peak hour volume in the off-peak direction is similar to counts from 2005 and 2009. In addition, the current (2011) daily traffic volumes along the corridor are similar to 2005 data and 2009 data south of CSAH 30. Therefore, it is assumed that the peak hour, peak

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direction reduction can be accounted for by traffic diversion to TH 169 as a result of the conversion of the signalized intersections at TH 169/CSAH 81 (Bottineau Boulevard) and TH 169/ CSAH 109 ( $85^{\text {th }}$ Avenue) into grade-separated interchanges.

## Forecast Method and Validation

Travel demand forecasts for the year 2030 were developed for the two-lane and four-lane alternatives using the Twin Cities Regional Travel Demand model as modified for use in the Bottineau Transitway Draft EIS (DEIS). A horizon year of 2030 is consistent with the adopted comprehensive plans as well as the horizon year of the DEIS. The regional model was modified for the DEIS to incorporate additional zone and network detail in the project area to provide better fidelity for transit and roadway analyses. Consequently, it was suitable for use in the CSAH 103 (Broadway Avenue) analysis. A portion of the network showing modeled zones and roadways is shown in Figure 1.

Figure 1: Travel Demand Model Detail in CSAH 103 (Broadway Avenue) Study Area


The travel demand model is an advanced version of a "four-step" model (trip generation, trip distribution, mode choice and route assignment) in its treatment of peak and offpeak travel characteristics, estimation of high-occupancy (HOV) and MnPASS use and equilibrium assignment algorithms. The activity-generator is socioeconomic data (population, household and employment along with selected special generators) as opposed to straight land use-based vehicle trip rates.

The travel demand model was validated to existing conditions by comparing published traffic flow volumes from Hennepin County and the Minnesota Department of Transportation to daily traffic volumes estimated from the model. Figure 2 shows a comparison of the percent error of modeled volumes compared to counts. Twelve of 17 links in the study area with traffic counts ( 71 percent) were found to fall within acceptable error limits specified by the FHWA Model Validation and Reasonableness Checking Manual. The study area is relatively small, therefore broader statistical measures of modeling fit would not provide meaningful results due to small sample sizes.

Figure 2: CSAH 103 (Broadway Avenue) Study Area Model Validation


The model validation was used to identify manual adjustments to be applied to the model results where necessary. The procedures used for the adjustments and factoring of peak and turn volumes are consistent with NCHRP-255 Highway Traffic Data for Urbanized Area Project Planning and Design.

## Traffic Forecasts

Traffic forecasts (2030 four-lane and two-lane) were developed using the Met Council's Regional Travel Demand Model along with existing traffic volumes provided by WSB and Associates, Inc.
Land use assumptions for the City of Brooklyn Park are consistent with those identified under 'Alternative B' in the City of Brooklyn Park Transportation Plan which is being assumed in the Bottineau Transitway Draft EIS ridership forecasts and is the comprehensive plan alternative accepted by the Metropolitan Council. In the vicinity of the study area along CSAH 30 ( 93 rd Avenue North) area between TH 169 and CSAH 14 (Zane Avenue) the City of Brooklyn Park has approved two
development proposals (Astra and Gateway) at higher intensities than assumed in the regionallyaccepted comprehensive plan. These proposals, if developed at maximum allowed intensities, would likely generate significantly more traffic than assumed in the City's Comprehensive Plan and in the DEIS forecasts. However, the City's land use plan is a guide and other areas may develop at lower intensities than anticipated. Based on discussions with the City of Brooklyn Park, the land use in the travel demand model was not modified due to these two developments.

Within the travel demand model, the future roadway network (2030) assumes the following within the study area:

- Candlewood Drive is extended west of CSAH 103 (Broadway Avenue) to 79th Avenue (Planned Roadway Improvement, City of Brooklyn Park Comprehensive Plan)
- Completion of TH 610 from CSAH 81 (Bottineau Boulevard) to l-94 (Commitment, MnDOT HIP)
- Full access intersection at TH 169 and 101st Avenue North
- Existing full access intersection at TH 169 and CSAH 30 (93rd Avenue)

The TH 169/93rd Avenue intersection was modeled according to the Metropolitan Council's default 2030 highway network and TIP, which is a full access at-grade intersection. However, the CSAH 30 ( 93 rd Avenue) interchange has been recently programmed as a partial access interchange with ramps to/ from the south. The loss of access to and from north TH 169 would be expected to increase the 2030 forecast volumes on CSAH 103 (Broadway Avenue) between TH 610 and CSAH 30 ( 93 rd Avenue) with traffic oriented between TH 169 to the north and CSAH 30 ( 93 rd Avenue) to the east diverted to TH 610 and CSAH 103 (Broadway Avenue) north of CSAH 30 ( 93 rd Avenue).

The TH 169/101st Avenue intersection was modeled according to the Metropolitan Council's default 2030 highway network, which assumed a full access intersection consistent with the intersection configuration in 2010. However, while it is currently a right-in/ right-out intersection, an applicattion has been made to construct an interchange, which would restore full access. If the current rightin/ right-out intersection configuration was assumed for 2030, an increase in traffic on Broadway Avenue at TH 610 would be expected compared to the reported forecast volumes that assumed a full access interchange at TH 169/101st Avenue.
For the purpose of this analysis a "No Build" transit system was assumed since ridership forecasts had not yet been developed for the transit alternatives. Subsequent sensitivity tests showed that the expected reduction in vehicle traffic on CSAH 103 (Broadway Avenue) due to Bottineau Transitway was no greater than 500 vehicles per day.
The 2030 traffic forecasts were compared to the current volumes and peaking characteristics of CSAH 103 (Broadway Avenue). The results indicate that the percentage of traffic carried in the peak hour will decrease by year 2030, and the peak directionality of the corridor will also decrease by year 2030. This is typical of arterial traffic patterns as volumes increase.

## Four-Lane Altemative Forecast Analysis

The four-lane alternative assumes roadway capacities on CSAH 103 of 1,500 to 1,700 vehicles per hour per direction, with the lower values assumed between CSAH 30 ( 93 rd Avenue North) and Candlewood Drive due to access and right-of-way patterns. The results of the four-lane analysis indicate the following:
Projected (2030) daily traffic volumes between CSAH 152 and CSAH 30 are similar to previously published volumes in the City of Brooklyn Park Transportation Plan and the SPAR for CSAH 103 (Broadway Avenue). However, the forecast of 28,000 ADT vehicles per day north of CSAH 30 is 4,000 to 8,000 vehicles per day larger than the previously published volumes ( 24,000 vehicles per day).

The increase in the forecast is is consistent with the change in existing volumes between 2005 and 2011.

Peak hour traffic volumes are expected to increase by the following annual traffic growth rates:

- CSAH 103 (Broadway Avenue)
- South of CSAH 152 (Brooklyn Boulevard): 1.25-1.50\%
- Between CSAH 109 (85th Avenue) and CSAH 30 (93rd Avenue):
1.50-2.00\%
- North of CSAH 30 (93rd Avenue):
2.50-3.50\%
- CSAH 152 (Brooklyn Boulevard) and CSAH 109 (85th Avenue):
0.75-1.25\%
- CSAH 30 (93rd Avenue):
2.50-3.50\%

These annual growth rate values are generally consistent with those recommended in the SPAR for CSAH 103 (Broadway Avenue).

## Two-Lane Altemative Forecast Analysis

The two-lane alternative assumes roadway capacities on CSAH 103 of 750 to 850 vehicles per hour per direction, with the lower values assumed between CSAH 30 ( 93 rd Avenue North) and Candlewood Drive. Results of the two-lane analysis indicate the following:

- Future (2030) traffic demand for the CSAH 103 (Broadway Avenue) corridor will exceed the daily planning-level capacity of a two lane facility by 30 percent.
- To avoid congestion and reduced speeds along the CSAH 103 (Broadway Avenue) corridor that would be expected to result from the two-lane alternative, approximately 3,000 daily vehicles between CSAH 152 (Brooklyn Boulevard) and CSAH 30 (93rd Avenue) will divert from CSAH 103 (Broadway Avenue) to other roadway facilities.
- During the a.m. and p.m. peak hours approximately 400-500 vehicles between CSAH 152 (Brooklyn Boulevard) and CSAH 30 ( 93 rd Avenue) will divert to other roadway facilities to avoid congestion. Most of the diverted vehicles are traveling in the peak direction and are traveling through the corridor, not having a local origin or destination.
- A majority of the diverted traffic will use TH 169 ( $45 \%$ ). The current 2030 forecast volume of 65,000 on TH 169 is consistent with current regional modeling and would result in available capacity. The 2030 forecast for TH 169 as shown in he Brooklyn Park Transportation Plan is 80,000 vehicles per day, which would not provide sufficient reserve capacity to accommodate additional traffic.
- The remaining traffic will use CSAH 14 (Zane Avenue) (15\%), CSAH 81 (Bottineau Boulevard) ( $10 \%$ ), Wyoming Avenue (10\%), and Noble Avenue (10\%). The remaining $10 \%$ will use other local roadways in the area.
Figure 3 graphically depicts the estimated year 2030 traffic diversions resulting from the two-lane alternative, which are detailed in Table 1.
A future year (2030) planning-level capacity analysis was completed for the roadways identified above that are expected to carry diverted traffic from CSAH 103 (Broadway Avenue) under the twolane alternative. This is a high-level analysis of available daily traffic volumes from the Brooklyn Park Transportation Plan. Additional data collection and analysis would be needed to determine if operational issues will develop during peak hour conditions.

Table 1
CSAH 103 (Broadway Avenue) Two-Lane Alternative Daily Traffic Diversions Between CSAH 109 (85th Avenue) and CSAH 30 (93rd Avenue) - Screen Line Analysis

|  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Roadway | Existing <br> $(2005)^{*}$ | Forecast <br> $2030 *$ | Forecast <br> Diversion | Forecast <br> 2030 with <br> Diversion | Planning Level <br> Capacity** |
| TH 169 | 56,000 | 65,000 | 1,350 | 66,350 | 80,000 |
| CSAH 14 (Zane Ave.) | 12,700 | 15,000 | 450 | 15,450 | 28,000 |
| CSAH 81 | 23,900 | 39,000 | 300 | 39,300 | 42,000 |
| Wyoming Ave. | 4,850 | 5,300 | 300 | 5,600 | 10,000 |
| Noble Ave. | 13,700 | 19,500 | 300 | 19,800 | 28,000 |
| Other Local Roadways | N/A | N/A | 300 | N/A | N/A |

* Traffic volumes are from the Brooklyn Park Transportation Plan (2008)
** Planning level capacities are consistent with those identified in the Brooklyn Park Transportation Plan
The projected (2030) peak hour turning movements (four-lane and two-lane) were developed based on the daily forecasts from the Met Council Travel Demand Model, but were adjusted to reflect access modifications assumed in the Bottineau LRT alignment plan and consistent with NCHRP 255 as described above. This mainly consists of rerouting movements that will be restricted to right-in/rightout access in the future. Turning movements for existing (2011), 2030 four-lane and 2030 two-lane conditions are shown in the attached tables.

Tables showing the projected turn movements under each alternative are shown in the attachments to this memorandum.


Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
Existing (2011) Turning Movement Counts

## AM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 25 | 219 | 16 | 24 | 577 | 95 | 15 | 4 | 9 | 14 | 30 | 29 |
| CSAH 103 and CSAH 152 | 2 | 34 | 196 | 33 | 59 | 604 | 399 | 143 | 360 | 13 | 79 | 454 | 84 |
| CSAH 103 and Retail Access | 3 | 19 | 398 | 6 | 8 | 1042 | 35 | 19 | 4 | 9 | 11 | 1 | 2 |
| CSAH 103 Candlewood Drive | 5 |  | 397 | 22 | 11 | 1012 |  |  |  |  | 73 |  | 32 |
| CSAH 103 and College Park Drive | 7 | 14 | 415 |  |  | 963 | 6 | 1 |  | 60 |  |  |  |
| CSAH 103 and 84th Avenue | 9 | 3 | 339 | 74 | 131 | 933 | 6 | 5 | 1 | 6 | 30 | 0 | 13 |
| CSAH 103 and CSAH 109 | 10 | 73 | 187 | 97 | 70 | 667 | 173 | 81 | 352 | 155 | 248 | 467 | 24 |
| CSAH 103 and Maplebrook Parkway | 11 | 4 | 273 | 15 | 3 | 847 | 7 | 15 | 0 | 15 | 48 | 4 | 16 |
| CSAH 103 and Setzler Parkway | 13 | 18 | 268 | 18 | 15 | 767 | 23 | 2 | 0 | 2 | 88 | 0 | 27 |
| CSAH 103 and 92nd Avenue | 14 | 14 | 274 | 9 | 8 | 789 | 80 | 5 | 0 | 2 | 14 | 0 | 25 |
| CSAH 103 and CSAH 30 | 15 | 25 | 255 | 24 | 6 | 729 | 410 | 70 | 95 | 44 | 104 | 136 | 22 |
| CSAH 103 and 94th Avenue | 16 |  |  |  |  |  |  |  |  |  |  |  |  |
| CSAH 103 and TH 610 S Ramp | 17 |  | 131 | 216 | 201 | 1025 |  |  |  |  | 120 |  | 109 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 194 | 46 | 27 | 545 |  |  |  |  | 681 |  | 72 |

Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
Existing (2011) Turning Movement Counts

## PM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 65 | 473 | 50 | 101 | 259 | 174 | 150 | 47 | 51 | 37 | 62 | 79 |
| CSAH 103 and CSAH 152 | 2 | 60 | 454 | 188 | 135 | 357 | 212 | 393 | 648 | 59 | 118 | 587 | 200 |
| CSAH 103 and Retail Access | 3 | 94 | 889 | 64 | 24 | 591 | 86 | 154 | 25 | 68 | 45 | 21 | 67 |
| CSAH 103 Candlewood Drive | 5 |  | 1002 | 108 | 16 | 643 |  |  |  |  | 58 |  | 30 |
| CSAH 103 and College Park Drive | 7 | 50 | 982 |  |  | 620 | 14 | 8 |  | 39 |  |  |  |
| CSAH 103 and 84th Avenue | 9 | 3 | 931 | 56 | 96 | 543 | 3 | 6 | 0 | 0 | 91 | 2 | 80 |
| CSAH 103 and CSAH 109 | 10 | 127 | 575 | 315 | 68 | 350 | 67 | 325 | 757 | 85 | 207 | 485 | 46 |
| CSAH 103 and Maplebrook Parkway | 11 | 20 | 859 | 67 | 9 | 444 | 25 | 15 | 3 | 12 | 29 | 2 | 13 |
| CSAH 103 and Setzler Parkway | 13 | 16 | 767 | 104 | 28 | 394 | 10 | 20 | 0 | 42 | 42 | 1 | 11 |
| CSAH 103 and 92nd Avenue | 14 | 11 | 780 | 7 | 5 | 409 | 5 | 34 | 1 | 20 | 3 | 2 | 6 |
| CSAH 103 and CSAH 30 | 15 | 32 | 738 | 50 | 19 | 334 | 74 | 321 | 176 | 50 | 35 | 80 | 47 |
| CSAH 103 and 94th Avenue | 16 |  |  |  |  |  |  |  |  |  |  |  |  |
| CSAH 103 and TH 610 S Ramp | 17 |  | 445 | 661 | 107 | 386 |  |  |  |  | 41 |  | 45 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 357 | 133 | 108 | 258 |  |  |  |  | 235 |  | 151 |

Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
2030 4-Lane Alternative - Turning Movement Projections
AM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 30 | 275 | 25 | 35 | 690 | 110 | 20 | 10 | 15 | 20 | 35 | 40 |
| CSAH 103 and CSAH 152 | 2 | 65 | 230 | 40 | 115 | 710 | 445 | 190 | 435 | 15 | 110 | 530 | 115 |
| CSAH 103 and Retail Access ${ }^{(1)}$ | 3 |  | 525 | 10 |  | 1255 | 45 |  |  | 15 |  |  | 15 |
| CSAH 103 Candlewood Drive ${ }^{(2)}$ | 5 | 10 | 510 | 20 | 20 | 1240 | 100 | 50 | 20 | 10 | 50 | 35 | 45 |
| CSAH 103 and College Park Drive ${ }^{(1)}$ | 7 | 30 | 535 | 40 | 155 | 1255 | 15 | 25 | 5 | 70 | 35 | 5 | 10 |
| CSAH 103 and 84th Avenue ${ }^{(1)}$ | 9 |  | 525 | 45 |  | 1415 | 15 |  |  | 10 |  |  | 15 |
| CSAH 103 and CSAH 109 | 10 | 90 | 340 | 110 | 110 | 980 | 200 | 150 | 405 | 175 | 275 | 520 | 40 |
| CSAH 103 and Maplebrook Parkway ${ }^{(1)}$ | 11 |  | 510 | 20 |  | 1270 | 15 |  |  | 20 |  |  | 30 |
| CSAH 103 and Setzler Parkway | 13 | 25 | 490 | 25 | 40 | 1105 | 35 | 10 | 5 | 5 | 175 | 20 | 40 |
| CSAH 103 and 92nd Avenue ${ }^{(1)}$ | 14 |  | 525 | 15 |  | 1170 | 90 |  |  | 10 |  |  | 35 |
| CSAH 103 and CSAH 30 | 15 | 60 | 470 | 30 | 20 | 1025 | 560 | 195 | 165 | 80 | 155 | 195 | 35 |
| CSAH 103 and 94th Avenue ${ }^{(3)}$ | 16 | 50 | 575 | 75 | 200 | 1530 | 175 | 75 | 10 | 50 | 25 | 10 | 50 |
| CSAH 103 and TH 610 S Ramp | 17 |  | 420 | 280 | 435 | 1560 |  |  |  |  | 345 |  | 500 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 750 | 170 | 390 | 1160 |  |  |  |  | 835 |  | 310 |

Notes:
(1) Projected turning movement volumes reflect access modifications (right-in/right-out) assumed in the Bottineau LRT alignment plan. Some movements may be lower than existing counts.
(2) Projected volumes assume construction of Candlewood Drive west of CSAH 103 and associated travel pattern shifts. Some movements may be lower than existing counts.
(3) Projected volumes were developed using assumed land use projections.

Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
2030 4-Lane Alternative - Turning Movement Projections
PM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 75 | 605 | 55 | 120 | 330 | 200 | 165 | 55 | 60 | 45 | 70 | 90 |
| CSAH 103 and CSAH 152 | 2 | 145 | 505 | 210 | 180 | 400 | 270 | 525 | 750 | 65 | 185 | 725 | 245 |
| CSAH 103 and Retail Access ${ }^{(1)}$ | 3 |  | 1200 | 75 |  | 775 | 105 |  |  | 75 |  |  | 75 |
| CSAH 103 Candlewood Drive ${ }^{(2)}$ | 5 | 20 | 1175 | 80 | 30 | 830 | 75 | 110 | 40 | 10 | 40 | 25 | 35 |
| CSAH 103 and College Park Drive ${ }^{(1)}$ | 7 | 60 | 1230 | 30 | 115 | 790 | 25 | 35 | 5 | 45 | 100 | 5 | 45 |
| CSAH 103 and 84th Avenue ${ }^{(1)}$ | 9 |  | 1275 | 35 |  | 925 | 15 |  |  | 5 |  |  | 50 |
| CSAH 103 and CSAH 109 | 10 | 165 | 810 | 350 | 90 | 605 | 90 | 420 | 840 | 95 | 240 | 545 | 75 |
| CSAH 103 and Maplebrook Parkway ${ }^{(1)}$ | 11 |  | 1230 | 75 |  | 770 | 35 |  |  | 15 |  |  | 20 |
| CSAH 103 and Setzler Parkway | 13 | 20 | 1115 | 115 | 60 | 665 | 20 | 30 | 5 | 50 | 90 | 15 | 20 |
| CSAH 103 and 92nd Avenue ${ }^{(1)}$ | 14 |  | 1155 | 10 |  | 720 | 15 |  |  | 25 |  |  | 15 |
| CSAH 103 and CSAH 30 | 15 | 75 | 1025 | 70 | 40 | 575 | 170 | 515 | 280 | 100 | 60 | 155 | 60 |
| CSAH 103 and 94th Avenue ${ }^{(3)}$ | 16 | 50 | 1525 | 25 | 75 | 660 | 200 | 175 | 10 | 50 | 75 | 10 | 250 |
| CSAH 103 and TH 610 S Ramp | 17 |  | 1135 | 815 | 340 | 795 |  |  |  |  | 140 |  | 410 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 1165 | 380 | 475 | 840 |  |  |  |  | 295 |  | 390 |

## Notes:

(1) Projected turning movement volumes reflect access modifications (right-in/right-out) assumed in the Bottineau LRT alignment plan. Some movements may be lower than existing counts.
(2) Projected volumes assume construction of Candlewood Drive west of CSAH 103 and associated travel pattern shifts. Some movements may be lower than existing counts.
(3) Projected volumes were developed using assumed land use projections.

Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
2030 2-Lane Alternative - Turning Movement Projections
AM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 30 | 250 | 25 | 35 | 490 | 110 | 20 | 10 | 15 | 20 | 35 | 40 |
| CSAH 103 and CSAH 152 | 2 | 65 | 205 | 40 | 65 | 510 | 270 | 165 | 435 | 15 | 110 | 530 | 115 |
| CSAH 103 and Retail Access ${ }^{(1)}$ | 3 |  | 475 | 10 |  | 830 | 45 |  |  | 15 |  |  | 15 |
| CSAH 103 Candlewood Drive ${ }^{(2)}$ | 5 | 10 | 460 | 20 | 20 | 815 | 75 | 50 | 20 | 10 | 50 | 35 | 45 |
| CSAH 103 and College Park Drive ${ }^{(1)}$ | 7 | 30 | 485 | 40 | 155 | 805 | 15 | 25 | 5 | 70 | 35 | 5 | 10 |
| CSAH 103 and 84th Avenue ${ }^{(1)}$ | 9 |  | 475 | 45 |  | 965 | 15 |  |  | 10 |  |  | 15 |
| CSAH 103 and CSAH 109 | 10 | 90 | 290 | 110 | 110 | 605 | 175 | 150 | 405 | 150 | 225 | 520 | 40 |
| CSAH 103 and Maplebrook Parkway ${ }^{(1)}$ | 11 |  | 460 | 20 |  | 870 | 15 |  |  | 20 |  |  | 30 |
| CSAH 103 and Setzler Parkway | 13 | 25 | 440 | 25 | 40 | 705 | 35 | 10 | 5 | 5 | 175 | 20 | 40 |
| CSAH 103 and 92nd Avenue ${ }^{(1)}$ | 14 |  | 475 | 15 |  | 770 | 90 |  |  | 10 |  |  | 35 |
| CSAH 103 and CSAH 30 | 15 | 60 | 420 | 30 | 20 | 700 | 585 | 195 | 165 | 55 | 105 | 195 | 35 |
| CSAH 103 and 94th Avenue ${ }^{(3)}$ | 16 | 50 | 525 | 75 | 200 | 1230 | 175 | 75 | 10 | 50 | 25 | 10 | 50 |
| CSAH 103 and TH 610 S Ramp | 17 |  | 395 | 255 | 435 | 1310 |  |  |  |  | 295 |  | 500 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 725 | 170 | 390 | 1010 |  |  |  |  | 735 |  | 310 |

Notes:
(1) Projected turning movement volumes reflect access modifications (right-in/right-out) assumed in the Bottineau LRT alignment plan. Some movements may be lower than existing counts.
(2) Projected volumes assume construction of Candlewood Drive west of CSAH 103 and associated travel pattern shifts. Some movements may be lower than existing counts.
(3) Projected volumes were developed using assumed land use projections.

Bottineau Transitway - CSAH 103 (Broadway Avenue) Traffic Study
2030 2-Lane Alternative - Turning Movement Projections
PM Peak Hour

| Intersection | Int. \# | NBL | NBT | NBR | SBL | SBT | SBR | EBL | EBT | EBR | WBL | WBT | WBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CSAH 103 and 76th Avenue | 1 | 75 | 405 | 55 | 120 | 305 | 200 | 165 | 55 | 60 | 45 | 70 | 90 |
| CSAH 103 and CSAH 152 | 2 | 145 | 305 | 210 | 180 | 375 | 245 | 350 | 750 | 65 | 185 | 725 | 195 |
| CSAH 103 and Retail Access ${ }^{(1)}$ | 3 |  | 775 | 75 |  | 725 | 105 |  |  | 75 |  |  | 75 |
| CSAH 103 Candlewood Drive ${ }^{(2)}$ | 5 | 20 | 750 | 80 | 30 | 780 | 75 | 85 | 40 | 10 | 40 | 25 | 35 |
| CSAH 103 and College Park Drive ${ }^{(1)}$ | 7 | 60 | 780 | 30 | 115 | 740 | 25 | 35 | 5 | 45 | 100 | 5 | 45 |
| CSAH 103 and 84th Avenue ${ }^{(1)}$ | 9 |  | 825 | 35 |  | 875 | 15 |  |  | 5 |  |  | 50 |
| CSAH 103 and CSAH 109 | 10 | 140 | 435 | 300 | 90 | 555 | 90 | 395 | 840 | 95 | 240 | 545 | 75 |
| CSAH 103 and Maplebrook Parkway ${ }^{(1)}$ | 11 |  | 830 | 75 |  | 720 | 35 |  |  | 15 |  |  | 20 |
| CSAH 103 and Setzler Parkway | 13 | 20 | 715 | 115 | 60 | 615 | 20 | 30 | 5 | 50 | 90 | 15 | 20 |
| CSAH 103 and 92nd Avenue ${ }^{(1)}$ | 14 |  | 755 | 10 |  | 670 | 15 |  |  | 25 |  |  | 15 |
| CSAH 103 and CSAH 30 | 15 | 50 | 675 | 45 | 40 | 525 | 170 | 540 | 280 | 100 | 60 | 155 | 60 |
| CSAH 103 and 94th Avenue ${ }^{(3)}$ | 16 | 50 | 1200 | 25 | 75 | 610 | 200 | 175 | 10 | 50 | 75 | 10 | 250 |
| CSAH 103 and TH 610 S Ramp | 17 |  | 935 | 690 | 340 | 745 |  |  |  |  | 140 |  | 410 |
| CSAH 103 and TH 610 N Ramp | 18 |  | 1015 | 330 | 475 | 815 |  |  |  |  | 270 |  | 390 |

Notes:
(1) Projected turning movement volumes reflect access modifications (right-in/right-out) assumed in the Bottineau LRT alignment plan. Some movements may be lower than existing counts.
(2) Projected volumes assume construction of Candlewood Drive west of CSAH 103 and associated travel pattern shifts. Some movements may be lower than existing counts.
(3) Projected volumes were developed using assumed land use projections.

## Memorandum

Date: May 24,2012<br>To: $\quad$ Brent Rusco, P.E., J oe Gladke, P.E. Hennepin County Regional Railroad Authority<br>From: JoNette Kuhnau, P.E., PTOE<br>Kimley-Horn and Associates, Inc.<br>Subject: Bottineau Transitway Draft Environmental Impact Statement CSAH 81 (Brooklyn Boulevard) Transit Headway Analysis

This memorandum presents the results of the traffic operations analysis of transit headways along CSAH 81 (Brooklyn Boulevard) within the City of Brooklyn Park. The memorandum includes a brief review of the purpose of the analysis, the modeling assumptions, and the 2030 analysis results.

This document is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor.

## BACKGROUND

Portions of Segment A and Segment C of the Bottineau Transitway consist of a shared freight rail/ transit corridor that runs parallel to County State Aid Highway (CSAH) 81 (Bottineau Boulevard), with crossings at four signalized intersections. Based on the presence of freight rail and the proposed transit vehicle speeds, signal preemption is proposed for each of the signalized crossings ${ }^{1}$. Both light rail transit (LRT) and bus rapid transit (BRT) modes were considered for the Bottineau Transitway. While 7.5-minute peak hour headways in each direction were assumed for LRT, these headways would not be able to accommodate the forecast ridership based on the passenger capacity of the BRT vehicles and therefore more vehicles at shorter headways would be needed to meet the forecast demand. The purpose of this analysis was to evaluate the impact of decreased transit headways on the traffic operations along CSAH 81 and determine the shortest headways that would continue to provide acceptable traffic operations. A secondary objective was also to confirm that a diagonal crossing of the transitway at CSAH 81/73rd Avenue could remain at-grade, and would not be required to be grade separated based on traffic operations and delay.

## MODELING ASSUMPTIONS

The focus of the modeling work was on BRT, due to the need for headways less than 7.5 minutes in each direction. The signalized intersections in the area of study are as follows:

- CSAH 81/73rd Avenue
- CSAH 81/71 st Avenue/ West Broadway Avenue (County Road 8)
- CSAH 81/63rd Avenue

[^10]- CSAH 81/Bass Lake Road (County Road 10)

Note that for the purposes of this discussion, CSAH 81 will be described as a north/ south corridor.
All modeling was completed for 2030 based on the Bottineau Transitway forecasting. VISSIM was chosen as the analysis tool because it has the ability to model both traffic and transit operations relatively accurately in a preemption scenario, whereas Synchro/SimTraffic does not have the capabilities to explicitly model preemption operations. Other key assumptions used in the modeling were as follows:

- Only the PM peak hour was modeled, based on the higher traffic volumes and congestion under existing and forecast conditions.
- All signalized intersections were assumed to have gated freight/ transit crossings and to operate with preemption. The length of each preemption, including flashers, gate lowering, transit vehicle crossing, and gate raising, ranges from 30-38 seconds based on allowable speeds through the intersection and the flasher and gate times specified in section 8 C .4 of the Minnesota Manual on Uniform Traffic Control Devices (MnMUTCD).
- BRT vehicles were assumed to be 38 feet long.
- Transit headways between 3.75 and 7.5 minutes were assumed in each direction, with 5-10 percent variability.
- Current freight rail traffic in the corridor consists of only one train per day. While this train could be traveling through the corridor during the peak hour, for the sake of simplicity and easier comparisons between transit alternatives, freight traffic was not included in the model.
- CSAH 81 was assumed to have been reconstructed as a six-lane divided roadway, according to the preliminary plans prepared by Hennepin County.
- The CSAH 81/73rd Avenue and CSAH 81/ Bass Lake Road intersections were chosen for the analysis to represent a low volume and high volume intersection, respectively.
- The CSAH 81/73rd Avenue intersection was modeled with a diagonal transitway crossing of the intersection, as shown in the Segment B alignment drawing, to represent the worst case traffic operations. With the geometrics of the diagonal crossing, only the northbound and southbound right-turn movements were allowed to operate concurrently with the transit phase.
- At the CSAH 81/ Bass Lake Road intersection, the northbound and southbound through movements were allowed to operate concurrently with the transit phase.

The modeling was completed using proposed 2030 geometrics provided by Hennepin County. Geometric improvements included converting CSAH 81 from a four-lane section to a six-lane section and adding northbound and southbound dual left turn lanes at Bass Lake Road.

A total of six scenarios were modeled for each intersection in the 2030 PM peak hour, as shown in Table 1.

Table 1. Transit Headway Scenarios

| Headway <br> (minutes) | CSAH 81 Signal Operations |  |
| :---: | :---: | :---: |
| 3.75 | Coordinated | Uncoordinated |
| 5.0 | Coordinated | Uncoordinated |
| 7.5 | Coordinated | Uncoordinated |

The purpose of modeling both coordinated and uncoordinated operations on CSAH 81 was to analyze two different preemption recovery scenarios. For locations where the transitway is side running, when a preemption call is received the controller is assumed to end the phase it was running, provide green time for the eastbound approach (to clear any vehicles potentially queued on the freight
rail/ transitway), and then go to the phases compatible with the rail/ transitway crossing (northbound/ southbound through movements and southbound left-turn movement). This means that the movements the northbound left-turn movement and all westbound movements are most likely to experience additional delay due to a preemption event because they are not served as part of the preemption clearance and are not compatible with the crossing movement. At the diagonal crossing at CSAH 81/73rd Avenue, it is not necessary to run the clearance phases since there is little or no risk that vehicles on the cross street approaches could be stopped on the transitway.

Under the coordinated scenario, the signal controller attempts to return to coordination as quickly as possible after a preemption event by returning to the northbound/ southbound through movements and then serving only minimum green times on all the non-coordinated phases. While this provides potentially improved traffic flow on CSAH 81, it also has the potential to have the greatest delays for the non-coordinated phases (mainline left-turn movements and all cross street movements). In general, maintaining corridor coordination is the preferred operation for Hennepin County roadways.

In the uncoordinated scenario, the signal controller does not immediately return to the coordinated phases, but is allowed to serve one of the non-coordinated phases in order to reduce overall intersection delays. In this case, the signal recovered from preemption into the northbound left-turn phase, which was most often skipped due to a preemption event.

## 2030 ANALYSIS

For the purposes of this analysis, LOS D is considered to be acceptable during the peak hour. The results of five one-hour simulations for the 2030 PM peak hour were used to produce average delay results for each scenario, which are shown in Table 2 on the following page.

The results show that at the lower volume intersection (CSAH 81/73rd Avenue), the transit headway does not have a significant impact on the overall intersection level of service. This is primarily because of the lower volumes on the cross street, which do not require long green times and also do not influence the intersection level of service as much as the high volume mainline movements. Several of the left-turn movements with long delays are due to the low volumes on those movements, which results in vehicles almost always arriving on the red and having to wait for the signal to cycle around to the left-turn phase. This would be expected to occur even without the presence of the transitway crossing. The other significant finding at the CSAH 81/73rd Avenue intersection is that
acceptable intersection operations can be maintained with an at-grade crossing of the transitway diagonally through the intersection. Based on this result, a grade separated crossing is not needed at CSAH 81/73rd Avenue as part of the Segment B alignment of the Bottineau Transitway project.

At the CSAH 81/Bass Lake Road intersection, the transit headways (frequency of preemptions) had a much more significant impact on the intersection operations. At 3.75 -minute headways and maintaining coordination on CSAH 81, the intersection would be expected to have failing operations, with many northbound and westbound vehicles waiting more than one signal cycle to be serviced. In addition, the northbound left-turn queues would be expected to back up into the mainline northbound through lanes, which would be both an operations and a safety issue. At 5.0-minute headways, the intersection has overall acceptable operations and the northbound left-turn movement no longer queues into the northbound through lanes, although several movements are still expected to operate at LOS $F$ with relatively long delays.

Table 2. 2030 Operations Results

| Headway (minutes) |  | Headway (minutes) |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | 3.75 | 5.0 | 7.5 |
|  |  | Intersection Delay ( $\mathrm{sec} / \mathrm{veh}$ ) and LOS | Intersection Delay ( $\mathrm{sec} / \mathrm{veh}$ ) and LOS | Intersection Delay (sec/ veh) and LOS |
| CSAH 81/ 73 rd Avenue |  |  |  |  |
| Coordinated | Intersection Delay (sec/veh) Level of Service <br> Comments | $\begin{gathered} 17 \\ B \end{gathered}$ <br> NB LT, SB LT, EB TH, and WB TH operate at LOS F EB LT and WB LT operate at LOS E | $\begin{gathered} 13 \\ B \end{gathered}$ <br> NB LT, SB LT, EB TH and WB TH operate at LOS F EB LT and WB LT operate at LOS E | $\begin{gathered} 12 \\ \text { B } \end{gathered}$ <br> NB LT and WB TH operate at LOS F SB LT, EB LT, EB TH, and WB LT operate at LOS E |
| Uncoordinated | Intersection Delay (sec/veh) Level of Service | $\begin{gathered} 22 \\ C \end{gathered}$ | $\begin{gathered} 18 \\ B \end{gathered}$ | $\begin{gathered} 17 \\ B \end{gathered}$ |
|  | Comments | All movements operate at LOS D or better | All movements operate at LOS D or better | All movements operate at LOS D or better |
| CSAH 81/ Bass Lake Road |  |  |  |  |
| Coordinated | Intersection Delay (sec/veh) Level of Service | $\begin{gathered} 100+ \\ F \end{gathered}$ | $\begin{gathered} 52 \\ \mathrm{D} \end{gathered}$ | $\begin{gathered} 29 \\ C \end{gathered}$ |
|  | Comments | All NB and WB movements operate with delays of $100+\mathrm{sec} / \mathrm{veh}$ | NB LT and WB TH operate <br> with delays of $100+$ $\mathrm{sec} / \mathrm{veh}$ <br> WB LT operates at LOS E | NB LT, WB LT, and WB TH operate at LOS F <br> EB LT operates at LOS E |
| Uncoordinated | Intersection Delay (sec/veh) Level of Service | $\begin{gathered} 51 \\ D \end{gathered}$ | $\begin{gathered} 29 \\ C \end{gathered}$ | $\begin{gathered} 25 \\ C \end{gathered}$ |
|  | Comments | All WB movements operate with delays of $100+\mathrm{sec} / \mathrm{veh}$ | WB LT and WB TH operate with delays of $100+\mathrm{sec} / \mathrm{veh}$ | WB LT and WB TH operate at LOS F <br> EB LT and SB LT operate at LOS E |

## SUMMARY

The CSAH 81/73rd Avenue and CSAH 81/Bass Lake Road intersections were modeled with preemption for the transitway crossing, with transit headways of $3.75,5.0$, and 7.5 minute headways to determine the impact on overall traffic operations. Based on the results of the modeling, the following recommendations are made for the Bottineau Transitway project:

- An at-grade crossing of the transitway will result in acceptable intersection operations at the CSAH 81/73rd Avenue intersection, and a grade separation of the transitway and highway is not necessary for traffic operations.
- The minimum (shortest) headways recommended are 5.0 minutes in order to maintain coordination in the CSAH 81 corridor and provide acceptable traffic operations.


## Memorandum

To: Brent Rusco, P.E.<br>Hennepin County Engineering and Transit Planning<br>From: Patrick Corkle, P.E., PTOE<br>SRF Consulting Group<br>Date: June 18, 2012<br>Subject: Bottineau Alignment A<br>Traffic Analysis Memorandum

This memorandum presents the results of the scoping level traffic operations analysis for alignment A of the Bottineau Transitway within the City of Maple Grove and Brooklyn Park. This document is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor.

The following sections focus on the traffic operational aspects of alignment A, including a summary of the existing conditions, the characteristics of the A alignment, forecast 2030 traffic volumes, and the 2030 operational modeling results for light rail transit (LRT). Additional considerations including noise analysis, parking impacts, pedestrian/bicycle circulation, property access, roadway network impacts, and emergency vehicle access, are important issues to be considered for alignment A, but are not part of the analysis documented in this memorandum.

## EXISTING CONDITIONS

The A segment for this analysis extends east-west north of County State Aid Highway (CSAH) 130 (Elm Creek Boulevard) in Maple Grove and Brooklyn Boulevard in Brooklyn Park from CSAH 61 (Hemlock Lane) to CSAH 81 (Bottineau Boulevard). At the CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) intersection, the transitway corridor turns south on the west side of CSAH 81 (Bottineau Boulevard). The focus of the operations analysis was the PM peak hour because it has the highest volumes and would therefore represent a worst case condition.

Alignment A may impact traffic operations at signalized intersections because of the potential for additional delay. Traffic signals are currently operating at the following intersections:

- CSAH 130 (Brooklyn Boulevard)/ Boone Avenue
- CSAH 81 (Bottineau Boulevard)/ CSAH 103 (Brooklyn Boulevard)
- CSAH 81 (Bottineau Boulevard)/73rd Avenue
- CSAH 81 (Bottineau Boulevard)/ CSAH 8 (71st Avenue)

The CSAH 130 (Brooklyn Boulevard)/ Northland Drive intersection was not analyzed. This intersection was considered a lower volume roadway and therefore no significant operational impacts are expected.

## Existing Traffic Volumes

The existing daily traffic volumes are shown in Table 1. The PM peak hour turning movement counts are shown in Table 2.

Table 1 Existing Daily Traffic Volumes

| Roadway | Location | Year | Daily Volume |
| :--- | :--- | ---: | ---: |
| CSAH 130 (Elm Creek Blvd) | West of TH 169 | 2010 | 17,000 |
| CSAH 130 (Brooklyn Blvd) | West of Boone Ave | 2009 | 19,800 |
| CSAH 130 (Brooklyn Blvd) | East of Boone Ave | 2009 | 18,400 |
| CSAH 81 (Bottineau Blvd) | South of CSAH 130 (Brooklyn Blvd) | 2009 | 22,400 |

Table 2 Existing PM Peak Hour Traffic Volumes

| Intersection | Count Year Agency | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| CSAH 130 (Brooklyn Blvd) at Boone Ave | 2007 | 271 | 78 | 499 | 8 | 6 | 7 | 43 | 1057 | 98 | 93 | 521 | 14 |
|  | Hennepin |  |  |  |  |  |  |  |  |  |  |  |  |
| CSAH 81 (Bottineau Blvd) at CSAH 130 (Brooklyn Blvd) | 2007 | 102 | 1354 | 212 | 159 | 578 | 40 | 232 | 832 | 61 | 205 | 446 | 141 |
|  | Hennepin |  |  |  |  |  |  |  |  |  |  |  |  |
| CSAH 81 (Bottineau Blvd) at 73rd Ave | 2011 | 18 | 1182 | 56 | 13 | 666 | 21 | 49 | 41 | 43 | 33 | 18 | 33 |
|  | SRF |  |  |  |  |  |  |  |  |  |  |  |  |
| CSAH 81 (Bottineau Blvd) at CSAH 8 (71st Ave) | 2003 | 27 | 1399 | 423 | 97 | 953 | 151 | 216 | 458 | 40 | 316 | 252 | 93 |
|  | Hennepin |  |  |  |  |  |  |  |  |  |  |  |  |

## Operations Analysis

The initial operational analysis for alignment A was completed in Synchro/ SimTraffic because it provides the ability to efficiently analyze a number of different geometric and operational parameters and more detailed analysis was completed in VISSIM where needed. Preemption can't be modeled directly in Synchro/ SimTraffic but the signal timing and phasing can be modified to reasonably approximate the impacts on traffic. VISSIM modeling more accurately replicates the transit preemption sequence and recovery, as well as accounting for truncated and skipped signal phases.

Transit is assumed to operate in a priority scheme at the CSAH 130 (Brooklyn Boulevard)/ Boone Avenue intersection, based on operating speeds of 35 miles per hour ( mph ) and below. In a priority scheme, LRT will generally operate concurrently with the through vehicle phases or as its own signal phase that will be called only when a transit vehicle is detected. In a priority scheme, it is important to note that the early green or green extension time is typically considered to be about 10 percent of the cycle, and therefore the transit vehicle still may have to stop at the signal.

Transit is assumed to operate in a preemption scheme for the intersections of CSAH 81 (Bottineau Boulevard) at 73rd Avenue and CSAH 8 (71st Avenue) under all the future operations scenarios. In a preemption scheme, when the transit vehicle approaches the intersection on the transitway, the traffic signal green phase is terminated and goes to the yellow and red phase. The approach being crossed has a green phase "clearance interval" to clear any queued vehicles from the transitway crossing. The roadway green indications rest in the nonconflicting movements, which would be the CSAH 81 (Bottineau Boulevard) through movements. The traffic movements at the 73rd Avenue intersection are fairly low, so a Synchro/ Sim-traffic model was used to model the preemption operations. VISSIM was used to model the CSAH 8 (71st Avenue) intersection due to the higher traffic volumes and potential operational impacts of preemption.

The existing traffic signal at CSAH 130 (Brooklyn Boulevard)/ Northland Drive was excluded from the existing and future operations analysis. This intersection was considered low-volume and therefore no significant operational impacts are expected.

## Existing Operations

The existing conditions PM peak hour analysis shows that the CSAH 81 (Bottineau Boulevard) intersection at CSAH 130 (Brooklyn Blvd) operates at Level of Service (LOS) E and the CSAH 81 (Bottineau Boulevard) intersection at CSAH 8 ( $71^{\text {st }}$ Avenue) operates at LOS D. Both of these intersections are nearing or at capacity. Table 3 shows the results from the existing conditions analysis.

Table 3 Existing Delay and LOS Results

| Intersection | Intersection |  | Comments |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/ veh) | LOS |  |
| CSAH 130 (Brooklyn <br> Blvd) at Boone Ave | 30.6 | C | All movements operate at LOS D or <br> better |
| CSAH 81 (Bottineau <br> Blvd) at CSAH 130 <br> (Brooklyn Blvd) | 58.8 | E | Many movements operate at LOS F, <br> operating near or at capacity |
| CSAH 81 (Bottineau <br> Blvd) at 73rd Ave | 12.3 | B | All movements operate at LOS E or <br> better |
| CSAH 81 (Bottineau <br> Blvd) at CSAH 8 (71st <br> Ave) | 50.8 | D | Some movements operate at LOS F, <br> operating near capacity |

## FUTURE CONDITIONS

Along alignment A, several modifications would be introduced in the future at the following locations:

- Alignment A through Maple Grove is under a mining operation. Therefore, none of these intersections exist (Zachary and Jefferson Highway (new alignment)) and they are expected to be planned with adequate geometrics to ensure no operational problems would occur.
- The transitway would be grade-separated with TH 169.
- The transitway would cross into a new median just west of the CSAH 130 (Brooklyn Boulevard)/ Northland Drive intersection. This intersection was not analyzed because of the low cross-street volumes and because the transitway would not impact eastbound traffic.
- The transitway would cross the Boone Avenue intersection at-grade in the median. Left-turn lanes would be added on the CSAH 130 (Brooklyn Boulevard) approaches.
- CSAH 81 (Bottineau Boulevard) will be expanded to a six-lane section as part of a planned roadway reconstruction project that is separate from the transitway project. The roadway reconstruction project includes the intersections of CSAH 130 (Brooklyn Boulevard), 73rd Avenue, and CSAH 8 (71st Avenue).
- The transitway will not impact the CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) intersection because the transitway crossing will be grade-separated.
- The transitway crosses CSAH 8 (73rd Avenue) at-grade on the west side of CSAH 81. This is currently a low volume approach. Alignment B would cross from CSAH 103 (Broadway Avenue) to CSAH 81 (Bottineau Boulevard), diagonally across the intersection. The analysis of alignment $B$ crossing at this intersection was analyzed and documented as part of a separate traffic memo.
- The transitway crosses CSAH 8 ( $71^{\text {st }}$ Avenue) at-grade on the west side of CSAH 81. The intersection includes additional cross-street improvements identified in the CSAH 81 (Bottineau Boulevard) reconstruction project, including dual left-turn lanes on each approach and right-turn lanes.


## 2030 Forecast Traffic Volumes

The forecast 2030 volumes were based on the "Bottineau Boulevard 2030 Highway and Bus Rapid Transit Ridership Forecast from March 7, 2006, by SRF Consulting Group". These are consistent with other previous work including the CSAH 81 (Bottineau Boulevard) reconstruction project and the Bottineau Transitway Alternatives Analysis "Traffic Operations Analysis Report" dated January 2010. Table 4 shows the year 2030 daily traffic volumes. Table 5 shows the 2030 PM peak hour traffic volumes.

Table 4 Year 2030 Daily Traffic Volumes

| Roadway | Location | Year | Daily Volume |
| :--- | :--- | :--- | ---: |
| CSAH 130 (Elm Creek Blvd) | West of TH 169 | 2030 | Not available |
| CSAH 130 (Brooklyn Blvd) | West of Boone Ave | 2030 | Not available |
| CSAH 130 (Brooklyn Blvd) | East of Boone Ave | 2030 | 25,000 |
| CSAH 81 (Bottineau Blvd) | South of CSAH 130 (Brooklyn Blvd) | 2030 | 38,000 |

## BottineauTransitway

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Table 5 Year 2030 PM Peak Hour Traffic Volumes

| Intersection | Forecast Year | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| (CSAH 130)/ Brooklyn Blvd at Boone Ave | 2030 | 350 | 80 | 500 | 10 | 10 | 10 | 50 | 1250 | 120 | 110 | 700 | 20 |
| (CSAH81)/ Bottineau Blvd at (CSAH 130)Brooklyn Blvd | 2030 | 240 | 1580 | 270 | 270 | 1200 | 50 | 240 | 1295 | 155 | 210 | 680 | 370 |
| (CSAH81)/ Bottineau Blvd at 73rd Ave | 2030 | 40 | 1940 | 180 | 60 | 1465 | 40 | 80 | 60 | 60 | 120 | 20 | 70 |
| (CSAH81)/ Bottineau Blvd at (CSAH 8)/71st Ave | 2030 | 35 | 1725 | 580 | 130 | 1305 | 210 | 300 | 535 | 45 | 355 | 310 | 135 |

DRAFT ENVIRONMENTAL IMPACT STATEMENT

Future Operations
Alignment A was modeled with LRT, which is assumed to operate under a transit preemption scheme for the intersections of CSAH 81 (Bottineau Boulevard) at 73rd Avenue and CSAH 8 (71st Avenue), based on the transit operating speeds ( 45 to 55 MPH ). For this analysis, LRT was assumed to operate three-car trains at 7.5 minute headways.

The transitway was modeled as operating with the through movement of the mainline. The transit preemption disrupts the phasing sequence and may result in short green times for movements conflicting with the transitway. It is possible to maintain coordination in the corridor in the future, but the frequency of preemptions would likely have operational impacts on the cross streets that would need to be weighed against the benefits to mainline traffic. The CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) intersection would not be impacted by preemptions since the transitway will be grade-separated.

For the transit signal priority operations at CSAH 130 (Brooklyn Boulevard)/ Boone Avenue, the transit movements were modeled in Synchro as a concurrent phase with the through movements.

Protected only phasing is needed to control left-turn movements for vehicles traveling parallel to the guideway that then turn left across the guideway. The through movement is generally compatible with the transit movement, but the left-turn vehicles must be protected so as not to turn across the path of an approaching transit vehicle. This is particularly a risk when a transit vehicle is approaching from the rear of the passenger vehicle. Left-turn movements from a cross street across the guideway can be either protected or permissive, as the through movements are not compatible with the transit phase. For the intersections where the transitway is side-running, right turn on red (RTOR) was banned for those rights turn movements across the transitway.

The operational results for alignment A under 7.5 -minute LRT headways are shown in Table 6.

Table 6 Year 2030 Delay and LOS Results

| Intersection | Intersection |  | Comments |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS |  |
| CSAH 103 (Brooklyn <br> Blvd) at Boone Ave | 40.9 | D | Most movements crossing the tracks <br> operate at LOS E |
| CSAH 81 (Bottineau <br> BIvd) at CSAH 130 <br> (Brooklyn Blvd) | 60.3 | E | The transitway does not impact the <br> intersection operations |
| CSAH 81 (Bottineau <br> Blvd) at 73rd Ave | 30.5 | C | Cross-street movements operate at <br> LOS E/F |
| CSAH 81 (Bottineau <br> Blvd) at CSAH 8 (71st <br> Ave) | 50.4 | D | Movements conflicting with the <br> transitway are LOS E or F |

The CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) intersection was included in the analysis to show the intersection is near the LOS D/E threshold (acceptable/ unacceptable) even though the transitway would be grade separated at this intersection. This was done to show the need for the grade separation, as the intersection is anticipated to be at or near unacceptable conditions even after significant improvements. The operation of this intersection is complicated even more by the freight rail movements.

## SUMMARY

The following is a summary of the results of the analysis:
Under existing conditions, the intersections of CSAH 81 (Bottineau Boulevard) at CSAH 130 (Brooklyn Boulevard) and CSAH 8 ( $71^{\text {st }}$ Avenue) are near the LOS D/E and capacity levels.

The alignment A northern terminus is in the gravel mining area of Maple Grove, it crosses over TH 169 into the median of CSAH 130 (Brooklyn Boulevard), and then heads south on CSAH 81 (Bottineau Boulevard). Below are the key intersections potentially impacted by the transitway:

- Zachary Lane: future roadway, assumed no impact
- Re-aligned Jefferson Highway: future roadway, assumed no impact
- TH 169: overpass, no impact
- Northland Drive: cross to median, lower volume roadway, assumed no impact
- Boone Avenue: median running transitway, analyzed
- CSAH 81 (Bottineau Boulevard): grade-separated crossing of CSAH 130 (Brooklyn Boulevard), no impact
- 73rd Avenue: side running transitway, analyzed
- CSAH 8 (71st Avenue): side running transitway, analyzed

Under year 2030 future conditions, the future CSAH 81 (Bottineau Boulevard) reconstruction project was assumed along with left-turn lane improvements at
the CSAH 81 (Bottineau Boulevard) intersections at CSAH 130 (Brooklyn Boulevard) and CSAH 8 (71st Avenue).

The analyzed intersections are expected to operate with acceptable LOS given the above improvements, except CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) will operate near unacceptable operations and near/ at capacity. The transitway would have no operational impacts to the CSAH 81 (Bottineau Boulevard)/ CSAH 130 (Brooklyn Boulevard) intersection, since it is proposed to be grade separated. The grade-separated crossing of CSAH 130 (Brooklyn Boulevard) at CSAH 81 (Bottineau Boulevard) is warranted because this intersection is expected to operate near unacceptable operation even with significant improvements provided in the future CSAH 81 (Bottineau Boulevard) reconstruction project.

## Memorandum

To: Brent Rusco, P.E.<br>Hennepin County Engineering and Transit Planning<br>From: Patrick Corkle, P.E., PTOE<br>SRF Consulting Group<br>Date: June 18, 2012<br>Subject: Bottineau Alignment C Traffic Analysis Memorandum

This memorandum presents the results of the scoping level traffic operations analysis for alignment C of the Bottineau Transitway within the Cities of Brooklyn Park, Crystal, and Robbinsdale. This document is one of a series of technical memoranda that address various traffic operations issues along the transitway corridor.

The following sections focus on the traffic operational aspects of alignment C , including a summary of the existing conditions, the characteristics of alignment C, forecast 2030 traffic volumes, and the 2030 operational modeling results for light rail transit (LRT). Additional considerations including noise analysis, parking impacts, pedestrian/bicycle circulation, property access, roadway network impacts, and emergency vehicle access, are important issues to be considered for alignment C , but are not part of the analysis documented in this memorandum.

## EXISTING CONDITIONS

Alignment C for this analysis extends north-south along the west side of County State Aid Highway (CSAH) 81 (Bottineau Boulevard) from north of I-94 to Bass Lake Road (CSAH 10). At this point, the transitway alignment leaves CSAH 81 and follows the railroad alignment to 36th Avenue, where alignment D alternatives begin. The focus of the operations analysis was the PM peak hour because it has the highest volumes and would therefore represent a worst case condition.

The C alignment may impact traffic operations at signalized intersections because of the potential for additional delay. Traffic signals are currently operating at the following intersections:

- CSAH 81 (Bottineau Boulevard)/I-94 North Ramp (no impact; not modeled)
- CSAH 81 (Bottineau Boulevard)/I-94 South Ramp (no impact; not modeled)
- CSAH 81 (Bottineau Boulevard)/ 63rd Avenue
- CSAH 81 (Bottineau Boulevard)/ CSAH 10 (Bass Lake Road) (analyzed separately)

The I-94 ramps were not analyzed, since the transitway crosses I-94 on the west side of the interchange. The l-94 interchange is a folded diamond to the east and therefore the transitway does not cross any of the ramps and would not impact the operation of the traffic signals. The CSAH 81 (Bottineau Boulevard)/ CSAH 10 (Bass Lake Road) intersection is a critical intersection with high volumes. Therefore, it was analyzed in VISSIM to best understand how the transitway and traffic signal would operate. The assumptions and results of this analysis are documented in the CSAH 81 (Brooklyn Boulevard) Transit Headway Analysis technical memorandum by Kimley-Horn and Associates, dated May 24, 2012.

Other unsignalized intersections or crossings exist along this alignment are at the following locations:

- Corvalias Avenue Crossing: non-intersection with gated crossing, not modeled
- West Broadway Avenue Crossing: non-intersection with gated crossing, not modeled
- $45 \frac{1}{2}$ Avenue Crossing: low volume/ non-intersection with gated crossing, not modeled
- TH 100: overpass, not modeled
- 42nd Avenue: non-intersection with gated crossing, analyzed separately
- 41st Avenue/ Noble Avenue: non-intersection with gated crossing, not modeled
- $39 ½$ Avenue Crossing: low volume/ non-intersection with gated crossing, not modeled

None of the non-intersections were modeled as part of this analysis. They were reviewed in the previous Alternatives Analysis work and determined not to be significantly impacted by the transitway. This is mainly because the only time the roadway traffic would be stopped is during the transitway vehicle crossing and the remaining time the roadway would be free to cross. The 42 nd Avenue crossing was previously analyzed, with the assumptions and results documented in the Operations Analysis of LRT and BRT at the 42 nd Avenue Crossing technical memorandum by WSB and Associates, dated June 2011.

## Existing Traffic Volumes

The existing daily traffic volumes are shown in Table 1. The PM peak hour turning movement counts are shown in Table 2.

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Table 1 Existing Daily Traffic Volumes

| Roadway | Location | Year | Daily Volume |
| :--- | :--- | ---: | ---: |
| CSAH 81 (Bottineau Blvd) | North of I-94 | 2010 | 30,000 |
| CSAH 81 (Bottineau BIvd) | North of 63rd Ave | 2010 | 25,000 |
| CSAH 81 (Bottineau Blvd) | South of 63rd Ave | 2010 | 24,200 |
| 63rd Ave | West of CSAH 81 (Bottineau BIvd) | 2010 | 8,800 |
| 63rd Ave | East of CSAH 81 (Bottineau Blvd) | 2010 | 13,300 |
| Corvallis Ave | Transitway Crossing | 2010 | 4,250 |
| Broadway Ave | Transitway Crossing | 2010 | 8,800 |
| 42nd Ave | Transitway Crossing | 2010 | 11,600 |
| 41st Ave/ Noble Ave | Transitway Crossing | 2010 | 2,150 |

Table 2 Existing PM Peak Hour Traffic Volumes

| Intersection | Count Year Agency | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| CSAH 81 (Bottineau Blvd) at 63 rd Ave | $\frac{2002}{\text { Hennepin }}$ | 158 | 1052 | 298 | 176 | 644 | 59 | 144 | 407 | 88 | 168 | 324 | 112 |

## Operations Analysis

The initial operational analysis for alignment C was completed in Synchro/ SimTraffic because it provides the ability to efficiently analyze a number of different geometric and operational parameters and more detailed analysis was completed in VISSIM where needed. Preemption can't be modeled directly in Synchro/ SimTraffic but the signal timing and phasing can be modified to reasonably approximate the impacts on traffic. VISSIM modeling more accurately replicates the transit preemption sequence and recovery, as well as accounting for truncated and skipped signal phases.

Transit is assumed to operate in a preemption scheme for all the signalized intersections along alignment C. In a preemption scheme, when the transit vehicle approaches the intersection on the transitway, the traffic signal green phase is terminated and goes to the yellow and red phase. The approach being crossed has a green phase "clearance interval" to clear any queued vehicles from the transitway crossing. The roadway green indications rest in the nonconflicting movements, which would be the CSAH 81 (Bottineau Boulevard) through movements.

The existing traffic signal at CSAH 81 (Bottineau Boulevard)/ CSAH 10 (Bass Lake Road) was excluded from alignment C existing and future operations analysis, because that work was completed under a separate technical memo. The unsignalized intersections were not analyzed, since the vehicle traffic is only stopped briefly during transitway crossings, and otherwise flows unimpeded.

## Existing Operations

The existing conditions PM peak hour analysis shows that the CSAH 81
(Bottineau Boulevard)/ 63rd Ave intersection operates at Level of Service (LOS) D. Table 3 shows the results from the existing conditions analysis.

Table 3 Existing Delay and LOS Results

| Intersection | Intersection |  | Comments |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS |  |
| CSAH 81 (Bottineau <br> Blvd) at 63rd Ave | 45.0 | D | All movements operate at LOS E or <br> better |

## FUTURE CONDITIONS

Along alignment C , several future modifications would be introduced at the following locations:

- The transitway alignment would have an overpass at TH 100.
- A future CSAH 81 (Bottineau Boulevard) reconstruction project will expand CSAH 81 (Bottineau Boulevard) to a six-lane roadway. The additional improvements at the 63rd Avenue intersection include dual
left-turn lanes on each approach, except northbound, and right-turn lanes on the 63rd Avenue approaches.


## 2030 Forecast Traffic Volumes

The forecast 2030 volumes were based on the "Bottineau Boulevard 2030 Highway and Bus Rapid Transit Ridership Forecast from March 7, 2006, by SRF Consulting Group". These are consistent with other previous work including the CSAH 81 (Bottineau Boulevard) reconstruction project and the Bottineau Transitway Alternatives Analysis "Traffic Operations Analysis Report" dated January 2010. Table 4 shows the year 2030 daily traffic volumes. Table 5 shows the 2030 PM peak hour traffic volumes.

Table 4 Year 2030 Daily Traffic Volumes

| Roadway | Location | Year | Daily Volume |
| :--- | :--- | ---: | ---: |
| CSAH 81 (Bottineau Blvd) | North of I-94 | 2030 | 46,000 |
| CSAH 81 (Bottineau Blvd) | North of 63rd Ave | 2030 | 36,000 |
| CSAH 81 (Bottineau Blvd) | South of 63rd Ave | 2030 | 35,000 |
| 63rd Ave | West of CSAH 81 (Bottineau Blvd) | 2030 | 12,400 |
| 63rd Ave | East of CSAH 81 (Bottineau Blvd) | 2030 | 18,700 |
| Corvallis Ave | Transitway Crossing | 2030 | Not available |
| Broadway Ave | Transitway Crossing | 2030 | Not available |
| 42nd Ave | Transitway Crossing | 2030 | 17,000 |
| 41st Ave/ Noble Ave | Transitway Crossing | 2030 | Not available |

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Table 5 Year 2030 PM Peak Hour Traffic Volumes

| Vntersection | Forecast Year | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| CSAH 81 (Bottineau Blvd) at 63 rd Ave | 2030 | 170 | 1600 | 325 | 290 | 900 | 100 | 260 | 625 | 110 | 220 | 475 | 240 |

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## Future Operations

Alignment C was modeled with LRT, which is assumed to operate under a transit preemption scheme in these corridors, based on the operating speeds (45 to 55 MPH) of the transit and the adjacent roadway. For this analys is, LRT was assumed to operate three-car trains at 7.5 minute headways.

The transitway was modeled with the through movement of the mainline. The preemption disrupts the phasing sequence and may result in short green times for movements conflicting with the transitway. It is possible to maintain coordination in the corridor in the future, but the frequency of preemptions would likely have operational impacts on the cross streets that would need to be weighed agains the benefits to mainline traffic. Protected only phasing is needed to control left-turn movements for vehicles traveling parallel to the guideway that then turn left across the guideway. The through movement is generally compatible with the transit movement. Left-turn movements from a cross street across the guideway can be either protected or permissive, as the through movements are not compatible with the transit phase. For the intersections where the transitway is side-running, right turn on red (RTOR) was banned for those rights turn movements across the transitway.

The operational results for alignment C under 7.5 -minute (LRT) headways are shown in Table 6. The results include future improvements to the CSAH 81 (Bottineau Boulevard) corridor.

Table 6 Year 2030 Delay and LOS Results

| Intersection | Intersection |  | Comments |
| :--- | :---: | :---: | :--- |
|  | Delay <br> (s/veh) | LOS |  |
| CSAH 81 <br> Blvd) at 63rdtineau Ave | 52.7 | D | Movements across the tracks <br> operate at LOS E or F |

## SUMMARY

The following is a summary of the results of the analysis:
Under existing conditions, the CSAH 81 (Bottineau Blvd)/63rd Ave intersection operates at LOS D.

The alignment C northern terminus is just north of the I-94 interchange with a southern terminus around 36 th Avenue. Below are the key intersections potentially impacted by the transitway:

- I-94 Interchange: no impact to the intersections as the transitway bypasses the interchange on the west side, no impact
- 63rd Avenue: side running transitway, analyzed

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- Bass Lake Road: evaluated under separate analysis
- Corvalias Avenue Crossing, West Broadway Avenue Crossing, 45 1/2 Avenue, 41st Avenue/ Noble Avenue Crossing, and $391 / 2$ Avenue Crossing Crossing: non-intersection gated crossings, not analyzed
- TH 100 overpass: grade separated, no impact
- 42 nd Avenue Crossing: non-intersection gated crossing, evaluated under separate analysis

Under year 2030 future conditions, the future CSAH 81 (Bottineau Boulevard) reconstruction to six-lanes was assumed along with left-turn lane and right-turn improvements at the CSAH 81 (Bottineau Blvd)/ 63rd Avenue.

The analyzed intersection at CSAH 81 (Bottineau Boulevard)/ 63rd Avenue is expected to operate at LOS D given the above improvements under year 2030 conditions.
\& Associates, Inc.

## Technical Memorandum

| To: | Joe Gladke, PE | Copy: $n / a$ |
| :--- | :--- | :--- |
| From: | Chad Ellos, PE <br>  <br> Tony Heppelmann, PE |  |
| Date: | October, 2012 (Revised) | File: |
| Subject: | Modeling Assumptions No. 01484-070 <br>  <br> Bottineau LRT Transitway - Broadway Avenue |  |

The purpose of this memorandum is to list the assumptions to be used in the modeling analysis of LRT running along the Broadway Avenue alignment. Assumptions were arrived at through conversations with Hennepin County staff and by reviewing previous documents created for the Bottineau Transitway Draft Environmental Impact Statement. Most assumptions were taken from the two documents listed below.

- Proposed LRT and BRT Operating Plan Assumptions (8-23-2011)
- Transit/Traffic Signal Operating Schemes Technical Memorandum (12-6-2011)


## 1. PROJECT LIMITS

The extents of the project include Broadway Avenue from just north of TH 610 North Ramp intersection to just south of the $76{ }^{\text {th }}$ Avenue intersection.

## 2. LRT ALIGNMENT

The LRT alignment was assumed to be side-running along the west side of Broadway Avenue from the northern extents to $94^{\text {th }}$ Avenue. At $94^{\text {th }}$ Avenue, it was assumed that the LRT alignment would transition to center-running and remain in the Broadway Avenue median to the southern extents of the project.

## 3. STATIONS

Three stations are located in the study area. These include:

- $93^{\text {rd }}$ Avenue P\&R Station
- $85^{\text {th }}$ Avenue Station
- Brooklyn Boulevard Station

Each station is located in the roadway median and consists of far-side platform locations on each side of the signalized intersection. Dwell time at stations was assumed to be 20 seconds.

## 4. LRT CHARACTERISTICS

- Maximum Speed $=45 \mathrm{mph}$
- Acceleration $=3.0 \mathrm{mphps}$
- Deceleration $=3.0 \mathrm{mphps}$
- Length $=188$ feet ( 2 cars at 94 feet per car)
- Full Priority (Preemption) at signalized intersections
- Average LRV Headway $=7.5$ minutes in each direction. An assumed random variance of plus or minus $20 \%$ around the scheduled time will be used to develop a random arrival schedule.


## 5. ADDITIONAL TRAFFIC AND GEOMETRIC CHARACTERISTICS

- Traffic Speeds on Broadway Avenue $=45 \mathrm{mph}$
- Forecast 2030 traffic volumes for a 4-Lane alternative were provided by Hennepin County and used in this analysis
- The roadway sections on CSAH 103 was assumed to be four lanes from south of $76{ }^{\text {th }}$ Avenue (south project limit) to north of the TH 610 North Ramp intersection (north project limit)
- Dual left-turn lanes for:
- EB approach at Brooklyn Boulevard
- EB and WB approaches at $85^{\text {th }}$ Avenue
- EB approach at $93^{\text {rd }}$ Avenue
- SB approach at the TH 610 North Ramp intersection
- Dedicated left-turn lanes have protected phasing on all approaches paralleling the tracks


## 6. GATE ARM OPERATIONS

Gate arms were assumed at all signalized intersections since the LRV speed was greater than 35 mph . During the preempt call, the mainline left turn phases were assumed to receive a track clearance phase prior to the gates descending. Cross-street traffic did not receive a track clearance phase. Permitted left-turning vehicles that may be waiting on the tracks were assumed to clear the intersection during the two seconds of all-red prior to the mainline left's track clearance phase. Generally, gate arm operations were assumed to follow the time tolerances listed below.

- Flashing warning prior to gates descending $=3$ to 5 seconds
- Gates descending $=10$ to 12 seconds
- Time after gates are down and before transit vehicle arrives at crossing $=5$ seconds
- Transit vehicle crossing the roadway = varies
- Gates ascending $=5$ seconds

TOTAL TIME = Approximately 25 seconds plus transit vehicle crossing time.

## 7. TRAFFIC SIGNAL - RECOVERY SEQUENCE

Once a preempt call was terminated, the traffic signal recovery sequence went first to the EBL and EBT phases, then to the next phase in the ring and barrier. The next phase was usually the WB through movement followed by the WB left-turn phase. Since the mainline left turning vehicles received a clearance phase prior to the LRV arriving at the intersection and the mainline through movements went with the LRV, the EB approach was the remaining critical movement that has the heaviest traffic volume (movement that had been delayed the longest). Recovering first to the EB approach allowed the intersection to best recover while minimizing vehicle queuing on all approaches.

2030 PM Peak Hour 4-Lane Alternative Traffic Volumes
VISSIM Simulation Summary Results LRT Preemption
Date: Monday, October 01, 2012
Measures of Effectiveness



[^0]:    ${ }^{1}$ The initial operational analysis for Alignment A was completed using Synchro/SimTraffic because the program provides the ability to efficiently analyze a number of different geometric and operational parameters. VISSIM was used to model operational features beyond the capability of the Synchro/SimTraffic software where needed. For additional discussion, see the Bottineau Alignment A Traffic Analysis Memorandum in the Appendix.

[^1]:    ${ }^{1}$ Daily traffic volumes obtained from $2006 \mathrm{Mn} /$ DOT MSA traffic flow maps.

[^2]:    ${ }^{1}$ Deceleration, dwell, and acceleration time.
    ${ }^{2}$ Deceleration, dwell, and acceleration time.

[^3]:    * Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
    approximately 4,645 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
    Source: WSB \& Associates
    :01484-05:Traffici([LOS Results.x|l]2030AM_LRT(36hh)

[^4]:    * Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
    approximately 4,645 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
    Source: WSB \& Associates
    :. $011484-05$ TTraffic [LOS Results. XIS]2030PM _LRT(36th)

[^5]:    *Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
    approximately 5,475 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
    Source: WSB \& Associates
    K: 01484-05STraffic|(LOS Results.xls/2030AM_LRT(344h)

[^6]:    * Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
    approximately 4,545 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
    ource: WSB \& Associates
    K:01484-051Traffic(LLOS Results.xls]2030AM_LRT(36h)_GROWTH

[^7]:    * Average LRT travel time was calculated from a point just north of 36th Avenue to just south of Abbott Avenue. The distance following the LRT alignment was
    approximately 4,545 feet. The travel time includes deceleration, dwell, and acceleration time associated with the North Memorial Station.
    source: WSB \& Associates
    K:O1484-05:Trafficic[LOS Results.xls]2030PM_LRT(36th)_GROWTI

[^8]:    ${ }^{1}$ Daily traffic volumes obtained from 2009 Mn /DOT traffic flow maps

[^9]:    ${ }^{2}$ Assumptions from: Central Corridor Light Rail Transit, Section 344260 - Signals Highway Crossings (July 2010)
    ${ }^{3}$ Range in times is due to the proximity of a station near the crossing. Transit vehicles are either accelerating or decelerating through the crossing.

[^10]:    ${ }^{1}$ Bottineau Transitway Draft Environmental Impact Statement, Transit/Traffic Signal Operating Schemes Technical Memorandum, Kimley-Horn and Associates, Inc., December 6, 2011.

